## West Culburra Beach Concept Plan



Prepared by: GTA Consultants (NSW) Pty Ltd for Sealark Pty Ltd
on 23/10/2020
Reference: N186580
Issue \#: C

# West Culburra Beach Concept Plan 

## Transport and Accessibility Impact Assessment

## Client: Sealark Pty Ltd

on 23/10/2020
Reference: N186580
Issue \#: C

Quality Record

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## 1. INTRODUCTION



## INTRODUCTION

### 1.1. Background

The development proposed at West Culburra Beach involves a mixed use subdivision over approximately 47 hectares (ha) of land bounded to the north by the Crookhaven River, Lake Woollumboola and the existing urban area of Culburra Beach to the east, Jervis Bay National Park to the south and private property to the west. Shoalhaven Council has designated the land allotment which the site is located as the Culburra Beach Expansion Area.

GTA Consultants was previously commissioned by Realty Realizations Pty Ltd to complete a transport and accessibility impact assessment (GTA 2013 TIA) for the proposed development. Previously, the West Culburra Beach Development included a mixture of medium density housing types, ranging from small lots for the 55+ aged group to multi-storey units. The West Culburra Beach development Concept plan has now been updated.

The updated proposal comprises three key precincts, proposed to be constructed over a period of approximately 10 years. On completion, the West Culburra Beach Development is proposed to include:

- Residential: approximately 293 low density residential dwellings ( 244 residential lots, assuming $20 \%$ will be dual occupancy dwellings) totalling 22.97 hectares
- Industrial: 13 industrial lots, totalling 5.12 hectares.
- Town Centre: A town centre totalling approximately 13.58 hectares, including:
- 95 medium density low-rise apartments
- 45 integrated housing units, 3 shop-top housing and commercial/ retail units
- A local sports facility.

Access to the residential and town centre development is proposed via three new roundabout intersections with Culburra Road, while a priority intersection is proposed for accessing the industrial development. It is proposed that the existing industrial area access at Strathstone Street will be closed with new access 30 metres east of the existing access point.

GTA Consultants was commissioned by Allen Price \& Scarratts on behalf of Sealark Pty Ltd to undertake a Transport and Accessibility Impact Assessment (TAIA) for the updated West Culburra Beach Concept Plan, and in particular to address Section 5 (Traffic and Access) of the Director-General's Environmental Assessment Requirements (DGR's) dated 27 May 2010, presented in Section 1.2.1.

### 1.2. Purpose of this Report

This report sets out an assessment of the anticipated transport implications of the proposed development, including consideration for the following:

- existing traffic and parking conditions surrounding the site
- pedestrian and bicycle requirements
- service vehicle considerations
- the traffic generating characteristics of the proposed development
- suitability of the proposed access arrangements for the site
- the transport impact of the development proposal on the surrounding road network.


## INTRODUCTION

### 1.2.1. Response to Director-General's Requirements

The Director General's Requirements (DGRs) issued on 27 May 2010 are addressed in this report. As they relate to traffic and transport, the DGRs have been reproduced in Table 1.1, with the relevant section(s) of this report referenced.

Table 1.1: Response to DGRs

| Traffic and Access | Response |  |
| :--- | :--- | :--- | :--- |
| 5.1) | Prepare a Transport and Accessibility Impact Study in accordance with Table 2.1 of the <br> Roads and Maritime Guide to Traffic Generating Developments, having regard to the <br> principles of the NSW Planning Guidelines for Walking and Cycling and the NSW State Plan <br> (2010) to include: |  |
| a) | Details and analysis of proposed access to the site. | Section 4 |
| b) | Network modelling using TRACKS. | Section 2.8 \& 7 |
| c) | Appropriate arrangements for the provision of road and public transport infrastructure needed <br> to service the site. Specifically in relation to the Nowra/Culburra bus service, inclusive of the <br> feasibility of the proposed diversion of the existing service, early provision of the service and <br> funding. | Section 5.2 |
| d) | An assessment based on the current speed zonings, with consideration of safe spacing of <br> intersections within 100km/h speed zones. | Section 0 |
| e) | An assessment of the impacts on the surrounding road network. | Section 7.3 |
| 5.2) | Provide for a road network allows for (potential) future public access to the coastal foreshore. | Section 5.4 |
| 5.3) <br> Transport policy package. |  | Section 5 |

### 1.3. References

It is noted that Roads and Maritime Services is now integrated with Transport for NSW (TfNSW). For the purposes of this report, all references to the former Roads and Maritime Services have been retained for clarity. In preparing this report, reference has been made to the following:

- an inspection of the site and its surrounds
- West Culburra Beach Subdivision Plan Transport and Accessibility Impact Assessment (Issue E), GTA Consultants, 2013 (GTA 2013 TIA)
- West Culburra Beach Subdivision Development Transport and Accessibility Impact Assessment Addendum Report, GTA Consultants, 2013
- Austroads Guide to Road Design, Part 3: Geometric Design (third edition) 2016
- Austroads Guide to Road Design, Part 4A: Unsignalised and Signalised Intersections (third edition) 2017
- Austroads Guide to Road Design, Part 4B: Roundabouts (third edition) 2015
- Austroads Guide to Road Design, Part 6A: Paths for Walking and Cycling (second edition) 2017
- Integrating Land Use and Transport 2001, NSW Department of Urban Affairs and Planning
- Sydney's Cycling Future, Transport for NSW, 2013
- Future Transport 2056, Regional NSW Services and Infrastructure Plan, Transport for NSW, 2018
- Nowra Bomaderry Structure Plan 2006, Shoalhaven City Council
- NSW Bicycle Guidelines, Roads and Maritime Services, 2005


## INTRODUCTION

- NSW Speed Zoning Guidelines, Roads and Maritime Services, 2011
- NSW Walking and Cycling Program Guidelines 2019-2020, Roads and Maritime Services
- Roads and Maritime Traffic Modelling Guidelines, Roads and Maritime Services, 2013
- Shoalhaven City Council, Traffic and Transport Unit, Calculation of Traffic Growth Factors \& Trip Generation Rates, correspondence dated 19 February 2013
- Shoalhaven Development Control Plan (SDCP 2014) Chapter G21 Car Parking Code 2014
- Shoalhaven City Council DCP, Culburra Beach Expansion Area 671996
- Shoalhaven City Council Draft DCP Culburra Beach Expansion Area DCP 67/ N13 under draft, being prepared by Shoalhaven City Council
- Shoalhaven Draft Local Environmental Plan (LEP) 2009
- Shoalhaven Local Environmental Plan (LEP) 2014
- Shoalhaven Integrated Transport Strategy, 2000
- Traffic and Parking Assessment - Proposed 18 Hole Championship Golf Course, Long Bow Point, Culburra Beach 2012, prepared by Traffic Solutions Pty Ltd
- Traffic Impact Assessment, Proposed Residential Planning Proposal, Callala Bay NSW (Revision 3), Stantec, 2019
- traffic surveys undertaken by Matrix Traffic and Transport (formerly Skyhigh) in May 2012 as referenced in the context of this report
- plans for the proposed development site prepared by Allen Price and Scarratts:
- Proposed Mixed Use Concept Plan (Drawing Number 25405-210, Revision 08)
- Preliminary Residential Precinct Layout Plan (Drawing Number 25405-101, Revision P1)
- Preliminary Industrial Precinct Layout Plan (Drawing Number 25405-102, Revision P1)
- Preliminary Town Centre Precinct Layout Plan (Drawing Number 25405-103, Revision P1)
- Preliminary Overall Concept Roundabout 01 Design (Drawing Number 25405-104, Revision P6)
- Preliminary Concept Roundabout 01 Design (Drawing Number 25405-105, Revision P6)
- Roundabout 01,02 and 03 Vehicle Movement Layout Plan (Drawing Number 25405-106, 107, Revision P6,25405-112, Revision P1 and 25405-116, 117, Revision P1)
- Preliminary Roundabout Sight Line Layout Plan (Drawing Number 25405-108, Revision P6, 25405 113, Revision P1 and 25405-118, Revision P1)
- Preliminary Concept Industrial Entry and Exit Layout Plan (Drawing Number 25405-109, Revision P4)
- Preliminary Concept Roundabout 02 Layout Plan and Design (Drawing Number 25405-110,111 Revision P1)
- Preliminary Concept Roundabout 03 Layout and Design (Drawing Number 25405-114, 115 Revision P1)
- Preliminary Typical Road Cross Sections Plan (Drawing Number 25405-119, 120, 121, 122,123 Revision P0)
- Proposed Mixed use Concept Plan- Bus Routes (Drawing Number 25405-220, Revision 01)
- Proposed Mixed use Concept Plan- Shared Footpath (Drawing Number 25405-221, Revision 01).
- other documents and data as referenced in this report.


## 2. EXISTING CONDITIONS



## EXISTING CONDITIONS

The subject site is located at Culburra Beach, 180 kilometres south of Sydney and 20 kilometres east of Nowra and covers an area of approximately 47 hectares. Under Shoalhaven LEP 2014, the site is located within a Deferred Matter (DM) Zone and is otherwise known as Lots 5 \& 6 of DP1065111.

The surrounding properties predominantly include residential and commercial uses to the east. The location of the subject site and its surrounding environs is shown in Figure 2.1.

Figure 2.1: Subject Site and Environs


Basemap: Google MyMaps (accessed March 2020)

## EXISTING CONDITIONS

### 2.1. Existing Road Network

### 2.1.1. Adjoining Roads

## Princes Highway

The Princes Highway (A1) is generally aligned in a north-south direction between the Sydney CBD and the Victorian border. Across the Nowra Bridge, the road carries approximately $48,000^{1}$ vehicles per day. As it pertains to the site, it is a two-way, divided 23 metres wide road set within an undefined/ variable road reserve, generally configured with two lanes in each direction with wide road shoulders. In the vicinity of the turn-off to Culburra Beach, the road is subject to $70 \mathrm{~km} / \mathrm{h}$ posted speed limit.

The Princes Highway is currently being upgraded between Berry and Bomaderry as part of a $\$ 450$ million planned upgrade along the Princes Highway Corridor. The Nowra Bridge over the Shoalhaven River is also planned to be upgraded, with construction works being recently awarded to a contractor.

## Culburra Road

Culburra Road is a Regional Road generally aligned in an east-west direction and carries approximately 5,200 vehicles per day ${ }^{2}$. It is a two-way, 7 -metre-wide road set within a 20 -metre-wide road reserve (approx.), configured with one traffic lane in each direction. Culburra Road is the key link between Culburra Beach and Nowra to the west and, in the vicinity of the site, is subject to a $100 \mathrm{~km} / \mathrm{h}$ posted speed limit and $80 \mathrm{~km} / \mathrm{h}$ elsewhere. North of Mayfield Road, the name of the road changes to Pyree Lane.

## Pyree Lane

Pyree Lane is a Regional Road aligned in a north-south direction and carries approximately 5,200 vehicles per day². It is a two-way, 6-metre-wide road set within a 22-metre-wide road reserve (approx.), configured with one traffic lane in each direction. Pyree Lane is the key link between Culburra Beach and Nowra to the west and is subject to an $80 \mathrm{~km} / \mathrm{h}$ posted speed limit. South of Mayfield Road, the name of the road changes to Culburra Road.

## Coonemia Road

Coonemia Road is a Local Road to the west of the site and is aligned in a north-south direction. It is a two-way, 7-metre-wide road set within a 20-metre-wide road reserve (approx.), configured with one traffic lane in each direction and carries approximately 2,600 vehicles per day ${ }^{1}$. Coonemia Road links Culburra Beach and the coastal villages of Callala Bay, Callala Beach and Currarong to the south.

## Greenwell Point Road/ Kalandar Street

Greenwell Point Road is a Regional Road aligned in an east-west direction. It is a two-way, 6-metre-wide road set within a 20-metre-wide road reserve (approx.), configured with one lane in each direction and carries approximately 6,000 vehicles per day². Greenwell Point Road provides the sole road access between Nowra and the coastal village of Greenwell Point. West of McKay Street in East Nowra, the name of the road changes to Kalandar Street.
[1] Based on RMS Traffic Volume Viewer
[2] Based on the peak hour traffic counts undertaken by Skyhigh in May 2012 and assuming a peak-to-daily ratio of 8\% for arterial roads and $10 \%$ for local roads.

## EXISTING CONDITIONS

## Forest Road

Forest Road is a local road aligned in an east-west direction. It is a two-way, 7-metre-wide road set within a 20-metre-wide road reserve (approx.), configured with one lane in each direction and carries approximately 2,600 vehicles per day ${ }^{2}$. Forest Road is the key link between the coastal villages of Callala, Currarong and the Princes Highway.

### 2.1.2. Surrounding Intersections

The following key intersections currently exist in the vicinity of the site:

- Culburra Road/ Coonemia Road (unsignalised)
- Pyree Lane/ Greenwell Point Road (unsignalised)
- Princes Highway/ Kalandar Street (signalised)
- Princes Highway/ Moss Street (signalised)
- Princes Highway/ Forest Road (unsignalised).


### 2.2. Existing Traffic Volumes

GTA Consultants commissioned traffic movement counts, queue lengths and travel time surveys on key intersections and roads surrounding the site in May 2012 as part of the GTA 2013 TIA and are shown in Figure 2.2. The intersection traffic movement counts were undertaken by Matrix Traffic and Transport Data (formerly Skyhigh) during the following peak periods:

- Friday 04 May 2012:
- 7:00am to 9:00am
- 4:00pm to 6:00pm
- Saturday 05 May 2012:
- 12:00pm to 2:00pm.

The existing weekday AM and PM peak hour traffic volumes are summarised in Figure 2.3 with Saturday peak hour traffic volumes summarised in Figure 2.4. Full results of the traffic movement counts are contained in Appendix A.

It is acknowledged that the survey data used as a basis for this assessment was collected some eight years ago (in 2012). However, the assessment and the corresponding results and conclusion are based on the $120^{\text {th }}$ highest hour equivalent traffic volumes to account for seasonal traffic. Also, the growth rates applied are similar to the historical growth rates indicated by survey data. These are discussed further in detail in Section 2.8.1.

As such, the data informing study outcomes is still considered fit-for-purpose.


WEST CULBURRA DEVELOPMENT

| Traffic Counts |  |  |  | Location | Count |
| :--- | :--- | :--- | :---: | :---: | :---: |
| Road/Intersection | W of Ingle Ring | Link |  |  |  |
| 1.The Lake Circuit | W of West Cres | Link |  |  |  |
| 2.Culburra Rd | W of Strathstone St | Link |  |  |  |
| 3.Culburra Rd |  | Movement |  |  |  |
| 4.Culburra Rd/Coonamia Rd |  | Movement |  |  |  |
| 5.Culburra Rd/Mayfield Rd |  | Movement |  |  |  |
| 6.Pyree Ln/Greenwell Pt Rd |  | Movement |  |  |  |
| 7.Greenwell Pt Rd//lindy Andy Ln |  | Movement |  |  |  |
| 8.Greenwell Pt Rd/Mayfield Rd |  | Movement |  |  |  |
| 9.Greenwell Pt Rd/Millbank Rd |  | Movement |  |  |  |
| 10.Kalandar St/Princes Hwy |  | Link |  |  |  |
| 11.Greenwell Pt Road | W of McKay St | Movement |  |  |  |
| 12.Coonamia Rd/Currarong Rd |  | Lovement |  |  |  |
| 13.Kalandar St/Kinghorne St |  | S of Kinghorne St |  |  |  |
| 14.Princes Hwy | Link |  |  |  |  |
| 15.Princes Hwy/Forest Rd |  | Movement |  |  |  |
| 16.Coonamia Rd/Forest Rd |  | Movement |  |  |  |
| 17.Princes Hwy/Moss St |  | Link |  |  |  |
| 18.Princes Hwy | S of Parma Road | Link |  |  |  |
| 19.Princes Hwy | Shoalhaven River | Link |  |  |  |
| 20.Albatross Rd | E of Yalwal Rd | Link/Movement |  |  |  |
| 21.BTU Rd | W of Princes Hwy | Link/Movement |  |  |  |
| 22.Princes Hwy | S of Central Ave | Link/Movement |  |  |  |
| Travel time survey |  |  |  |  |  |

Travel time survey
1.Penguins Head Rd East to Princess Hwy via Greenwell Pt Rd
2.Coonamia Road North to Princess Hwy via Forest Rd
3.Princes Hwy between Moss St and Parma Road
sub-sections travel time
A.Prince Edward Ave
B.Culburra Rd
C.Culburra Rd
D.Culburra Rd
E.Pyree Ln

Greenwell Point Rd

| Jindy Andy Ln to Millbank Rd |
| :--- | :--- |

G.Greenwell Point Rd/Kalandar St Millbank Rd to Princes Hwy

Key

- Classified Link Count (Tube count)
- Turning movement Count
- Classified Link/Turning movement Count
$\longleftrightarrow$ Main route travel time survey
A-A Section iravelime survey

Figure 2.3: Existing AM/PM Peak Hour Traffic Volumes



## EXISTING CONDITIONS

### 2.3. Relevant Policy and/or Case Studies

### 2.3.1. Long Bow Point Golf Course, Traffic and Parking Assessment

A traffic and parking assessment was prepared by Traffic Solutions Pty Ltd in March 2012 to support a Development Application (DA) for a proposed 18 hole championship golf course at Long Bow Point, Culburra Beach. The proposed golf course is located west of the established residential areas of Culburra Beach on the southern side of Culburra Road as shown in Figure 2.5

Figure 2.5: Proposed Golf Course, Long Bow Point, Culburra Beach


Basemap: Google Maps (accessed March 2020)
Traffic generation estimates for the proposed golf course development were based on surveys undertaken on a Wednesday and Saturday at Nowra Golf Club. It was estimated that the proposed golf course would generate 33 and 53 vehicle movements (two-way) during the weekday AM and PM peak periods, and 66 vehicle movements (two-way) during a Saturday peak hour.

As GTA Consultants understands it, the SSD 8406 for the development has been refused by the Independent Planning Commission on 26 September 2012.

### 2.3.2. Nowra Bomaderry Structure Plan

The Nowra Bomaderry Structure Plan was prepared and adopted by Shoalhaven City Council in October 2006 and was endorsed by NSW Department of Planning in 2008. The Structure Plan identified major infrastructure needs within the Nowra Bomaderry area. The East Nowra Sub-Arterial has been identified as one of the key infrastructure projects and Shoalhaven City Council is currently seeking funding opportunities for this project.

## EXISTING CONDITIONS

## East Nowra Sub-Arterial Road

The East Nowra Sub Arterial Road (ENSA) is proposed to connect Greenwell Point Road (in the vicinity of Old Southern Road) to the Princes Highway, at North Street and Junction Street. ENSA will provide a much-needed alternative connection to the highway from the East Nowra, Worrigee and coastal village areas. The link will relieve pressures along the Kalandar Street/ Princes Highway route to the Nowra CBD. One of the key identified aims for ENSA is to "Remove traffic from rural lanes such as Jindy Andy Lane and Millbank Road which have increased in traffic and have experienced high crash rates." An indicative alignment of the ENSA is shown in Figure 2.6.

Figure 2.6: East Nowra Sub-Arterial (ENSA) Alignment


Source: Nowra CBD Transport Strategy, 2003 (Eppell Olsen and Partners)

## EXISTING CONDITIONS

### 2.3.3. Princes Highway Upgrade Program

The Princes Highway Upgrade Program is a series of planned upgrades along Princes Highway to improve safety and efficiency along the route. Current projects include the following:

- Albion Park Rail Bypass (scheduled for 2022 completion) - As of March 2020, access upgrades have been opened between Woollybutt and Durgadin Drive. The project serves to tie-in the existing Princes Highway at Dapto and Oak Flats and interchanges at the Illawara Highway and at Tongarra Road
- Princes Highway upgrade at Gerringong (completed)
- Foxground and Berry bypass (completed)
- Berry to Bomaderry upgrade (scheduled for 2022 completion) - As of February 2020, sections of the new alignment that form part of the overall Berry to Bombaderry project has started operation. These works having commenced in September 2018 and expected to be fully complete by 2022. The project does not allow for ramps near the sites access, but is expected to improve safety, increase capacity, improve traffic flow, deliver better and more reliable journeys and increase overtaking opportunities
- Nowra Bridge project (scheduled for 2024 completion) - As of February 2020, it has been announced that the nearby, and network-related Nowra Bridge has been commissioned for upgrades to better cater for heavy and over-weight vehicles, as well as the expansion from a two-way two-lane bridge to a two-way four-lane bridge
- Termeil Creek upgrade (completed)
- Burrill Lake Bridge (completed)
- Batemans Bay Bridge replacement project (scheduled for 2023 completion).

A 6.3km section of the Princes Highway has been upgraded since the GTA 2013 TIA between Kinghorne Street and Forest Road. Key features of the upgrade include:

- duplication of the Princes Highway from two to four traffic lanes
- realignment of the Princes Highway between Warra Warra Road and Forest Road, west of the road's previous alignment
- reconstruction of the Forest Road intersection to allow all turning movements
- relocation of the BTU Road intersection approximately 400 metres north of its former location
- new pedestrian and cycling facilities.

The Review of Environmental Factors (REF) for the project was completed in November 2009 and estimated a 2.5\% linear growth rate in traffic volumes along this section of the Princes Highway, up to 2028. This growth rate was based on the recorded Annual Average Daily Traffic (AADT) counts taken on the Princes Highway (station number 07.707) over a five-year period, being 25,636 in 2003 to 27,888 in 2008. The REF projected traffic volumes are summarised in Table 2.1.

## EXISTING CONDITIONS

Table 2.1: Princes Highway Projected Traffic Volumes (Princes Highway Upgrade REF, 2009)

| Year | Annual Average Daily Traffic <br> (AADT) | Growth (\%per annum) |
| :---: | :---: | :---: |
| 2012 | 29,511 |  |
| 2018 | 33,688 | $2.4 \%$ |
| 2022 | 34,919 | $1.8 \%$ |
| 2028 | 39,250 | $2.1 \%$ |

The recorded Annual Average Daily Traffic (AADT) counts taken on the Princes Highway (station number 07051) for different years was obtained to understand the actual growth along Princes Highway and is presented in Table 2.2.

Table 2.2: Princes Highway AADT - South of Illaroo Road (RMS Traffic Volume Viewer)

| Year | Annual Average Daily Traffic <br> (AADT) | Growth (\%per annum) |
| :---: | :---: | :---: |
| 2011 | 48,626 |  |
| 2015 | 51,676 | $1.6 \%$ |
| 2017 | 53,580 | $1.7 \%$ |

The per annum growth rate between 2011 and 2017 (1.7\%) is similar to the growth rate projected between 2012 and $2022(1.8 \%)$ along the Princes Highway, resulting in a good correlation between the projected growth rates in the REF and actual growth rates to-date.

Survey data (intersection turn counts) along key intersection at Princes Highway was collected by GTA for a different project in the Nowra Region. 2012 data was compared to the 2018 data at Princes Highway / Moss Street intersection. Growth rate between the 2012 and 2018 counts at Princes Highway was similar to that presented in Table 2.2.

### 2.3.4. Callala Bay Residential Planning Proposal

A traffic impact assessment was prepared by Stantec in 2019 for a proposed Callala Bay residential development and associated planning proposal. The Callala Bay development site is approximately 8.5 kilometres south-west of the West Culburra Beach site and involves some 367 low-density dwellings. The proposal is expected to generate 261 trips in the AM peak hour and 287 trips in the PM peak hour. The report concluded that, with the additional development traffic, all intersections assessed continued to perform at good operating levels (Level of Service A) as the surrounding network has abundant capacity to accommodate the additional traffic.

The traffic analysis completed assessed local intersections, with the only overlap with the assessment in this report being the intersection of Forest Road and Callala Beach Road. Should this rezoning proceed, further cumulative assessment may be warranted for the Princes Highway/ Forest Road intersection. As subsequently identified in this report, additional traffic volumes at the Princes Highway / Forest Road intersection generated by the West Culburra Beach development is low. As such, the cumulative impacts of the Callala Bay residential planning proposal have not been considered further in this report.

## EXISTING CONDITIONS

Figure 2.7: Callala Bay residential planning proposal site location


Source: Traffic Impact Assessment, Proposed Residential Planning Proposal, Callala Bay NSW (Revision 3), Stantec, 2019

### 2.4. Existing Public Transport

The site is approximately 850 metres west of the Culburra Beach Police Station bus stop, serviced by Routes 110 and 111.

The two services combined provide up to 10 services per day in each direction and connects the local area of Culburra Beach with Greenwell Point, Nowra and Bomaderry, which also serves as a connection to the regional rail services at Bomaderry Station. The bus route is shown in Figure 2.8.

## EXISTING CONDITIONS

Figure 2.8: Bus Routes


Source: Transport for New South Wales, Mapbox and OpenStreetMap (accessed February 2020)

### 2.5. Existing Pedestrian Infrastructure

The nearest dedicated pedestrian infrastructure is located within the established residential areas and town centre of Culburra Beach, immediately east of the proposed development. The footpath network within the urban area of Culburra Beach is limited with many streets having wide verges instead of constructed footpaths.

### 2.6. Existing Cycle Infrastructure

The nearest dedicated cycle infrastructure to the site is a 2.5 -metre-wide shared path adjacent to Prince Edward Avenue between The Lake Circuit and Penguins Head Road, connecting to the Culburra Beach town centre. This is shown in Figure 2.9.

## EXISTING CONDITIONS

Figure 2.9: Culburra Beach Bicycle Routes


Source: Shoalhaven Bike Map (2013) - Map 008, Shoalhaven City Council

### 2.7. Crash Analysis

GTA Consultants obtained vehicle crash data from Roads and Maritime for the following 11 key intersections between Culburra Beach and Nowra for the five-year period from 2014 to 2018 (crash statistics for year 2019 onwards are not yet available at the TfNSW website):

- Culburra Road/ Coonemia Road
- Culburra Road/ Mayfield Road
- Greenwell Point Road/ Pyree Lane
- Greenwell Point Road/ Jindy Andy Lane
- Greenwell Point Road/ Mayfield Road
- Greenwell Point Road/ Millbank Road/ Worrigee Road
- Princes Highway/ Kalandar Street
- Coonemia Road/ Currarong Road/ Forest Road
- Kalandar Street/ Kinghorne Street/ Albatross Road
- Princes Highway/ Forest Road
- Princes Highway/ Moss Street.

The crash history within 100 metres of the approaches to the above intersections were analysed to determine whether there any crash clusters or safety issues at these locations. The results of the crash analysis are presented in Table 2.3.

## EXISTING CONDITIONS

Table 2.3: Reported Crash Summary (January 2014 - December 2018)

| Intersection No. | Intersection Name | No. of Crashes <br> (within 100 m of intersection) |
| :---: | :--- | :--- |
| 1 | Culburra Road/ Coonemia Road | 5 (resulting in 8 people injured) |
| 2 | Culburra Road/ Mayfield Road | 0 |
| 3 | Greenwell Point Road/ Pyree Lane | 4 (resulting in 3 people injured, 1 fatality) |
| 4 | Greenwell Point Road/ Jindy Andy Lane | 2 (resulting in 1 person injured) |
| 5 | Greenwell Point Road/ Mayfield Road | 4 (resulting in 1 person injured) |
| 6 | Greenwell Point Road/ Millbank Road/ Worrigee Road | 3 (no injuries) |
| 7 | Princes Highway/ Kalandar Street | 17 (resulting in 13 injured) |
| 8 | Coonemia Road/ Currarong Road/ Forest Road <br> (includes Forest Road/ Callala Bay Road intersection) | 1 (no injuries) |
| 9 | Kalandar Street/ Kinghorne Street | 4 (resulting in 1 person injured) |
| 10 | Princes Highway/ Forest Road | 3 (no injuries) |
| 11 | Princes Highway/ Moss Street | 20 (resulting in 23 injured) |

Table 2.3 presents that notable crash clusters occurred at Intersections 3 (Greenwell Point Road/ Pyree Lane), 7 (Princes Highway/ Kalandar Street) and 11 (Princes Highway/ Moss Street). Crash analysis has been carried out at these three intersections and these refer to the Road User Movement (RUM) code classification which is presented in Table 2.4.

## EXISTING CONDITIONS

Table 2.4: RUM Code Classification


Source: Transport for NSW (accessed February 2020)

## EXISTING CONDITIONS

### 2.7.1. Greenwell Point Road/ Pyree Lane

The location and the severity of the incidents at this intersection is presented in Figure 2.10.
Figure 2.10:Crash Map - Greenwell Point Road/ Pyree Lane


Source: Centre for Road Safety, Transport for New South Wales (accessed March 2020)
A fatal crash of type RUM code 21 was recorded at this intersection which occurs between a turning vehicle and straight travelling vehicle as presented in Figure 2.11.

Figure 2.11:Collision at Greenwell Point Road/ Pyree Lane


Basemap: Nearmap (captured 2019)

## EXISTING CONDITIONS

The speed zone along Greenwell Point Road is $80 \mathrm{~km} / \mathrm{h}$. Local area traffic management treatment may be effective in reducing speeds, resulting in fewer, and less severe crashes. The RUM -21 crash was an isolated incident and the intersection can be made safer via traffic management treatments at the intersection.

### 2.7.2. Princes Highway/ Kalandar Street

The location and the severity of the incidents at this intersection is presented Figure 2.12.
Figure 2.12:Crash Map - Princes Highway/ Kalandar Street


Source: Centre for Road Safety, Transport for New South Wales (accessed March 2020)
Majority of the crashes at this intersection are classified within the RUM codes 30 to 39, indicating that incidents are reported for vehicles travelling in the same direction. This suggests that driver behaviours may be unsafe at this intersection, which may be related to signal control planning encouraging unsafe manoeuvres or rapid breaking.

### 2.7.3. Princes Highway/ Moss Street

The location and the severity of the incidents at this intersection is presented in Figure 2.13.
Figure 2.13:Crash Map - Princes Highway/ Moss Street


[^0]
## EXISTING CONDITIONS

The majority of crashes at this intersection are classified as either RUM code 21 or RUM code 30. It is noted that filter right turns are in operation at this intersection. Considering the RUM 21 classification, providing signalling for exclusive right turns may be beneficial, as filter right turn behaviour may be the cause for right-through collisions. It is also noted that Princes Highway is proposed to be upgraded just north of Moss street as part of the Nowra Bridge upgrade ${ }^{2}$ Project.

### 2.8. Existing Intersection Operation

The DGRs stipulate that network modelling be undertaken using TRACKS modelling software to assess the current (and future) performance of the intersections in the study area.

TRACKS is a suite of software programs produced by Gabites Porter Consultants of Christchurch, New Zealand. The traffic authority has a TRACKS model of the area that is required for use as the basis of our analysis. However, GTA was unable to obtain a model prior to the 2013 TIA and this has not been revisited. On 14 May 2012, Scott Wells, Traffic and Transport Unit Manager, Shoalhaven City Council wrote:
"There has been an ITUC meeting to discuss third party use of TRACKS models, I am yet to see the minutes, however there was general acceptance, subject to conditions. It was agreed there would be no fee for use however a condition would be to ensure the level of model validation in the area required for testing was improved prior to use. Engagement for that purpose would be by Council at your clients cost, the updated model and all data would be Council's. Once the model is updated and agreed sufficient for use for your purposes, and all costs to achieve the improved level of validation have been paid for, you could then use the model subject to conditions."

Negotiations between GTA Consultants and Shoalhaven City Council took place for the release of the TRACKS model to undertake to the required network modelling. In subsequent correspondence Scott Wells wrote on 24 January 2013:
"...we (Council Traffic Unit) never asked for TRACKS modelling, it was an RMS request for DPI to include in DGRs and this was included in the DGRs without consultation with Council. The only available TRACKS model that covers this area is an AADT model and there has never been specific validation in the area subject of assessment. This means without checking against field data there is no high level confidence in regards to the strategic distributions to/from the site and Princes Highway.... For the purposes of your study use of SIDRA at Princes Highway/Moss street and Princes Highway/Kalandar Street should suffice in my view".

Consequently, assessment of the traffic impact of the proposed development has been undertaken using SIDRA INTERSECTION, a computer based modelling package which calculates intersection performance on an individual intersection basis. Conversely TRACKS software assesses traffic impacts on a network wide scale.

The commonly used measure of intersection performance, as defined by RMS Traffic Modelling Guidelines (2013), is vehicle delay. SIDRA INTERSECTION determines the average delay that vehicles encounter and provides a measure of the level of service. Table 2.5 shows the criteria that SIDRA INTERSECTION adopts in assessing the level of service in line with the RMS Traffic Modelling Guidelines (2013).

[^1]
## EXISTING CONDITIONS

Table 2.5: SIDRA INTERSECTION Level-of-Service Criteria

| Level of Service <br> (LOS) | Average Delay per <br> vehicle (secs/veh) | Traffic Signals, Roundabout | Give Way \& Stop Sign |
| :---: | :---: | :---: | :---: |
| A | Less than 14 | Good operation | Good operation |

Source: Table 14.3 of RMS Traffic Modelling Guidelines (2013)
The road network under consideration as part of this assessment includes the following 11 intersections:

- Culburra Road/ Coonemia Road (priority controlled)
- Culburra Road/ Mayfield Road (priority controlled)
- Greenwell Point Road/ Pyree Lane (priority controlled)
- Greenwell Point Road/ Jindy Andy Lane (priority controlled)
- Greenwell Point Road/ Mayfield Road (priority controlled)
- Greenwell Point Road/ Millbank Road/ Worrigee Road (stop controlled)
- Princes Highway/ Kalandar Street (signalised)
- Coonemia Road/ Currarong Road/ Forest Road (priority controlled)
- Kalandar Street/ Kinghorne Street (roundabout)
- Princes Highway/ Forest Road (priority controlled)
- Princes Highway/ Moss Street (signalised).


### 2.8.1. Base Scenario $-120^{\text {th }}$ Highest Annual Hour

As stated in Section 2.2, traffic volume data was collected in May 2012. Queuing at intersections was also recorded so that the base year model could be validated.

As the NSW South Coast is a popular tourist destination subject to influxes of tourists over long weekends and during school holidays, particularly during the summer school holidays. To reflect this seasonal increase in traffic volumes in the vicinity of the development site, the $120^{\text {th }}$ highest annual hour $(\mathrm{HH})$ has been used as the Design Hourly Volume (DHV) for the base traffic scenario as instructed by Scott Wells of Shoalhaven City Council in correspondence dated 24 January 2013:
"We would also consider the assessment incomplete without undertaking adjustment of the surveyed flows to equivalent $120^{\text {th }} \mathrm{HH}$ demand flow levels consistent with AUSTROADS guidelines".

Use of the $120^{\text {th }} \mathrm{HH}$ as the DHV reflects a peak hour within the highest $1 \%$ of all hourly volumes recorded over a year and as such represents a period of high seasonal traffic volumes.

The traffic counts undertaken as part of this assessment were completed during a period of low tourist activity. Subsequently, the recorded peak hour traffic flows require application of an appropriate growth factor to represent the $120^{\text {th }} \mathrm{HH}$.

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### 2.8.2. Calculation of Growth Factors

The calculation of an appropriate growth factor to be applied to the May 2012 recorded traffic flows was undertaken by the Traffic and Transport Unit of Shoalhaven City Council. The growth factors were calculated by analysing 2008 annual hourly traffic volume data from Greenwell Point Road and Forest Road as well as data from the RMS permanent traffic count stations at Falls Creek, north of Jervis Bay Road (approximately 4km south of Forest Road, count station 07.053). The growth factors to be applied to the recorded May 2012 traffic flows are summarised below with further details of the calculation of growth factors contained in Appendix B:

Table 2.6: Growth Factors to be Applied to May 2012 Recorded Flows to Calculate $120^{\text {th }} \mathrm{HH}$ Flows

| Road | Friday AM Peak Hour <br> $(8-9 \mathrm{am})$ | Friday PM Peak Hour <br> $(4-5 \mathrm{pm})$ | Saturday Peak Hour <br> $(12-1 \mathrm{pm})$ |
| :--- | :---: | :---: | :---: |
| Local Roads \& Traffic to/ from <br> Princes Highway | 1.12 | 1.41 | 1.25 |
| Princes Highway through traffic <br> (north-south movements)* | 1.13 | 1.07 | 1.18 |

[1] * Relates to the Princes Highway intersections with Moss Street, Kalandar Street and Forest Road.
Source: Shoalhaven City Council

Application of the growth factors shown in Table 2.6 to the surveyed May 2012 peak hour volumes and the equivalent $120^{\text {th }} \mathrm{HH}$ traffic flows were used for the base scenario assessments of the Friday AM, Friday PM and Saturday Peak hours.

Analysis of the annual hourly traffic across different years at RMS permanent count station at Princes Highway (south of Illaroo road) is presented in Figure 2.14.

Figure 2.14:Yearly Profile - Princes Highway permanent count station


Source: RMS NSW Traffic Volume Viewer (2020)

The graph indicates that the traffic volumes (especially during the peak season) are generally similar across different years (between 2011 and 2018) and also follow a similar trend with higher traffic during the holiday

## EXISTING CONDITIONS

season. It has therefore been assumed that the above assessment of the DHV is still relevant and has not been re-investigated in detail for the intervening period between 2012 and 2020.

### 2.8.3. Previous Modelling Results

The previous GTA 2013 TIA was carried out with Version 5 of SIDRA INTERSECTION software. Since then, the software has been upgraded with added functionality and algorithms. Upon updating the models to the latest Version 8 of SIDRA INTERSECTION, algorithmic changes to the software has altered some results reported in the GTA 2013 TIA. The Version 5 intersection modelling results for existing traffic conditions have been reproduced in Table 2.7, along with the updated Version 8 results.

Table 2.7: Existing Network Performance -SIDRA INTERSECTION Version 5 and 8 results comparison

| Intersection | Peak | SIDRA INTERSECTION Version 5 Results |  |  |  | SIDRA INTERSECTION Version 8 Results |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Degree of Saturation (DOS) | Delay (sec) | 95th Percentile Queue (m) | Level of Service (LOS) | Degree of Saturation (DOS) | $\begin{aligned} & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | 95th <br> Percentile <br> Queue (m) | Level of Service (LOS) |
| Culburra <br> Road/ <br> Coonemia <br> Road | Friday AM | 0.22 | 7 | 5 | NA | 0.13 | 11 | 4 | A |
|  | Friday PM | 0.15 | 6 | 4 | NA | 0.13 | 11 | 4 | A |
|  | Saturday | 0.14 | 7 | 3 | NA | 0.12 | 11 | 3 | A |
| Culburra <br> Road/ Mayfield Road | Friday AM | 0.19 | 1 | 4 | NA | 0.06 | 12 | 0 | A |
|  | Friday PM | 0.21 | 1 | 11 | NA | 0.01 | 34 | 0 | C |
|  | Saturday | 0.14 | 1 | 7 | NA | 0.01 | 12 | 0 | A |
| Greenwell <br> Point Road/ <br> Pyree Lane | Friday AM | 0.22 | 9 | 7 | NA | 0.23 | 12 | 7 | A |
|  | Friday PM | 0.56 | 10 | 37 | NA | 0.57 | 12 | 38 | A |
|  | Saturday | 0.21 | 8 | 6 | NA | 0.22 | 12 | 6 | A |
| Greenwell <br> Point Road/ <br> Jindy Andy <br> Lane | Friday AM | 0.23 | 4 | 7 | NA | 0.06 | 23 | 2 | B |
|  | Friday PM | 0.22 | 4 | 6 | NA | 0.01 | 16 | 0 | B |
|  | Saturday | 0.18 | 4 | 5 | NA | 0.03 | 16 | 1 | B |
| Greenwell <br> Point Road/ Mayfield Road | Friday AM | 0.20 | 2 | 9 | NA | 0.02 | 13 | 0 | A |
|  | Friday PM | 0.24 | 2 | 21 | NA | 0.03 | 19 | 1 | B |
|  | Saturday | 0.17 | 2 | 15 | NA | 0.01 | 13 | 0 | A |
| Greenwell Point Road/ Millbank Road/ | Friday AM | 0.39 | 8 | 15 | NA | 0.14 | 25 | 4 | B |
|  | Friday PM | 0.24 | 6 | 7 | NA | 0.26 | 27 | 7 | B |

EXISTING CONDITIONS

| Intersection | Peak | SIDRA INTERSECTION Version 5 Results |  |  |  | SIDRA INTERSECTION Version 8 Results |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Degree of Saturation (DOS) | $\begin{aligned} & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | 95th Percentile Queue (m) | Level of Service (LOS) | Degree of Saturation (DOS) | $\begin{aligned} & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | 95th <br> Percentile Queue (m) | Level of Service (LOS) |
| Worrigee Road | Saturday | 0.15 | 6 | 4 | NA | 0.12 | 20 | 3 | B |
| Princes <br> Highway/ <br> Kalandar <br> Street | Friday <br> AM | 1.05 | 86 | 403 | F | 1.04 | 95 | 399 | F |
|  | $\begin{aligned} & \text { Friday } \\ & \text { PM } \end{aligned}$ | 1.10 | 120 | >500 | F | 1.24 | 153 | 107 | F |
|  | Saturday | 0.97 | 63 | 389 | E | 1.11 | 100 | >500 | F |
| Coonemia <br> Road/ <br> Currarong <br> Road/ Forest <br> Road | Friday AM | 0.12 | 12 | 3 | NA | 0.10 | 9 | 3 | A |
|  | $\begin{aligned} & \text { Friday } \\ & \text { PM } \end{aligned}$ | 0.25 | 12 | 8 | NA | 0.26 | 9 | 9 | A |
|  | Saturday | 0.20 | 12 | 6 | NA | 0.21 | 9 | 6 | A |
| Kalandar Street/ <br> Kinghorne Street | Friday AM | 0.73 | 15 | 69 | B | 0.75 | 25 | 74 | B |
|  | $\begin{aligned} & \text { Friday } \\ & \text { PM } \end{aligned}$ | 0.77 | 16 | 78 | B | 0.77 | 20 | 78 | B |
|  | Saturday | 0.37 | 10 | 18 | A | 0.38 | 12 | 18 | A |
| Princes <br> Highway/ Forest Road | Friday <br> AM | 0.74 | 2 | 5 | NA | 0.23 | 15 | 3 | A |
|  | Friday PM | 0.77 | 5 | 17 | NA | 0.79 | 103 | 9 | F |
|  | Saturday | 0.60 | 3 | 13 | NA | 0.70 | 62 | 9 | E |
| Princes Highway/ Moss Street | Friday AM | 1.03 | 89 | 388 | F | 1.01 | 84 | 350 | F |
|  | $\begin{aligned} & \text { Friday } \\ & \text { PM } \end{aligned}$ | 1.24 | 200 | >500 | F | 1.16 | 132 | 472 | F |
|  | Saturday | 0.89 | 50 | 200 | D | 0.92 | 57 | 243 | E |

In general, the $95^{\text {th }}$ percentile queues are comparable between the two versions of SIDRA used in the GTA 2013 TIA and the updated assessment.

The Princes Highway and Forest Road intersection operates as a seagull intersection. The templates for a seagull-type intersection have changed between versions 5 and 8 of SIDRA INTERSECTION. The Princes Highway and Forest Road intersection has been modelled as a network to simulate the seagull operation. The following changes were made to match the intersection performance to observed queues and driver behaviour:

- Gap acceptance parameters were altered for PM peak period at this intersection to match the observed queues.
- A critical gap of 6 seconds instead of default 7 seconds was adopted for the right turn from Forest Road into Princes Highway.
- A follow-up headway of 3.5 seconds, instead of default 4 seconds was adopted for the right turn from Forest Road into Prince Highway.


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On the basis of the above, Version 8 of SIDRA INTERSECTION has been adopted for the discussion of results and all subsequent analysis in this report.

It is noted that previously overall average results were reported for priority intersections. The RMS Traffic Modelling Guidelines (2013) mention that for priority intersections results should be reported for worst movement, and this has been updated moving forward.

### 2.8.4. Existing Intersection Operation

Table 2.7 summarises the intersection operation at all 11 intersections within the network. At priority intersections, the LOS of the worst turning movement has been reported while at signalised intersections an overall average is reported for the intersection.

On the basis of the above assessment, under equivalent $120^{\text {th }} \mathrm{HH}$ traffic volumes:

- Most priority-controlled intersections operate well during the three respective peak periods with minimal delays and queues on all approaches except for Princes Highway and Forest Road intersection
- The existing seagull intersection at Princes Highway and Forest Road experiences high (>100 seconds) delays for the Forest Road (east) approach. SIDRA INTERSECTION is limited in simulating dynamic arrival patterns that can be experienced on Regional Roads of this nature. Nonetheless, the queues experienced at this intersection is less than 20 metres long.
- The existing Princes Highway intersections at Kalandar Street and Moss Street experience long delays and queues during peak periods, particularly during the Friday AM and Friday PM peak hours. The other intersections in the study area currently operate satisfactorily.


### 2.9. Performance of Rural Road Network

GTA Consultants undertook an assessment of the existing local road network surrounding the development site to assess road design aspects (cross-section parameters) for compliance with Austroads Guidelines and Roads and Mairitime Guidelines in relation to:

- lane widths
- rural turning lanes
- intersection configurations
- warrants for overtaking lanes.

As advised in correspondence from Scott Wells dated 19 February 2013, this assessment was to be limited to the local road network surrounding the site as a similar assessment of the State Road network in the vicinity of the site (i.e. the Princes Highway) was not required.

This issue is considered in further detail in Section 8 of this report.

## 3. DEVELOPMENT PROPOSAL



### 3.1. Land Use

The proposed mixed-use subdivision development comprises three precincts as shown in Figure 3.1, referred to as:

- Residential Precinct: Residential area which has an indicative capacity of approximately 293 dwellings on 244 lots ranging from $500 \mathrm{~m}^{2}$ to $1,200 \mathrm{~m}^{2}$.
- Industrial Precinct: Industrial area with 13 industrial lots located adjacent to the existing industrial area.
- Town Centre Precinct: Town centre area with a variety of development products including integrated housing and medium density low-rise apartments, as well as other commercial and recreation-orientated uses.

Figure 3.1: West Culburra Beach Concept Plan - Proposed Staging


Source: Drawing No. 26505 - 213, prepared by Allen Price \& Scarratts Pty Ltd, dated 28/09/2020
A summary of the lot yield is presented in Table 3.1.

DEVELOPMENT PROPOSAL

Table 3.1: Approximate Lot Yield

| Classification | Yield |
| :--- | :--- |
| Residential Precinct: Low Density Housing | 293 dwellings ${ }^{[1]}$ |
| Industrial Precinct: Industrial Lots | 13 lots |
| Town Centre Precinct: Integrated Housing | 45 integrated dwellings |
| Town Centre Precinct: Medium Density | 95 low-rise apartments |
| Town Centre Precinct: Retail/ Residential (Shop top housing) | 3 shop-top mixed residential/ non-residential lots |
| Town Centre Precinct: Sports Ground | $25,600 \mathrm{~m}^{2}$ |

[1] Estimated from 244 residential lots, see Section 1.1
Approximately 36,850 square metres of parkland/ open space is also proposed. These areas are intended to provide amenity and separation for residents of the development and are not expected to generate any traffic on the surrounding road network.

It is anticipated that each development precinct will have an independent access to Culburra Road. The residential precinct proposes a roundabout intersection near the bend of Culburra Road, approximately 1100 metres west of the Culburra/ West Crescent intersection. The industrial precinct proposes an extension to the existing industrial area access, approximately 800 metres west of the Culburra Road/ West Crescent intersection. The town centre precinct proposes a roundabout intersection approximately 375 metres west of the Culburra Road/ West Crescent intersection.

### 3.2. Pedestrian and Bicycle Facilities

An elevated shared path following the foreshore alignment, with focus on retaining access to the foreshore to the north is proposed as part of the updated proposal. In addition, shared paths are assigned for all roads with widths above 20 metres. The proposed shared path plan is shown in Figure 3.2.

As a minimum, a shared path would also be provided on the northern side of Culburra Road, to connect the subdivision with the town centre.

Figure 3.2: Pedestrian and Bicycle Path


Source: Drawing No. 25405-211, prepared by Allen Price \& Scarratts Pty Ltd, dated 05/03/2020
Street patterns are in alignment with NSW Planning Guidelines for Walking and Cycling, as further discussed in Section 5.4.

### 3.3. Parking

The car parking requirements for different development types are contained in Shoalhaven City Council Car Parking Code (SDCP 2014: Chapter G21). It is anticipated that car parking for the development will be provided in accordance with the requirements of SDCP 2014: Chapter G21. The corresponding rates are provided in Table 3.2. For the purposes of analysis, it has been assumed that:

- all integrated/ medium density housing is between 55 metres and 86 metres in land size
- for industrial lots, $70 \%$ of the lot area (developable area) has been assumed to be the GFA.

All development products will have sufficient ability to provide the required car parking, noting on-street parking would be available for any non-residential overflow parking (including the sporting facilities) and visitor parking requirements.

DEVELOPMENT PROPOSAL

Table 3.2: SDCP 2014: Parking Requirements

| Land Use | DCP Rate | Units OR m² GFA | DCP requirement |
| :--- | :---: | :---: | :---: |
| Residential | 2 per dwelling | 293 dwellings | 586 spaces |
| Integrated Housing (between <br> $55 m^{2}$ and $86 m^{2}$ land size) | 1.5 per unit | 45 dwellings | 68 spaces |
| Medium Density | 1.5 per dwelling | 95 dwellings | 143 spaces |
| Shop top housing | 1.5 per dwelling | 3 lots (approx. 24 dwellings) ${ }^{[2]}$ | 36 spaces |
| Commercial/Retail (shop top <br> housing) | 1 per $24 m^{2}$ GLFA | $5,846 m^{2[3]}$ | 244 spaces |
| Industrial (General) | 1 per $100 m^{2}$ GFA | $23,030 m^{2[4]}$ | 230 spaces |
| Sports Oval | 30 spaces per playing field <br> (local playing field) | 1 full-size oval | 30 spaces |

[2] assuming medium density for shop top housing, approx. 8 dwellings per lot
[3] GLFA $=75 \%$ of GFA, and GFA $=80 \%$ of developable lot area
[4] $70 \%$ of the total developable area $\left(32,900 \mathrm{~m}^{2}\right)$

## 4. VEHICLE ACCESS



## VEHICLE ACCESS

### 4.1. Introduction

It is proposed that access to the site is provided via two new roundabouts from Culburra Road to serve the general residential precinct and the town centre precinct. An upgrade of the existing priority-controlled Canal Street East/ Culburra Road/ West Crescent intersection to a roundabout is also proposed at the eastern end of the site.

Access to the industrial precinct is proposed via an extension of Regmoore Close, which is currently accessed via the Strathstone Street/ Culburra Road priority-controlled intersection that provides access to the existing industrial area. It is proposed to close the existing Strathstone Street connection to Culburra Road, with Regmoore Close extended east by approximately 60 metres to form a new priority-controlled intersection with Culburra Road. An auxiliary left turn treatment is proposed for movements into the site from Culburra Road.

Concept designs for the intersections providing access to all three precincts have been prepared by Allen Price \& Scarratts Pty Ltd and are reviewed in the following sections.

### 4.2. Intersection Assessment and Concept Design

### 4.2.1. Roundabout Intersections

As a result of the modified subdivision proposal, three roundabouts are now proposed. The proposal consists of:

1. A roundabout at the Canal Street East/ Culburra Road/ West Crescent intersection, which comprises an upgrade of the existing priority-controlled intersection currently provided at this location
2. A new roundabout (referred to as the 'western roundabout' for the balance of our assessment) located approximately 1 kilometre west of the Canal Street East/ Culburra Road/ West Crescent intersection, and 320 metres west of the existing Strathstone Street / Culburra Road intersection, which will provide access to the residential precinct; and
3. A new roundabout (referred to as the 'eastern roundabout' for the balance of our assessment) located approximately 280 metres west of the Canal Street East/ Culburra Road / West Crescent intersection, which will provide access to the town centre precinct.

GTA has completed an assessment of the proposed intersections, with consideration given to the topography, sight distances and road geometry. Updated concept designs have been prepared by Allen Price \& Scarratts, which are shown individually in Figure 4.1 to Figure 4.3. and as a full drawing set in Appendix E.

## VEHICLE ACCESS

Figure 4.1: Canal Street East/ Culburra Road/ Canal Street/ West Crescent Intersection upgrade


Source: Drawing No. 25405-115, Rev P1 prepared by Allen Price \& Scarratts Pty Ltd, dated 03/2020
The proposed intersection upgrade replaces the existing at-grade priority-controlled intersection and will provide alternative access to the town centre. The roundabout consists of:

- a 10-metre (radius of five metres) diameter non-mountable central island surrounded by a 7 metre (radius of 12 metres) mountable apron
- $\quad$ single lane entries on all approaches
- a layout designed to accommodate a 19-metre-long articulated vehicle based on a Regional Collector Road/ Local Road intersection for all through, left-turn, and right-turn movements along Culburra Road, taking into consideration the industrial land uses located to the west of the proposed roundabout.



## VEHICLE ACCESS

Figure 4.2: Residential Access Preliminary Concept Layout ('Western Roundabout')


Drawing No. 25405-105 Rev P6, prepared by Allen Price \& Scarratts Pty Ltd, dated 03/2020

The proposed 'Western roundabout' is located 320 metres west of the existing Strathstone Street/ Culburra Road intersection and will provide access to the future residential subdivision located to the north of the roundabout.
The roundabout consists of:

- a 10-metre (radius of five metres) diameter non-mountable central island surrounded by a 7 metre (radius of 12 metres) mountable apron
- $\quad$ single lane entries on the east and west approaches and a two lane entry on the north approach
- a layout designed to accommodate a 19-metre-long articulated vehicle based on a Regional Collector Road/ Local Road intersection for all through, left-turn, and right-turn movements along Culburra Road, taking into consideration the industrial land uses located to the east of the proposed roundabout (it is noted that the layout drawings prepared by Allen Price \& Scarratts indicate that the proposed layout can also accommodate the through movements of a 25 m B-Double in each direction along Culburra Road).



## VEHICLE ACCESS

Figure 4.3: Town Centre Access Preliminary Concept Layout ('Eastern Roundabout')


Source: Drawing No. 25405-112 Rev P1, prepared by Allen Price \& Scarratts Pty Ltd, dated 03/2020

The proposed 'Eastern roundabout' is located 280 metres west of the Canal Street East/ Culburra Road/ West Crescent intersection and will provide access to the future town centre subdivision located to the north of the roundabout. The roundabout consists of:

- a 10-metre (radius of five metres) diameter non-mountable central island surrounded by a 7 metre (radius of 12 metres) mountable apron
- $\quad$ single lane entries on all approaches
- a layout designed to accommodate a 19-metre-long articulated vehicle based on a Regional Collector Road/ Local Road intersection for all through, left-turn, and right-turn movements along Culburra Road, taking into consideration the industrial land uses located to the west of the proposed roundabout.

GTA has conducted a review of all three roundabout designs in accordance with the Austroads Guide to Road Design Part 4A (2017) (AGRD-4A) and Part 4B (2015) (AGRD-4B) and further details on some of the design aspects are discussed in the following sections.

## Approach Sight Distance (ASD) Assessment

To review the viability of a roundabout, the approach sight distance (ASD) must be calculated (Criterion 1 of AGRD-4B). This is the minimum level of sight distance which must be available on the minor road approaches to all intersections to ensure that drivers are aware of the presence of an intersection. It is also desirable on the major road approaches and should be achieved where practicable.

Reference is made to Table 3.1 of AGRD-4A which provides the ASD values for cars. For the operating ( $85^{\text {th }}$ percentile) speed, V , a speed of $90 \mathrm{~km} / \mathrm{h}$ has been adopted for Culburra Road for the eastern and western approaches to the western roundabout and $60 \mathrm{~km} / \mathrm{h}$ has been adopted for Culburra Road for the eastern and western approaches to the other roundabouts further to the east. An operating speed of $60 \mathrm{~km} / \mathrm{h}$ has been adopted for the other connecting collector roads at each intersection. See Section 4.3 for the rationale based on the proposed speed zoning changes.

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A reaction time, $R_{T}$, of 2.0 seconds has been adopted for operating speeds of $60 \mathrm{~km} / \mathrm{h}$ and 2.5 seconds for operating speeds of $90 \mathrm{~km} / \mathrm{h}$, as per the most conservative values in Table 3.1 of AGRD-4A. This approach is more conservative than outlined in the TfNSW (previously RMS) Supplement to Austroads Guide to Road Design Part 3 (2016), which adopts a reaction time of 1.5 seconds for all operating speeds $90 \mathrm{~km} / \mathrm{h}$ and lower.

The adopted coefficient of deceleration, $d$, is 0.36 for cars. This aligns with default AGRD-4A values and guidance in the TfNSW Supplement. For trucks which will also use each leg of these intersections, a value of 0.29 has been adopted for 'd', based on the default value used to assess truck stopping sight distances as per Table 5.6 in Austroads Guide to Road Design Part 3 (2020) (AGRD-3).

The longitudinal grade, a, is presumed to be $0 \%$.
Adopting the parameters above, and referencing Table 3.1 of AGRD-4A for cars and Table 5.6 of AGRD-3 for trucks, the required ASD on each leg at each intersection is as follows (note - first value is for cars; second value shown in brackets is for trucks):

- Western Roundabout

```
O Culburra Road (west) = 151 metres (172 metres)
O Culburra Road (east)=151 metres (172 metres)
- Road 01 (north) = 73 metres (82 metres).
```

- Eastern Roundabout
- Culburra Road (west) $=73$ metres ( 82 metres)
- Culburra Road (east) $=73$ metres ( 82 metres)
- Road 08 (north) $=73$ metres ( 82 metres).
- Culburra Road/ Canal Street/ West Crescent Roundabout
- Culburra Road (west) $=73$ metres ( 82 metres)
- Prince Edward Avenue (east) $=73$ metres ( 82 metres)
- Canal Street E (north) $=73$ metres ( 82 metres)
- West Crescent (south) $=73$ metres ( 82 metres).

The above requirements are illustrated on the concept plans prepared by Allen Price \& Scarratts (Appendix E), which have adopted the values associated with trucks as the worst case due to their longer stopping distances.

It is important to note that the ASD requirement will need to be checked against the vertical alignment and any adjacent obstructions to visibility (e.g. fencing, retaining walls or landscaping) as the design development progresses, to ensure that the combination of vertical and horizontal geometry meets the minimum distances calculated above.

## Minimum Gap Sight Distance (MGSD) Assessment

In addition to meeting the ASD requirements, each roundabout also needs to accommodate the Minimum Gap Sight Distance, MGSD, requirements in accordance with Criterion 2 of AGRD-4B.

The following parameters have been adopted based on the design speed for each of the approaches to the roundabout:

- Western Roundabout
- Speed of vehicles from the approach to the right = 50km/h (Culburra Road)/ 30km/h (Road 01) (Section 3.2.2 of AGRD-4B)
- Circulating speed $=30 \mathrm{~km} / \mathrm{h}$ (Figure 3.1 of AGRD-4B for arterial roads)


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- Eastern Roundabout
- Speed of vehicles from the approach to the right $=30 \mathrm{~km} / \mathrm{h}$ (Section 3.2.2 of AGRD-4B)
- Circulating speed $=20 \mathrm{~km} / \mathrm{h}$ (Figure 3.1 of AGRD-4B for local roads).
- Culburra Road/ Canal Street/ West Crescent Roundabout
- $\quad$ Speed of vehicles from the approach to the right $=30 \mathrm{~km} / \mathrm{h}$ (Section 3.2.2 of AGRD-4B)
- Circulating speed $=20 \mathrm{~km} / \mathrm{h}$ (Figure 3.1 of AGRD-4B for local roads).

Therefore, the MGSD that should be provided based on the above assumptions and Table 3.1 of AGRD-4B is:

- Western Roundabout (critical acceptance gap of 5 seconds for arterial road roundabout)
- MGSD to approach to the right $=70$ metres (to Culburra Road)/ 42 metres (to Road 01)
- MGSD to circulating vehicles $=42$ metres.
- Eastern Roundabout (critical acceptance gap of 4 seconds for local road roundabout)
- MGSD to approach to the right $=33$ metres
- MGSD to circulating vehicles $=22$ metres.
- Culburra Road/ Canal Street/ West Crescent Roundabout
- MGSD to approach to the right $=33$ metres
- MGSD to circulating vehicles $=22$ metres.

These MGSD values have been illustrated on the concept plans prepared by Allen Price \& Scarratts (Appendix E) for each of the proposed roundabouts to outline areas where sight lines need to be maintained. A mark-up of Figure 3.1 of AGRD-4B has been replicated in Figure 4.4 to illustrate the various MGSD measurements.

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Figure 4.4: Sight distance criteria for roundabouts, adopted from AGRD-4B, Figure 3.1


Source: AGRD-4B

## General

SDCP 2014 Chapter G11 requires the subdivision road network to connect with the external road network in a manner which maximises movement efficiency for all traffic routes. A roundabout will offer the most effective means of managing traffic at the intersection, minimising the average delay on all approaches.

### 4.2.2. Industrial Precinct Access

The proposed priority access to the industrial development is presented in Figure 4.5. It is proposed that the current Strathstone Street connection to Culburra Road will be closed, with access to Strathstone Street provided via Regmoore Close. Regmoore Close will be extended approximately 60 metres east to form a new priority-controlled intersection with Culburra Road. A 'Rural Auxiliary Left-turn Lane Treatment' (AUL) lane of approximately 141 metres is proposed to cater for access into the industrial precinct.

A review of the proposed AUL treatment, based on a design speed of $90 \mathrm{~km} / \mathrm{h}$ (refer section 4.3 for proposed speed zoning changes), indicates that the design exceeds the minimum diverge/ deceleration length of 125

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metres (based on stop condition at a comfortable deceleration of 2.5 metres per second) required in accordance with Table 5.2 of AGRD-4A. The proposed design of the AUL lane is therefore acceptable.

Figure 4.5: Proposed Access to Industrial Area


Source: Drawing No. 25405 - 109, prepared by Allen Price \& Scarratts Pty Ltd, dated 03/2020

## Sight Distance

For this access, similar ASD values to those identified for the applicable roundabouts (i.e. based on an operating speed of $90 \mathrm{~km} / \mathrm{h}$ along Culburra Road and $60 \mathrm{~km} / \mathrm{h}$ on Regmoore Close) are required to be accommodated by the design. Accordingly, the ASD to be provided on each of the approaches is as follows (note - first value is for cars; second value shown in brackets is for trucks):

- Culburra Road (west) $=151$ metres ( 172 metres)
- Culburra Road (east) = 151 metres ( 172 metres)
- $\quad$ Regmoore Close (north) $=73$ metres ( 82 metres).

The above requirements are illustrated on the concept plans prepared by Allen Price \& Scarratts (Appendix E), which have adopted the values associated with trucks as the worst case due to their longer stopping distances.

In addition to the above, Safe Intersection Sight Distance (SISD) must be achieved at this intersection. This is the minimum level of sight distance which should be provided on the major road at this type of intersection. While it utilises similar parameters to those previously discussed in relation to ASD in section 4.2.1, it is measured in a different way as described in AGRD-4A.

Based on a design speed, V , of $90 \mathrm{~km} / \mathrm{h}$; a coefficient of deceleration, d , of 0.36 ; and a Reaction Time, $\mathrm{R}_{\mathrm{T}}$, of 2.5 s ; the SISD required to be provided in each direction along Culburra Road from Regmoore Close (as per Figure 3.2 and Table 3.2 of AGRD-4A) is 226 m . This is the requirement for cars, with the equivalent value for trucks (based on a value for d of 0.29 as discussed previously) being 248 m . The SISD requirements for trucks are also shown on the concept drawings prepared by Allen Price \& Scarrats based on this advice.

Both the ASD requirements and also the SISD requirements will need to be checked against the vertical alignment and any adjacent obstructions to visibility (e.g. fencing, retaining walls or landscaping) as the design development progresses, to ensure that the combination of vertical and horizontal geometry meets the minimum distances calculated above.

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## Intersection Layout

Swept path analysis prepared for this intersection has identified the need for kerb modifications and line marking adjustments to accommodate simultaneous left-turn entry and exit and also right-turn entry and exit for two 20mlong articulated vehicles. This analysis is reproduced in Figure 4.6 and Figure 4.7. It is recommended that these modifications are considered further as part of the detailed design development, along with the potential need for westbound widening along Culburra Road to accommodate a Basic Right Turn (BAR) treatment (replicating the existing intersection treatment).

Figure 4.6: Left turns in/ out of Industrial Precinct for 20m articulated vehicles


Figure 4.7: Right turns in/ out Industrial Precinct for 20 m articulated vehicles


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### 4.2.3. Summary of Design Review

A general design review has been undertaken for the intersections outlined above against Part 4A and Part 4B of the Austroads Guide to Road Design. It is noted that:

- The design is generally compliant with the relevant sections of the corresponding Austroads Guides, noting the potential kerb modifications required at the Industrial Precinct access as discussed above.
- Comments in relation to the specific sight distance requirements that need to be satisfied (as part of detailed design) at each intersection have been provided in the preceding sections.
- Swept path analysis has been prepared by Allen Price \& Scarrats at each intersection, based on a 19-metre semi-trailer being able to perform all through, left-turn and right-turn manoeuvres without mounting of the central island (in the case of the roundabouts). This is the largest vehicle expected to service the majority of the development.
- $\quad$ Swept path assessments for a 25 m B-Double have also been prepared by Allen Price \& Scarrats at the Western roundabout (through movements only in each direction along Culburra Road) and at the industrial precinct access (left-in from Culburra Road and right-out onto Culburra Road only).
- The results are presented in Appendix E and demonstrate that the proposed designs are generally able to satisfactorily accommodate the turning requirements of these vehicles. The one exception is for the BDouble movements at the industrial access, where it is noted that the current design can only accommodate movement in one direction at a time. This is considered to be acceptable for the infrequent number of movements that will be made by these vehicles. However, minor changes to accommodate the simultaneous movement of semi-trailers at this intersection are recommended as discussed in section 4.2.2.


### 4.2.4. Road Widths

To determine the suitability of the proposed road widths, reference is made to the Shoalhaven City Council Engineering Design Specifications (February 1999). This document outlines, in Table D1.13, the minimum and maximum carriageway widths depending on the road hierarchy for residential subdivisions. An extract of this table is provided in Table 4.1.

Table 4.1: Road hierarchy and design widths (extract from D1.13 of Shoalhaven City Council Engineering Design Specifications)

| Road Type | Maximum traffic volume <br> $(\mathrm{vpd})$ | Minimum Carriageway <br> Width (metres) | Maximum Carriageway <br> Width (metres) |
| :--- | :---: | :---: | :---: |
| Access Place | $<150$ | 3.5 | 3.7 |
| Access Street (minor) | 500 | 5.0 | 6.0 |
| Local Street (up to <br> 1000vpd) | 1000 | 7.0 | 7.5 |
| Local Street | 2000 | 7.0 | 9.0 |
| Collector Street | 3000 | 7.0 | 9.0 |
| Local Distributor Road | $3000-6000$ | 9.0 | 11.0 |

From the preliminary concept drawings, the following roads and road widths are identified and summarised in Table 4.2.

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Table 4.2: Road Widths

| Location | Road number | Width (metres) | Adopted Classification | Alignment with Engineering Design Specifications |
| :---: | :---: | :---: | :---: | :---: |
| Residential Precinct | Road 01 (north of roundabout ${ }^{3}$ ) | 8 | Local Street | Yes |
|  | Road 01 (south of roundabout) | 11 | Local Distributor Road | Yes |
|  | Road 02 | 8 | Collector Street | Yes |
|  | Road 03 | 6 | Local Street | Yes, (recommend widening to 7 metres) |
|  | Road 04 | 6 | Access Street (minor) | Yes |
|  | Road 05 | 6 | Access Street (minor) | Yes |
|  | Road 06 | 6 | Access Street (minor) | Yes |
| Industrial Precinct | Road 07 | 10 | Note 1 | Note 1 |
| Town Centre Precinct | Road 08 | 7 | Collector Street | Yes |
|  | Road 09 | 7 | Local Street | Yes |
|  | Road 10 | 7 | Collector Street | Yes |
|  | Access Road 01 | 7 | Local Street | Yes |

Note 1: As the Engineering Design Specifications document outlines road widths for residential subdivisions, application of the guidance to the road for the industrial precinct is unsuitable. However, the proposed 10 metre road width is generally considered appropriate for the surrounding land use and larger design vehicles. It is recommended that a swept path assessment be prepared for 19 -metre articulated vehicles to validate the suitability of the provided road width, particularly at the 90 -degree bends.

Although Road 03 complies with the specifications, the location and connectivity could result in Road 03 functioning as a local road rather than an access street. Therefore, it is recommended that the road is widened by one metre or local area traffic management applied to reduce the likelihood of use by residents in adjacent streets. This can be addressed at the detailed design stage.

On this basis, the assessment presented in the above table confirms that all of the proposed roads comply with Council's Engineering Design Specifications document and are therefore acceptable.

### 4.2.5. Bus Access

A bus route is proposed within the residential precinct, with access into the precinct via Road 01 and operating as a clockwise loop via Road 02 which travels along the perimeter of the precinct. Road 02 has a road width of 8 metres (4 metre lanes each way) which is generally appropriate in catering for bus movements. However, it is recommended that a swept path assessment for buses is prepared to confirm the suitability of the road width at the bends.

Three bus bays are proposed along Road 02, each measuring 30 metres in length inclusive of the draw-in and draw-out tapers. This aligns with the standard bus stop length of 30 metres as provided in Section 3.7 of the NSW Government State Transit Bus Infrastructure Guide Issue 2 (2011). This is appropriate, assuming that standard 12.5-metre-long buses are used for the precinct.

[^2]
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It is assumed, given the residential nature of the precinct, that the bus bays only need to accommodate a single bus at a time.

### 4.2.6. Emergency Vehicle Access

The NSW Rural Fire Service Planning for Bush Fire Protection (2019) outlines minimum access requirements for fire service vehicles.

The minimum carriageway width for the design fire truck is outlined in Table 4.3.
Table 4.3: Extract from Planning for Bush Fire Protection (Table A3.2)

| Curve radius (inside edge in metres) | Swept path (metres) |
| :--- | :---: |
| $<40$ | 4.0 |
| $40-69$ | 3.0 |
| $70-100$ | 2.7 |
| $>100$ | 2.5 |

The emergency egress routes provided to the industrial precinct are 4.0 metres wide. This is considered appropriate as it caters for the Rural Fire Truck swept path which requires a width of up to 4.0 metres depending on the curvature and alignment of the road.

Turning circles are also proposed at the end of the cul-de-sacs in the residential and town centre precincts. These are nominated as 24 metres wide in diameter which aligns with a Type A turning head as per Figure A3.3 of the Planning for Bush Fire Protection guideline. However, it is noted that the width of the roadway in the design is restricted to approximately 18.5 metres only. During design development, it is recommended that a 24 -metre diameter manoeuvring space is provided (e.g. with a mountable section or allowance for overhang or similar) to accommodate a single-turn manoeuvre for the Rural Fire Truck. Departures from a standard 24-metre diameter cul-de-sac should be consulted with NSW RFS to ascertain the suitability of the design.

### 4.3. Speed Zonings

A 100 kilometres per hour speed zone is currently in place on Culburra Road, which commences approximately 360 metres west of the Canal Street East/ Culburra Road/ West Crescent intersection. Figure 4.8 shows the existing speed zoning in the vicinity of the development site.

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Figure 4.8: Culburra Road - Existing Speed Zoning


Background image source: NearMap (captured 2019)
Roads and Maritime Services (RMS) is responsible for all permanent speed zoning in NSW regardless of the classification of the road. RMS was consulted for the previous GTA 2013 TIA regarding the proposed speed changes, however, they did not support a reduction in the Culburra Road speed zone and considered the existing 100 kilometres per hour speed zone on this section of Culburra Road to be appropriate.

Based on the revised access arrangements and considering the overall development, it is considered that a reduction of the speed limit to 80 kilometres per hour between the $50 \mathrm{~km} / \mathrm{h}$ speed zone of the town centre, and extending to a point approximately 200 metres west of the proposed new 'western roundabout' at the western end of the development site, is appropriate and will offer improved safety outcomes. It is also noted that an existing advisory speed sign of 75 kilometres per hour is placed just before the bend at the western end for eastbound traffic and the natural topography is expected to reduce driver speed.

With the extension of the existing Culburra Beach urban footprint to the west as part of the proposed development, it is also appropriate to adjust the current interface between the existing 100 kilometres per hour (rural) speed zone and the 50 kilometres per hour (urban) speed zone. Noting that the second proposed roundabout ('eastern roundabout') is at the approximate location of the current change in speed zone, the following is recommended:

- Reduce the 100 kilometres per hour speed limit to 80 kilometres per hour prior to the proposed residential access roundabout ('western roundabout'), in conjunction with a new urban entry treatment. This would significantly improve road safety outcomes for turning movements at the three proposed development access points.
- Reduce the above 80 kilometres per hour speed limit to 50 kilometres per hour prior to the proposed town centre roundabout ('eastern roundabout'). This represents a minor change to the extent of the existing 50 kilometres per hour speed zone extent.
- Undertake streetscape improvements between the two roundabouts (in conjunction with the relevant development stage adjacent) in order to provide appropriate visual cues to drivers that they have entered the Culburra Beach urban area.


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Figure 4.9 depicts these proposed changes.
Figure 4.9: Culburra Road - Proposed Speed Zoning


Background Image Source: NearMap

### 4.4. Bicycle Parking \& Associated Facilities

Bicycle parking for the site is governed by Australian Standard and Austroads guidelines as referenced in Section 5.11 P19 of Chapter G21 of the SDCP 2014. Neither provides rates for bicycle parking and end-of-trip facilities, hence reference is made to the NSW Planning Guidelines for Walking and Cycling 2004. An empirical assessment indicates that bicycle parking and end-of-trip facilities are not specifically required for the residential uses, however, bicycle parking and end-of-trip facilities should be provided for:

- employees of the industrial lots (3-5 percent for staff)
- employees of the commercial land uses (3-5 percent for staff)
- $\quad$ visitors, customers and patrons of the commercial land uses (5-10 percent for visitors).


## 5. SUSTAINABLE TRANSPORT INFRASTRUCTURE



## SUSTAINABLE TRANSPORT INFRASTRUCTURE

### 5.1. Policy and Planning Guidelines

### 5.1.1. Future Transport 2056

Future Transport 2056 outlines transportation goals for the whole of NSW, to be realised in 2056. The six key outcomes as outlined in the plan are:

- Customer experiences are seamless, interactive and personalised, supported by technology and data.
- The liveability, amenity and economic success of communities and places are enhanced by transport.
- The transport system powers NSW's future $\$ 1.3$ trillion economy and enables economic activity across the state.
- Every customer enjoys safe travel across a high performing, efficient network.
- Transport enables everyone to get the most out of life, wherever they live and whatever their age, ability or personal circumstances.
- The transport system is economically and environmentally sustainable, affordable for customers and supports emissions reduction.

Notable in the documentation is the creation of streetscapes depending on related land use - considering the residential and local centre usage of the site, the building of local streets and places for people, wherein cars travel slowly and may not have access to certain streets, would be encouraged.

## Regional NSW Services Infrastructure Plan

Regional NSW Services Infrastructure Plan is an in-depth attachment to the Future Transport 2056 documentation. The plan outlines a strategic investigation to increase access from inland NSW to Newcastle/ Sydney/ Wollongong. In the Illawarra - Shoalhaven region, the expansion of the 30-minute catchment for public transport will seek to improve the site's public transport access to its regional centre.

Existing transport networks involve connection regional centres to Sydney; however, future strategies involve the 'hub-and-spoke' model, involving inter-connection between regional centres. Moreover, the goal to shift mode share away from car usage suggests that better public transport can be expected near the site. In relation to design, the consideration of cycling and walking infrastructure will help in aligning the development with State Government objectives.

### 5.1.2. Integrating Land Use and Transport (2001)

Although partially superseded by Future Transport 2056, the NSW Department of Urban Affairs and Planning's Integrating Land Use and Transport (2001) policy package provided guidelines for planning and development which aim to encourage development that:

- increases access to public transport, walking and cycling
- encourages people to travel shorter distances and make fewer trips
- reduces car dependency.

The aim of integrating land use and transport is to ensure that urban structures, buildings for RMS, land use locations, development designs, subdivisions and street layouts achieve the following planning objectives:

- improving access to housing, jobs and services by walking, cycling and public transport
- increasing the choice of available transport and reducing dependence on cars
- reducing travel demand including the number of trips generated by development and the distances travelled, especially by car
- supporting the efficient and viable operation of public transport services.


## SUSTAINABLE TRANSPORT INFRASTRUCTURE

Integrating Land Use and Transport identifies the following key transport planning concepts which recognise people's basic travel needs:

- Convenience - the transport mode needs to be easy to find and use, and to transfer from one mode to another.
- Information - reliable information at accessible locations is essential to encourage use of various travel alternatives.
- Proximity - transport facilities and services, such as cycle paths and bus services, need to be in close, convenient and obvious locations to people's trip origins and destinations.
- Destination choice - the more destinations that can be linked on a public transport route, the more attractive it will be.
- Directness - routes should take the shortest and least deviating course, with priority to achieve fast travel times for walking, cycling and public transport (e.g. pedestrian links, dedicated bus lanes, and bikeways).
- Security - the environment for walking and waiting needs to be comfortable and safe from personal attack or conflicts with traffic (e.g. waiting areas sheltered from the elements, natural surveillance, good lighting, bike lanes on major roads).


## NSW Walking and Cycling Program Guidelines (2004)

The NSW Planning Guidelines for Walking and Cycling (2004) aims to assist land-use planners and related professionals to improve consideration of walking and cycling. The guidelines have been designed to provide a walking and cycling focus to the NSW Government's Integrating Land Use \& Transport Planning policy package.

### 5.2. Future Bus Routes

As discussed in Section 2.4, the site is serviced by 10 bus services per day, with no weekend or public holiday services.

Noting the existing policy and planning guidelines relevant to the site, it is anticipated that the Culburra Beach to Nowra bus services will be more frequent to accommodate new developments at the site, surrounding areas and the village centre of Orient Point. Moreover, alterations to the route may be implemented, critically to cover access to the village centre.

### 5.2.1. Consultation with Bus Operator

Shoalhaven Integrated Transport Strategy highlights the importance of early consultation with bus operators to ensure early provision of bus services in new residential developments which offers advantages for:

- the vendor - the availability of public transport can be a strong selling point
- the bus operator - if new families move into a new development and have a service available immediately it is likely that the operator will have regular patrons. Once a family has to purchase a second car it is unlikely that the family will use the bus again.

For the previous GTA 2013 TIA, Kennedy's Bus and Coach bus service was consulted as they are the provider of the Culburra Beach-Nowra service. Initial consultation indicated that the operator welcomes additional patronage within their normal bus operations and would be happy to extend their current service arrangements.

The operator highlighted the road geometry requirements for the development to allow for a minimum 12.5 to 13.5 metre long bus and noted that consideration needs to be given to the provision of disabled access for low floor wheelchair buses which will be compulsory on all route services by 2020.

Correspondence with Kennedy's Bus and Coach bus service is contained in Appendix F.

## SUSTAINABLE TRANSPORT INFRASTRUCTURE

### 5.3. Bus Stops

Integrating Land Use and Transport outlines that public transport stops should be designed and managed to provide the following:

- good pedestrian access from surrounding areas, including direct, safe and well-lit street connections or pedestrian links, safe pedestrian crossings and clear lines of sight to the stop
- safe, well-lit and comfortable waiting areas with shelter and information on available services
- direct and convenient connections from the footpath to the shelter/ waiting area and from the shelter/ waiting area to the doors of the public transport vehicle, and vice versa
- clear identification of the public transport nodes and access points by attractive design and signage
- access for all users, including appropriate provision for people with disabilities
- bus stops with adequate lighting, shelter and passive security.

SDCP 2014 Chapter G11, P48 contains the requirements for the location and design of bus stops within subdivision developments:

- public transport stops provide for pedestrian safety, security, comfort and convenience
- bus stops are designed to prevent vehicles from overtaking a stationary bus, or vehicle speeds are reduced to ensure safe pedestrian crossing
- bus stops are located and designed to provide shelter, seats, adequate lighting and timetable information, are overlooked from nearby buildings, and are located to minimise adverse impact on the amenity of nearby dwellings.

SDCP 2014 Chapter G11, P47 suggests the following measures are considered as part of the development:

- routes for regular bus services are designed for a minimum pavement width of 9.0 metres
- bus stops are, or are planned for 400 m spacings where the route serves residential development
- the siting of bus stops is related to the pedestrian path network.

The location of bus stops is crucial to ensure ease of access for residents and in turn encourage the use of sustainable transport.

It is recommended that all new bus stops provide the following as a minimum:

- shelter
- seating
- lighting
- timetable information.

An indicative location plan of new bus stops is shown in Figure 5.1, subject to further discussion with TfNSW, the bus operator and other relevant stakeholders.

## SUSTAINABLE TRANSPORT

 INFRASTRUCTUREFigure 5.1: Indicative Bus Stop Location


Source: Drawing No. 25405 - 220, prepared by Allen Price \& Scarratts Pty Ltd, updated 30/09/2020

### 5.4. Walking and Cycling Network

### 5.4.1. Street Pattern

Although most strategic consideration has been covered by Future Transport 2056, The NSW Planning Guidelines for Walking and Cycling highlights the importance of street pattern as a determinant of walkability and cyclability. Street pattern determines how far a person can travel by foot or by bicycle within a set timeframe, as well as the feel of a neighbourhood from a pedestrian's or cyclist's perspective. Local streets such as those proposed within residential, industrial and town centre precincts should be highly interconnected with many junctions onto main road making walking and cycling trips short and direct. In contrast, layouts with unconnected cul-de-sacs make walking and cycling trips longer and less interesting as sight lines are limited and there are few (if any) alternative route options available for any one trip as shown in Figure 5.2.

## SUSTAINABLE TRANSPORT INFRASTRUCTURE

Figure 5.2: Street Patterns and Accessibility

The 'perfect' street pattern for direct access to centres and public transport stops (Ref 5.10)


A more organic street pattern for direct access to centres and public transport stops (Ref 5.10)

 showing long walk from A to B and only one route available

Modified grid design showing shorter walk from $\mid A$ to $B$ and several routes available


Source: NSW Planning Guidelines for Walking and Cycling
The indicative street layout for the development shown in Figure 3.2 has many similarities to the 'perfect' street pattern shown in Figure 5.2, offering many options for access through the development. This through site permeability is vital for encouraging walking and cycling for short trips within these precincts and to key destinations.

### 5.4.2. Key Destinations

The key walking and cycling destinations in proximity of development areas are likely to be:

- Culburra Beach shops - east of the development area
- Crookhaven River foreshore reserve - north of the development area
- public reserves throughout the proposed development
- beaches - east of the established urban area of Culburra.

Providing direct and easy walking and cycling access to these locations via dedicated, high quality facilities is imperative to promote walking. The indicative street layout shown in Figure 3.1 has numerous streets aligned in a north-south direction enabling direct walking access to the Crookhaven River foreshore reserves from within the development, as well as a dedicated boardwalk for access through the foreshore. The proposed public reserves shown as green in Figure 3.1 provide further options for direct walking and cycling access to the foreshore area, separated from vehicle traffic.

### 5.4.3. Walking and Cycling Infrastructure

The Shoalhaven Integrated Transport Strategy lists the following as a critical implementation action:

- Action 4 - Build a network of cycleways and footpaths which link schools, shops, employment areas, bus interchanges and also link outlying villages to Park and Ride interchanges along the trunk corridor.

The foreshore route offers an excellent opportunity as a recreational walking and cycling route and to promote cycle tourism. This route is proposed to be a shared path. The connection of all routes to the established areas of Culburra Beach and in particular to the Culburra Beach shops is crucial in discouraging private car use for short trips within the area. The connection of this east-west connector to the existing footpath network requires further consideration. It is recommended that the walking and cycling network within the established areas of Culburra Beach be upgraded to provide a consistent standard of facility and in particular to improve access to the beaches, east of the town.

The provision of dedicated, high quality facilities adjacent to all proposed collector roads are imperative to improve accessibility within the development area. Proposed shared path layout plan is presented in Figure 3.2 .

## SUSTAINABLE TRANSPORT INFRASTRUCTURE

### 5.4.4. Design Requirements

SDCP 2014 Chapter G11 outlines the design requirements for walking and cycling infrastructure within subdivision developments. A minimum of a 1.2-metre-wide footpath is required on local and collector streets within a subdivision, seen in A38.2 of the DCP. As such it is anticipated that footpaths will be provided on both sides of all local streets within the subdivision at a minimum 1.2 metres wide.

The design requirements for bicycle and pedestrian facilities are included in the NSW Bicycle Guidelines and Austroads Part 6A: Paths for Walking and Cycling (2017). The width requirements for shared paths are summarised in Table 5.1. Bicycle facilities included as part of the development are proposed to be shared paths rather than separated paths given the moderate cyclist volumes expected and consistency with other facilities in the broader area.

Table 5.1: Shared Path Widths

|  | Suggested path width (m) |  |  |
| :---: | :---: | :---: | :---: |
|  | Local access path | Regional path |  |
| Desirable minimum width | 2.5 | 3.0 | Recreational path |
| Minimum width <br> maximum | $2.0^{(1)}-3.0^{(2)}$ | $2.5^{(1)}-4.0^{(2)}$ | 3.5 |

1 A lesser width should only to be adopted where cyclist volumes and operational speeds will remain low.
2 A greater width may be required where the numbers of cyclists and pedestrians are very high or there is a high probability of conflict between users (e.g. people walking dogs, in-line skaters etc.).
3 May be part of a principal bicycle network in some jurisdictions.

Source: Austroads Part 6A: Paths for Walking and Cycling 2017

### 5.5. Sustainable Transport Infrastructure Summary

- The development offers the opportunity to provide improved public transport services to all residents of Culburra Beach and Orient Point through higher frequency weekday bus services and the provision of weekend and public holiday services.
- The early provision of bus services for the development areas is considered paramount in promoting public transport usage. Initial consultation with the bus operator indicates that the operator welcomes additional patronage within their normal bus operations and would be happy to extend their current service arrangements.
- The indicative street layout offers many options for access through the development area, providing vital through site permeability. The street layout is consistent with the aims of the NSW Planning Guidelines for Walking and Cycling to promote walking and cycling, particularly for short trips.
- A minimum of a 1.2-metre-wide footpath is required on local and collector streets within a subdivision in-line with SDCP 2014 Chapter G11
- Future proposed cycleways will be conducive to promoting pedestrian and cyclist access, particularly for short trips, in accordance with Future Transport 2056 (2018) and Integrating Land Use and Transport (2001) principles.
- For the shared path designated for the foreshore, it is recommended to provide a minimum 3 metre width given this intended to have broader regional connectivity.
- For the recommended shared path adjacent to Culburra Road, a minimum 2.5 metre width should be adopted.


## 6. LOADING \& WASTE COLLECTION RESPONSE/ ASSESSMENT



## LOADING \& WASTE COLLECTION RESPONSE/ ASSESSMENT

### 6.1. Loading and Servicing Facilities

It is understood that refuse collection for the residential and mixed-use areas will involve kerbside collection by a 12.5-metre-long Council garbage vehicle, equivalent to a Heavy Rigid Vehicle (HRV). The proposed $20-25$ metre road reserve widths would generally be able to accommodate kerbside parking.

For the industrial precinct, internal road design should allow for a minimum 20-metre-long articulated vehicle (AV), typically expected for industrial land uses.

For the town centre precinct of the development, largest design vehicle that is expected is a 12.5 -metre-long Council garbage vehicle/ HRV. Kerbside waste collection is proposed and should be sufficient for servicing townhouses, commercial and retail purposes. Detailed loading design elements would be addressed at the subsequent Development Application stage.

All access intersections from Culburra Road have been designed to accommodate vehicles at least the dimension of a 12.5-metre-long heavy rigid vehicle, as discussed in Section 0

## 7. TRAFFIC IMPACT ASSESSMENT



## TRAFFIC IMPACT ASSESSMENT

### 7.1. Traffic Generation

### 7.1.1. Design Rates

Traffic generation estimates for the proposed development would usually be sourced from the Guide to Traffic Generating Developments (Roads and Maritime, 2002) and its addition, TDT 2013/4a, as well as previously determined area-specific rates (12S1231000 West Culburra Beach Subdivision Development Transport and Accessibility Impact Assessment Issue E). Estimates of peak hour and daily traffic volumes using this guide are set out in Table 7.1. For the purposes of trip generation, it has been assumed that:

- For mixed use, housing component has been assumed to be medium density, therefore based on this approximately 24 dwelling (over 3 lots) have been assumed.
- For the commercial/retail component of mixed use (shop top), the GFA has been assumed to be $80 \%$ of the developable lot area.
- For the industrial component, the GFA has been assumed to be $70 \%$ of the total developable lot area

Table 7.1: Estimated Development Traffic Generation (RMS Rates)

| Precinct | Land Use | Design Generation Rates |  | Traffic Generation Estimates (vehicles) |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Peak Hour | Daily | Peak Hour | Daily |
| Residential Precinct | Residential (293 x detached houses) | $0.71-0.78$ vehicle movements/ dwelling | 7.4 vehicle movements/ dwelling | 208-229 | 2168 |
| Industrial Precinct | Industrial (approx. 23,030m² GFA) ${ }^{[8]}$ | 0.39-1.3 vehicle movements/ $100 \mathrm{~m}^{2}$ GFA | $\begin{gathered} 3.78-11.99 \\ \text { vehicle } \\ \text { movements/ } \\ 100 \mathrm{~m}^{2} \text { GFA } \end{gathered}$ | 90-299 | 871-2761 |
| Town Centre Precinct | Residential <br> (45 x townhouses (assumed)) | $0.5-0.65$ vehicle movements/ dwelling | $5.0-6.5$ vehicle movements/ dwelling | 23-29 | 225-293 |
|  | Residential <br> (95 x apartments (assumed)) | $0.4-0.5$ vehicle movements/ dwelling | 4.0-5.0 vehicle movements/ dwelling | 38-48 | 380-475 |
|  | Residential <br> (3 lots x shop top housing, 24 dwellings) ${ }^{[6]}$ | $0.4-0.5$ vehicle movements/ dwelling | 4.0-5.0 vehicle movements/ dwelling | 10-12 | 100-120 |
|  | Retail <br> (approx. $5,846 \mathrm{~m}^{2}$ GLFA) ${ }^{[7]}$ | 20 vehicle movements/ $1000 \mathrm{~m}^{2}$ GLFA | 314 vehicle movements/ $1000 m^{2}$ GLFA | $88^{[9]}$ | 1377 |
|  | Non-Residential (Sports facility) | N/A | N/A | N/A | N/A |
| Total |  |  |  | 457-705 vehicle movements/ hour |  |

[5] Estimated rate based on a peak-daily ratio of $10 \%$.
[6] assuming medium density for shop top housing.
[7] GLFA $=75 \%$ of GFA, and GFA $=80 \%$ of developable lot area for retail
[8] $70 \%$ of the total developable area $\left(32,900 \mathrm{~m}^{2}\right)$ for industrial lots
[9] Applied discount factor of $25 \%$ to account for internal trips

## TRAFFIC IMPACT ASSESSMENT

Both the retail and sports facility developments are expected to primarily serve internal trips. Therefore, these values are not included in the impact assessment as all trips would be contained in the subdivision local road network.

### 7.1.2. Empirical Traffic Generation Leaving the Culburra Beach Area

The Traffic and Transport Unit of Shoalhaven City Council previously provided empirical traffic generation rates for the established area of Culburra Beach. The trip rates were calculated by using traffic volume data from 2008 annual hourly counts on Greenwell Point Road and Forest Road and residential occupancy data from the 2011 census to determine the number of vehicle trips entering the regional road network (west of Culburra Beach) per occupied dwelling in the relevant peak hours. Table 7.2 summarises the empirical traffic generation rates.

Table 7.2: Empirical Traffic Generation Rates (Shoalhaven City Council)

| Peak Hour Scenario | Traffic Generation Rate |
| :--- | :---: |
| Friday AM | 0.22 |
| (vehicles per occupied dwelling per peak hour) |  |

Source: Shoalhaven City Council (Appendix B)
As shown in Table 7.2, based on analysis of the existing traffic generating characteristics of the established urban area of Culburra Beach, it is anticipated that the proposed development would generate 0.22, 0.21 and 0.23 vehicle trips per dwelling during the respective Friday AM, Friday PM and Saturday peak hours. As advised by Shoalhaven City Council these rates are based on detached dwellings and reductions could be justified for semi-detached or non-detached dwellings.

### 7.1.3. Applied Rates

## Residential

Application of the empirical traffic generation rates provided by Shoalhaven City Council to the proposed development results in an estimated traffic generation estimates as summarised in Table 7.3.

Table 7.3: Estimated Development Traffic Generation (Shoalhaven City Council Empirical Traffic Generation Rates)

| Peak Hour Scenario | Traffic Generation Rate <br> (Shoalhaven City Council) | Proposed Residential <br> Dwellings (Maximum) | Traffic Generation <br> Estimates (vehicles) |
| :--- | :---: | :---: | :---: |
| Friday AM | 0.22 |  | 96 |
| Friday PM | 0.21 | $436^{[10]}$ | 92 |
| Saturday | 0.23 |  | 100 |

[10] 293 detached houses, 45 integrated housing, 95 (medium density) low rise apartments, 3 shop top housing
As shown in Table 7.3, based on the adoption of the traffic generation rates provided by Shoalhaven City Council the proposed development is expected to generate 101, 96 and 105 vehicle trips during the respective Friday AM, Friday PM and Saturday peak hours on the regional road network (west of Culburra Beach).

It is noted that approximately one third of the residential dwellings proposed as part of the development are either semi- detached or non-detached (164 of the 457 total dwellings proposed), likely to house fewer residents than low-density, detached housing. As such the traffic generation estimates contained in Table 7.3 represents a

## TRAFFIC IMPACT ASSESSMENT

conservative estimate of the traffic that is likely to be generated by the development (i.e. greater than what could be expected).

Given the existing traffic generating characteristics of the established area of Culburra Beach on the regional road network (west of Culburra Beach) the adoption of the empirical traffic generation estimate is considered appropriate.

## Non-Residential

In the absence of Council-provided empirical data for non-residential developments, the rates outlined in the Guide to Traffic Generating Developments' TDT 2014/04a (RMS, 2013) have been adopted for the industrial development. The shop-top retail development is expected to generate internal trips only. In summary:

- 74 industrial trips are expected to be generated in the Friday AM period
- 90 industrial trips are expected to be generated in the Friday PM period
- no industrial trips are expected for the Saturday peak, as most industrial plants are expected to be closed in the weekend.

Table 7.4: Estimated Non-Residential Development Traffic Generation

| Peak Hour Scenario | Traffic Generation Rate <br> (Roads and Maritime) | Proposed GFA $\left(\mathrm{m}^{2}\right)$ | Traffic Generation <br> Estimates (vehicles) |
| :--- | :---: | :---: | :---: |
| Friday AM | 0.32 |  | 74 |
| Friday PM | 0.39 | $23,030 \mathrm{~m}^{2}$ | 90 |

### 7.2. Distribution and Assignment

The directional distribution and assignment of traffic generated by the proposed development will be influenced by a number of factors, including the:

- configuration of the arterial road network in the immediate vicinity of the site
- existing operation of intersections providing access between the local and arterial road network
- distribution of households in the vicinity of the site
- surrounding employment centres, retail centres and schools in relation to the site
- configuration of access points to the site.

The distribution and assignment of traffic generated by the proposed development has been informed by the following:

- analysis 2016 Census Journey to Work (JTW) Data
- analysis of the May 2012 Traffic Count Data
- consultation with Shoalhaven City Council's Traffic and Transport Unit.


### 7.2.1. 2016 Census Journey to Work Data

It is noted that the existing site is a greenfield, and the SA4 statistical area which it is zoned in does not have existing residential developments. Thus, no journey to work data is available for the site (SA1-1127415). West of the site, development exists with residential housing as the predominant zoning and infrastructure (SA1 1127406 and SA1 - 1127416). Considering the geographical and land use similarity, the nearby zones have been analysed to obtain an understanding of the travel patterns.

GTA Consultants undertook analysis of all trips made by the JTW mode of 'Car, as Driver', which represented $76 \%$ of all journeys to work in the travel zone. Destinations of these trips were grouped into four broad

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geographical categories; north, south, east and west. There were also a number of trips that were classified as 'Unknown', 'Sydney Undefined', 'NSW Undefined' or 'No Fixed Address' in the data. These results were not statistically significant and have been excluded from the calculation.

Modal distribution has been determined in Table 7.5 and the trip destinations summarized in Table 7.6. Table 7.7 outlines where workers arrive from, if working in the Culburra Beach region.

Table 7.5: Modal Distribution/ Method of Travel to Work (Journey to Work 2016)

| SA1 (UR) | 1127416 | 1127406 | Total | Mode Share (\%) |
| :--- | :---: | :---: | :---: | :---: |
| Car, as driver | 141 | 92 | 230 | $76 \%$ |
| Did not go to work | 22 | 9 | 30 | $10 \%$ |
| Car, as passenger | 7 | 3 | 18 | $6 \%$ |
| Not stated | 5 | 0 | 5 | $2 \%$ |
| Walked only | 4 | 9 | 11 | $4 \%$ |
| Worked at home | 4 | 0 | 5 | $2 \%$ |
| Other | 0 | 0 | 3 | $1 \%$ |
| Total | 183 | 113 | 302 | $100 \%$ |

Table 7.6: Destination Nodes (Journey to Work 2016)

| SA4 | SA3 | SA2 | 1127416 | 1127406 | Values |
| :--- | :--- | :--- | :---: | :---: | :---: |
| Illawarra | TOTAL |  | 0 | 7 | 4 |
| Capital Region | TOTAL |  | 0 | 0 | 5 |
| Southern Highlands and <br> Shoalhaven | TOTAL |  | 151 | 102 | 254 |
|  | Shoalhaven | TOTAL | 151 | 102 | 254 |
|  |  | Berry | 0 | 3 | 3 |
|  |  | Callala Bay | 6 | 3 | 9 |
|  |  | Culburra Beach | 51 | 32 | 83 |
|  |  | Huskisson | 4 | 4 | 8 |
|  |  | North Nowra | 11 | 9 | 20 |
|  |  | Nowra | 151 | 102 | 127 |
|  |  |  | 0 | 0 | 0 |
| Sydney (Amalgamated) | TOTAL |  |  | 10 | 3 |

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Table 7.7: Origin Nodes (Journey to Work 2016)

| SA4 | SA3 | SA2 | 1127416 | 1127406 | Values |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Illawarra | TOTAL |  | 13 | 4 | 17 |
| Southern Highlands and Shoalhaven | TOTAL |  | 488 | 263 | 752 |
|  | Shoalhaven | TOTAL | 487 | 263 | 745 |
|  |  | Berry | 7 | 3 | 10 |
|  |  | Callala Bay | 57 | 7 | 64 |
|  |  | Culburra Beach | 323 | 170 | 490 |
|  |  | Huskisson | 6 | 0 | 6 |
|  |  | North Nowra | 27 | 28 | 52 |
|  |  | Nowra | 39 | 39 | 78 |
|  |  | St Georges | 16 | 4 | 20 |
|  |  | Sussex Inlet | 4 | 0 | 4 |
|  |  | Tomerong | 4 | 7 | 11 |
|  |  | Ulladulla | 3 | 0 | 8 |
|  | Southern Highlands | TOTAL | 3 | 0 | 3 |
| Sydney (Amalgamated) | TOTAL |  | 13 | 3 | 16 |

The directional distribution as calculated from JTW data is presented in Figure 7.1 and Figure 7.2.

Figure 7.1: Culburra Beach Residence, Going to Work - Directional Distribution


Figure 7.2: Culburra Beach Non-residential, Place of Work- Directional Distribution


Basemap source: Google Maps (accessed February 2020)

### 7.2.2. May 2012 Traffic Count Data

The May 2012 traffic count data was analysed to determine the distribution of vehicles into and out of the road network under consideration. This was determined by calculating the percentage of vehicles entering or exiting this network during the respective peak hours based on the May 2012 intersection traffic counts. The six entry/exit points to the network are shown graphically in Figure 7.3 and includes the following intersections:

- North - Princes Highway/ Moss Street
- West - Princes Highway/ Moss Street, Kalandar Street/ Kinghorne Street
- South - Princes Highway/ Forest Road
- East - Greenwell Point/ Pyree, Culburra Road/ Coonemia Road and the Coonemia Road/ Currarong Road.


## TRAFFIC IMPACT ASSESSMENT

Figure 7.3: Directional Distribution Analysis - Road Network Entry/Exit Locations


Background Image Source: Google Maps
While this area does not represent a 'closed' network, whereby there are other entry and exit points to the road network, the analysis was undertaken to provide an indication of the existing directional distribution of vehicles into and out of the road network under consideration. The existing directional distribution of vehicles into and out of this road network during the Friday AM, Friday PM and Saturday peak hours is summarised in Table 7.8, Table 7.9 and Table 7.10 respectively.

Table 7.8: Existing Directional Distribution - Friday AM Peak Hour (May 2012 Traffic Counts)

| Direction | Entry/Exit Location | Outbound |  | Inbound |  |
| :--- | :--- | :--- | :--- | :--- | :--- |
| West | Moss Street | 518 | $19 \%$ | 260 | $7 \%$ |
|  | Kinghorne Street | 417 | $15 \%$ | 405 | $10 \%$ |
| North | North of Moss Street | 1085 | $40 \%$ | 1680 | $42 \%$ |
|  | South of Forest Road | 468 | $17 \%$ | 1211 | $30 \%$ |
| East | Culburra Road | 143 | $6 \%$ | 225 | $6 \%$ |
|  | Currarong Road | 22 | $1 \%$ | 45 | $1 \%$ |
|  | Greenwell Point Road | 66 | $2 \%$ | 142 | $4 \%$ |
|  |  | Total | 2719 | $100 \%$ | 3968 |

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Table 7.9: Existing Directional Distribution - Friday PM Peak Hour (May 2012 Traffic Counts)

| Direction | Entry/Exit Location | Outbound |  | Inbound |  |
| :--- | :--- | :--- | :--- | :--- | :--- |
| West | Moss Street | 383 | $10 \%$ | 597 | $17 \%$ |
|  | Kinghorne Street | 388 | $10 \%$ | 432 | $13 \%$ |
| North | North of Moss Street | 1489 | $37 \%$ | 1580 | $46 \%$ |
| South | South of Forest Road | 1342 | $33 \%$ | 600 | $17 \%$ |
| East | Culburra Road | 239 | $6 \%$ | 127 | $4 \%$ |
|  | Currarong Road | 48 | $1 \%$ | 20 | $1 \%$ |
|  | Greenwell Point Road | 137 | $3 \%$ | 64 | $2 \%$ |

Table 7.10: Existing Directional Distribution - Saturday Peak Hour (May 2012 Traffic Counts)

| Direction | Entry/Exit Location | Outbound |  | Inbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| West | Moss Street | 266 | 8\% | 431 | 14\% |
|  | Kinghorne Street | 269 | 8\% | 299 | 9\% |
| North | North of Moss Street | 1287 | 40\% | 1332 | 42\% |
| South | South of Forest Road | 962 | 30\% | 683 | 22\% |
| East | Culburra Road | 176 | 6\% | 190 | 6\% |
|  | Currarong Road | 36 | 1\% | 28 | 1\% |
|  | Greenwell Point Road | 207 | 7\% | 186 | 6\% |
| Total |  | 3203 | 100\% | 3149 | 100\% |

### 7.2.3. Consultation with Shoalhaven City Council

Shoalhaven City Council's Traffic and Transport Unit was previously consulted extensively in relation to the directional distribution of traffic generated by the proposed development in an effort to gain an understanding of known local traffic patterns and key trip generators. In addition to the empirical traffic generation rates shown in Table 7.3, Council's Traffic and Transport Unit provided directional splits for traffic generated by the development in the relevant peak hours as shown in Table 7.11.

Table 7.11: Empirical Traffic Generation Rates and $120^{\text {th }}$ HH Directional Splits (Shoalhaven City Council)

| Peak Hour Scenario | Traffic Generation Rate <br> (Shoalhaven City Council) | Directional Split $-120^{\text {th }} \mathrm{HH}$ <br> Outbound (westbound) | Inbound (eastbound) |
| :--- | :---: | :---: | :---: |
| Friday AM | 0.22 | $76 \%$ | $24 \%$ |
| Friday PM | 0.21 | $25 \%$ | $75 \%$ |
| Saturday | 0.23 | $50 \%$ | $50 \%$ |

[^3]On the basis of the above, the directional distribution of traffic generated by the residential development on the road network west of Culburra Beach during the Friday AM, Friday PM and Saturday peak hours are summarised in Table 7.12, Table 7.13 and Table 7.14 respectively (the numbers in brackets in the 'Outbound' and 'Inbound' columns represent the corresponding number of vehicles).

The directional split presented in Table 7.11 is valid for residential development. Regarding commercial/ retail and industrial lots, the peak direction is anticipated to be inbound for the AM peak and outbound for the PM peak. Therefore, for the industrial lots and the town centre the directional splits is assumed as follows:

- Friday $\mathrm{AM}-75 \%$ inbound and $25 \%$ outbound
- Friday PM - $24 \%$ inbound and $76 \%$ outbound.

This distribution is also shown graphically in Figure 7.4, Figure 7.5 and Figure 7.6 for AM, PM and Saturday peaks respectively while the total and development volumes are presented in Figure 7.7, Figure 7.8 and Figure 7.9 for the AM, PM and Saturday peaks respectively.

Figure 7.4: Directional Distribution - Resi Friday AM Peak Hour, Non-Resi Friday PM Peak Hour


Figure 7.5: Directional Distribution - Resi Friday PM Peak Hour, Non-Resi Friday AM Peak Hour


Figure 7.6: Directional Distribution - Saturday Peak Hour


Figure 7.7: Total and Development Volumes - Friday AM Peak Hour


Figure 7.8: Total and Development Volumes - Friday PM Peak Hour


Figure 7.9: Total and Development Volumes - Saturday Peak Hour


Table 7.12, Table 7.13 and Table 7.14 present the estimated increase in turning movements on the surrounding road network following full site development. Percentages are shown with traffic volumes presented in brackets.

Table 7.12: Proposed Directional Distribution - Friday AM Peak Hour

| Direction | Route | Residential Outbound | Residential Inbound | Nonresidential Outbound | Non-residential Inbound |
| :---: | :---: | :---: | :---: | :---: | :---: |
| West | West of Princes Highway (via Jindy Andy Lane) | 10\% (9) | 2\% (2) | 2\% (1) | 9\% (5) |
|  | West of Princes Highway (via Millbank Road) |  |  |  |  |
|  | West of Princes Highway (via Kalandar Street) | 11\% (10) | 2\% (2) | 3\% (2) | 11\% (6) |
| North | North of the Shoalhaven River (via Jindy Andy Lane) | 11\% (10) | 3\% (3) | 2\% (1) | 10\% (5) |
|  | North of the Shoalhaven River (via Millbank Road) |  |  |  |  |
|  | North of the Shoalhaven River (via Kalandar Street) | 16\% (15) | 5\% (5) | 2\% (1) | 24\% (14) |
| South | South of Forest Road | 8\% (7) | 6\% (5) | 7\% (4) | 8\% (5) |
| East | East of Princes Highway (via Jindy Andy Lane) | 2\% (2) | 0\% (0) | 0\% (0) | 0\% (0) |
|  | East of Princes Highway (via Millbank Road) |  |  |  |  |
|  | East of Princes Highway (via Kalandar Street) | 13\% (12) | 2\% (2) | 3\% (2) | 3\% (2) |
|  | East of Princes Highway (via Worrigee Road or Old Southern Road) | 2\% (2) | 2\% (2) | 2\% (1) | 2\% (1) |
|  | East of Princes Highway (via Forest Road) | 3\% (3) | 2\% (2) | 3\% (2) | 8\% (5) |
|  | Total | 76\% (69) | 24\% (22) | 25\% (14) | 75\% (43) |

Table 7.13:Proposed Directional Distribution - Friday PM Peak Hour

| Direction | Route | Residential Outbound | Residential Inbound | Nonresidential Outbound | Nonresidential Inbound |
| :---: | :---: | :---: | :---: | :---: | :---: |
| West | West of Princes Highway (via Jindy Andy Lane) | 2\% (2) | 9\% (8) | 10\% (6) | 2\% (1) |
|  | West of Princes Highway (via Millbank Road) |  |  |  |  |
|  | West of Princes Highway (via Kalandar Street) | 3\% (3) | 11\% (10) | 11\% (6) | 2\% (1) |
| North | North of the Shoalhaven River (via Jindy Andy Lane) | 2\% (2) | 10\% (9) | 11\% (6) | 3\% (2) |
|  | North of the Shoalhaven River (via Millbank Road) |  |  |  |  |
|  | North of the Shoalhaven River (via Kalandar Street) | 2\% (2) | 24\% (21) | 16\% (9) | 5\% (3) |
| South | South of Forest Road | 7\% (6) | 8\% (7) | 8\% (5) | 6\% (4) |
| East | East of Princes Highway (via Jindy Andy Lane) | 1\% (1) | 0\% (0) | 2\% (1) | 0\% (0) |
|  | East of Princes Highway (via Millbank Road) |  |  |  |  |
|  | East of Princes Highway (via Kalandar Street) | 3\% (3) | 3\% (3) | 13\% (8) | 2\% (1) |
|  | East of Princes Highway (via Worrigee Road or Old Southern Road) | 2\% (2) | 2\% (2) | 2\% (1) | 2\% (1) |
|  | East of Princes Highway (via Forest Road) | 3\% (3) | 8\% (7) | 3\% (2) | 2\% (1) |
|  | Total | 25\% (22) | 75\% (65) | 76\% (44) | 24\% (14) |

Table 7.14: Proposed Directional Distribution - Saturday Peak Hour

| Direction | Route | Residential <br> Outbound | Residential <br> Inbound | Non- <br> residential <br> Outbound | Non- <br> residential <br> Inbound |
| :--- | :--- | :---: | :---: | :---: | :---: |
| West of Princes Highway (via Jindy Andy <br> Lane) | $3 \%(3)$ | $6 \%(6)$ | $3 \%(1)$ | $6 \%(3)$ |  |
| West of Princes Highway (via Millbank <br> Road) |  |  |  |  |  |
| West of Princes Highway (via Kalandar <br> Street) | $5 \%(5)$ | $3 \%(3)$ | $5 \%(2)$ | $3 \%(1)$ |  |
|  | North of the Shoalhaven River (via Jindy <br> Andy Lane) |  | $5 \%(5)$ | $4 \%(4)$ | $5 \%(2)$ |

### 7.3. Traffic Impact

The West Culburra Beach development will be the major source of growth in the Culburra Beach area over the next 10 years. It is anticipated that the development will be completed in stages, with full site development reached approx. 8 years after commencement.

An assessment of the impacts that the anticipated development traffic would have on the surrounding road network can be made by comparing intersection performance prior to and following full site development.

The proposed development is anticipated to generate an additional 174, 186 and 105 vehicle movements (two-way) on the road network west of Culburra Beach during the respective Friday AM, Friday PM and Saturday peak hours.

Table 7.15 presents a summary of intersection operating conditions following full site development while full results are contained in Appendix C.

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Table 7.15: Future Operating Conditions (Equivalent 120th HH plus Development Traffic)

| Intersection |  | SIDRA Version 8 Results |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Degree of Saturation (DOS) | Delay (sec) | 95th Percentile Queue (m) | Level of Service (LOS) |
| Culburra Road/ Coonemia Road | Friday AM | 0.19 | 13 | 6 | A |
|  | Friday PM | 0.19 | 13 | 5 | A |
|  | Saturday | 0.17 | 12 | 5 | A |
| Culburra Road/ Mayfield Road | Friday AM | 0.01 | 15 | 0 | B |
|  | Friday PM | 0.01 | 48 | 0 | D |
|  | Saturday | 0.01 | 15 | 0 | B |
| Greenwell Point Road/ Pyree Lane | Friday AM | 0.36 | 15 | 14 | B |
|  | Friday PM | 0.69 | 15 | 62 | B |
|  | Saturday | 0.31 | 14 | 10 | A |
| Greenwell Point Road/ Jindy Andy Lane | Friday AM | 0.25 | 25 | 7 | B |
|  | Friday PM | 0.01 | 19 | 0 | B |
|  | Saturday | 0.03 | 18 | 1 | B |
| Greenwell Point Road/ Mayfield Road | Friday AM | 0.02 | 15 | 1 | B |
|  | Friday PM | 0.04 | 21 | 1 | B |
|  | Saturday | 0.01 | 15 | 0 | A |
| Greenwell Point Road/ <br> Millbank Road/ <br> Worrigee Road | Friday AM | 0.16 | 28 | 4 | B |
|  | Friday PM | 0.30 | 31 | 9 | C |
|  | Saturday | 0.13 | 21 | 4 | B |
| Princes Highway/ Kalandar Street | Friday AM | 1.06 | 103 | 457 | F |
|  | Friday PM | 1.25 | 170 | $>500$ | F |
|  | Saturday | 1.11 | 105 | >500 | F |
| Coonemia Road/ Currarong Road/ Forest Road | Friday AM | 0.12 | 9 | 4 | A |
|  | Friday PM | 0.28 | 9 | 10 | A |
|  | Saturday | 0.24 | 9 | 8 | A |
| Kalandar Street/ Kinghorne Street | Friday AM | 0.76 | 26 | 77 | B |
|  | Friday PM | 0.78 | 20 | 81 | B |
|  | Saturday | 0.38 | 12 | 19 | A |
| Princes Highway/ <br> Forest Road | Friday AM | 0.25 | 15 | 3 | B |
|  | Friday PM | 1.00 | 183 | 18 | F |
|  | Saturday | 0.79 | 75 | 11 | F |
| Princes Highway/ Moss Street | Friday AM | 1.07 | 115 | 423 | F |
|  | Friday PM | 1.21 | 123 | 394 | F |
|  | Saturday | 0.96 | 55 | 230 | D |

On the basis of the above assessment, under equivalent $120^{\text {th }} \mathrm{HH}$ traffic volumes with the addition of traffic generated by the development:

- the local priority-controlled intersections operate satisfactorily, with minimal delays and queues on all approaches during the three respective peak periods
- with the additional development traffic, the intersections along Princes Highway at Kalandar Street, Forest Road and Moss Street will continue to operate at poor levels (LOS F) particularly during the Friday AM and Friday PM peak periods.


### 7.3.1. Princes Highway Signalised Intersections

Table 7.16 provides a summary of the increase in traffic volumes from development traffic at the key Princes Highway signalised intersections of Kalandar Street and Moss Street.

Table 7.16: Signalised Intersection Traffic Volume Comparison

| Intersection | Existing Equivalent 120th HH <br> Traffic Volumes through Intersection <br> (vehicles) |  |  | Development Traffic Increase through <br> Intersection |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Friday AM | Friday PM | Saturday | Friday AM | Friday PM | Saturday |
| Princes Highway/ <br> Kalandar Street | 3,575 | 4,473 | 4,089 | $68(2 \%)$ | $65(1 \%)$ | $38(1 \%)$ |
| Princes Highway/ <br> Moss Street | 3,690 | 4,246 | 3,495 | $74(2 \%)$ | $73(2 \%)$ | $51(1 \%)$ |

As shown in Table 7.16, the addition of development traffic at the Princes Highway intersections of Kalandar Street and Moss Street represents only a marginal increase in the total volume of traffic travelling through the intersections following full site development. During the three peak hours examined, a maximum increase of $2 \%$ on existing traffic volumes is anticipated. It should also be noted that proposed East Nowra Sub-Arterial (Section 2.3.2) Road is expected to alleviate congestion at Greenwell Point Road by providing alternative route to Nowra and North -western Suburbs. At this stage, it is not clear what upgrades are proposed along Princes Highway as part of the Nowra-Bomaderry Structure Plan, however the plan is endorsed by the NSW Government. In reference to this, any mitigation works required at intersections along Princes Highway have not been tested as part of scope of this project and it is anticipated that these intersections will be upgraded as part of the Structure Plan.


### 7.3.2. Proposed Access at Culburra Road

At this concept stage, the layout of the proposed roundabout accesses to the residential and town centre precincts and the priority-controlled access to the industrial development are not finalised. The proposed development is anticipated to generate less than 100 trips within any peak hour. From the existing conditions assessment (Section 2.8) it is expected that that the local network (with the exception of Princes Highway) will operate at good level of service (LoS A) indicating there is room to accommodate additional traffic.

As such, modelling of the proposed new accesses along Culburra Road has not been carried out, noting the proposed roundabouts have significant throughput capacity. However, given the low traffic volumes during typical road network peak hours, it is anticipated that the proposed accesses will operate at satisfactory levels with the full development traffic.

### 7.3.3. Internal Road Layout

The proposed internal road layout follows a typical pattern that is also generally consistent with the local area. The minimum road reserve width is 20 metres for local roads and 25 metres for the central collector road and perimeter road of the residential subdivision. These easily accommodate a 9-12-metre-wide carriageway and appropriate intersection arrangements for buses, waste collection vehicles and emergency vehicles.

Internal roundabouts within the residential precincts appropriately manage the key four-way intersections, minimise potential vehicle conflicts and reduce travel speeds.

The industrial area has a proposed 25 -metre-wide road reserve which would satisfactorily accommodate the anticipated heavy vehicle activity with appropriate parking restrictions around curves.

On the basis of the above, it is expected that the internal road network will operate satisfactorily, with minimal queuing and/or delays.

## 8. RURAL ROAD ASSESSMENT



## RURAL ROAD ASSESSMENT

As part of the assessment, Shoalhaven City Council previously requested that GTA Consultants consider the following:

- Austroads cross-section warrants based on existing road characteristics.
- Annual Average Daily Traffic (AADT) of study roads to understand the expected impact on an average day.
- Peak Seasonal Daily Traffic (PSDT) to understand the expected impact during peak seasonal times of the year such as school holidays and across the summer period. This is based on the $120^{\text {th }}$ highest hour.
- Warrants for overtaking lanes on roads in the study area.

GTA Consultants has responded to each assessment criteria as requested (noting references to updated guidance where necessary), which are set out in the following sections.

### 8.1. Austroads Cross-Section Warrants

Guidance on single lane rural road cross-sections have been sourced from Austroads Guide to Road Design Part 3: Geometric Design Table 4.5 which is reproduced in Figure 8.1.

Figure 8.1: Austroads Table 4.5: Single carriageway rural road widths (m)

| Element | Design AADT |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1-150 | 150-500 | 500-1000 | 1000-3000 | > 3000 |
| Traffic lanes ${ }^{(1)}$ | $\begin{gathered} 3.7 \\ (1 \times 3.7) \end{gathered}$ | $\begin{gathered} 6.2 \\ (2 \times 3.1) \end{gathered}$ | $\begin{gathered} 6.2-7.0 \\ (2 \times 3.1 / 3.5) \end{gathered}$ | $\begin{gathered} 7.0 \\ (2 \times 3.5) \end{gathered}$ | $\begin{gathered} 7.0 \\ (2 \times 3.5) \end{gathered}$ |
| Total shoulder | 2.5 | 1.5 | 1.5 | 2.0 | 2.5 |
| Minimum shoulder seal (2).(3).(4),(5).(6) | 0 | 0.5 | 0.5 | 1.0 | 1.5 |
| Total carriageway | 8.7 | 9.2 | 9.2-10.0 | 11.0 | 12.0 |

1 Traffic lane widths include centrelines but are exclusive of edge-lines.
2 Where significant numbers of cyclists use the roadway, consideration should be given to fully sealing the shoulders. Suggest use of a maximum size 10 mm seal within a 20 km radius of towns.
3 Wider shoulder seals may be appropriate depending on requirements for maintenance costs, soil and climatic conditions or to accommodate the tracked width requirements for Large Combination Vehicles.
4 Short lengths of wider shoulder seal or lay-bys to be provided at suitable locations to provide for discretionary stops.
5 Full width shoulder seals may be appropriate adjacent to safety barriers and on the high side of superelevation.
6 A minimum 7.0 m seal should be provided on designated heavy vehicle routes (or where the AADT contains more than 15\% heavy vehicles).

Source: Austroads Guide to Road Design Part 3 (2016)
GTA Consultants has reviewed each of the study area roads identified in Figure 8.2 based on information from aerial photography (Nearmap, Six Maps, Google), Google Street view and our previous site inspections of the study area, to categorise each section of road according to Figure 8.1. The results for rural sections of roads are summarised in Table 8.1 with the urban roads summarised in Table 8.2.

A rural road was considered any road without a formal kerb and an urban road was considered as any of those roads with a formal kerb passing through residential areas.


## RURAL ROAD ASSESSMENT

Table 8.1: Summary of Existing Rural Road Characteristics

| Road | From | To | Urban / <br> Rural | Speed Limit | Traffic Lane Nidth (m) | Average Sealed Shoulder (N/W) | Average Sealed Shoulder (S/E) | Average Gravel Shoulder (N/W) | Average Gravel Shoulder (S/E) | Total Carriageway Width | Rural Design AADT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Forest Road | Coonemia Road | Callala Beach Road | Rural | 80 | 6 | 0 | 0 | 0.5 | 0.5 | 7 | 150 to 500 |
| Forest Road | Callala Beach Road | East Stump Road | Rural | 80/90/100 | 6 | 1 | 1 | 0 | 0 | 8 | 150 to 500 |
| Forest Road | East Stump Road | Chestnut Road | Rural | 100 | 6 | 0.5 | 0.5 | 1 | 1 | 9 | 150 to 500 |
| Forest Road | Chestnut Road | Manuka Road | Rural | 100 | 6 | 1 | 1 | 0.5 | 0.5 | 9 | 150 to 500 |
| Forest Road | Manuka Road | Gimlet Road | Rural | 100/90 | 6 | 1 | 1 | 1 | 1 | 10 | 150 to 500 |
| Forest Road | Gimlet Road | Vineyard Road | Rural | 90 | 6 | 1 | 1 | 1 | 1 | 10 | 150 to 500 |
| Forest Road | Vineyard Road | Western Road | Rural | 90 | 6 | 1 | 1 | 1 | 1 | 10 | 150 to 500 |
| Forest Road | Western Road | Princes Highway | Rural | 60 | 6 | 0 | 0 | 0 | 0 | 6 | 150 to 500 |
| Comerong Island Road | Jindy Andy Lane | 90 degree left turn | Rural | 60 | 6 | 0 | 0 | 0 | 0 | 6 | 150 to 500 |
| Comerong Island Road | 90 degree left turn | Milbank Road | Rural | 60/80 | 6 | 0 | 0 | 0 | 0 | 6 | 150 to 500 |
| Terara Road | Milbank Road | Nondalga Crescent | Rural | 50/60 | 6 | 0 | 0 | 0 | 0 | 6 | 150 to 500 |
| Greenwell Point Road | West Street | Pyree Lane | Rural | 100/80/50 | 5.4 | 0 to 0.3 | 0 to 0.3 | 0 | 0 | 5.4 to 6 | 150 to 500 |
| Greenwell Point Road | Pyree Lane | Jindy Andy Lane | Rural | 80 | 6.4 | 0.3 to 0.5 | 0.3 to 0.5 | 0 | 0 | 7 to 8.4 | 500 to 1000 |
| Greenwell Point Road | Jindy Andy Lane | Apperleys Lane | Rural | 80 | 6.2 | 0 to 0.3 | 0 to 0.3 | 0 | 0 | 6.2 to 6.8 | 150 to 500 |
| Greenwell Point Road | Apperleys Lane | Worrigee Road | Rural | 60/80 | 7 | 0.5 to 1 | 0.5 to 1 | 0 | 0 | 8 to 9 | 1000 to 3000 |
| Greenwell Point Road | Worrigee Road | Old Southern Road | Rural | 60 | 6.2 | 0 to 3m | 0 to 2 m | 0 | 0 | 6.2 to 11.2 | 500 to 1000 |
| Greenwell Point Road | Old Southern Road | Clipper Road | Rural | 60 | 6.7 | 0.3 | 0.3 | 0 | 0 | 7.3 | 150 to 500 |

## RURAL ROAD ASSESSMENT

Table 8.2: Summary of Existing Urban Road Characteristics

| Road | From | To | Urban / <br> Rural | Speed Limit | Traffic Lane Width (m) | Average Sealed Shoulder (NM) | Average Sealed Shoulder (S/E) | Average Gravel Shoulder (NW) | Average Gravel Shoulder (S/E) | Total Carriageway Width | Rural Design AADT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Moss Street | Wondalga Crescent | Princes Highway | Urban | 50 | 12 | Kerb | Kerb | N/A | N/A | 12 | N/A |
| Greenwell Point Road | Clipper Road | McKay Street | Urban | 60 | 12 | Kerb | Kerb | N/A | N/A | 12 | N/A |
| Kalandar Street | McKay Street | Stuart Street | Urban | 60 | 10.4 | Kerb | Kerb | N/A | N/A | 10.4 | N/A |
| Kalandar Street | Stuart Street | Wallace Street | Urban | 60 | 10.8 | Kerb | Kerb | N/A | N/A | 10.8 | N/A |
| Kalandar Street | Wallace Street | Princes Highway | Urban | 60 | 12 | Kerb | Kerb | N/A | N/A | 12 | N/A |

### 8.2. Existing Daily Traffic

Shoalhaven City Council provided GTA Consultants with peak to daily traffic conversion factors to apply to the May 2012 volumes counted at the study intersections to determine AADT and PSDT. These factors have been applied to the existing May 2012 turning movement volumes, and are shown in Figure 8.2. Shoalhaven City Council provided two conversion factors for both AADT and PSDT, and each were based on the Friday (8:00am-9:00am) or Saturday (12:00pm-1:00pm) peak hours. When applied to the turning volumes, in some cases the factors yielded different daily volumes. In these cases, as requested by Council, the higher or 'worst case' value has been selected for assessment.

Figure 8.2: Existing AADT and Seasonal Traffic Volumes


Base Map Source: maps.google.com.au
The existing daily traffic based on the factors provided by Shoalhaven City Council has been compared to the design capacity based on Austroads requirements. The comparison is provided in Table 8.3.


RURAL ROAD ASSESSMENT

Table 8.3: Design AADT and Existing Daily Traffic

| Road | From | To | Rural <br> Design <br> AADT | Existing <br> AADT | Existing <br> PSDT |
| :--- | :--- | :--- | :--- | :--- | :---: |
| Coonemia Road | Culburra Road | Forest Road | 150 to 500 | 3,079 | 3,786 |
| Forest Road | Coonemia Road | Callala Beach Road | 150 to 500 | 3,529 | 4,184 |
| Forest Road | Callala Beach Road | Princes Highway | 150 to 500 | 3,178 | 3,767 |
| Greenwell Point Road | West Street | Pyree Lane | 150 to 500 | 5,526 | 6,551 |
| Greenwell Point Road | Pyree Lane | Jindy Andy Lane | 500 to 1000 | 7,888 | 9,352 |
| Greenwell Point Road | Jindy Andy Lane | Apperleys Lane | 150 to 1000 | 5,947 | 7,051 |
| Greenwell Point Road | Apperleys Lane | Worrigee Road | 1,000 to | 6,144 | 7,285 |
| Jindy Andy Lane | Greenwell Point Road | Comerong Island Road | 150 to 500 | 2,250 | 2,667 |
| Pyree Lane | Coonemia Road | Jindy Andy Lane | 150 to 500 | 5,624 | 6,668 |

### 8.3. Anticipated Daily Development Traffic

The anticipated average and peak seasonal daily traffic has been estimated based on previous discussions with Council and investigations by GTA Consultants. The additional development traffic and expected future traffic post development is summarised in Figure 8.3 and Figure 8.4 respectively.

Figure 8.3: Anticipated Development Daily Traffic


Base Map Source: maps.google.com.au
The methods for calculating the traffic volumes generated by the development are explained in Section 7. According to GTA calculations, the worst case day for peak hour traffic generation onto the road network west of Culburra Beach was found to be Saturday and traffic distribution was assigned accordingly.


Figure 8.4: Anticipated Post Development Daily Traffic


Basemap source: maps.google.com.au (accessed February 2020)
Figure 8.3 and Figure 8.4 indicate that the rural road network surrounding Culburra Beach is expected to experience increases of between 345 and 1,991 vehicles on an average day, and between 472 and 2,361 vehicles at seasonal peaks. Culburra Road is expected to experience the greatest increases, originating directly from the development at 1,991 vpd (AADT) and 2,361 vpd (PSDT). Greenwell Point Road, Pyree Lane and Forest Road are also expected to see increased volumes.

However, it is also recognised that the performance of the road is more likely to be dictated by the peak hour performance of the intersections along its length. As shown in Section 7 the surveyed intersections Level of Service is unchanged with the addition of development traffic under $120^{\text {th }} \mathrm{HH}$ conditions.

### 8.4. Warrants for Overtaking Lanes

GTA Consultants has also investigated the warrants for overtaking lanes for the study roads identified in Figure 8.5. Guidance on the warrants and installation of overtaking lanes is provided in Austroads Guide to Road Design Part 3: Geometric Design Section 9.4 and Section 5.6.4.

Section 9.4 notes that "in deciding whether an overtaking lane is warranted, the evaluation needs to be carried out over a significant route length and not be isolated to the particular length over which the additional lane may be constructed."


## RURAL ROAD ASSESSMENT

Table 9.1 of Austroads Guide to Road Design Part 3: Geometric Design provides the traffic volume guidelines for providing overtaking lanes. The document also states that "Table 9.1 gives the current-year design volumes (AADT) at which overtaking lanes would normally be justified. These guidelines apply for short lowcost overtaking lanes at spacings of 10 to 15 km or more along a road in a given direction. If spacing is less than this, a specific cost benefit analysis will need to justify the construction at the shorter spacing."

The existing speed limits and road section lengths are provided in Figure 8.5.
Figure 8.5: Existing Speed Limit and Road Section Lengths


Basemap source: maps.google.com.au (accessed February 2020)
Figure 8.5 shows that study roads contain a mixture of $50,60,80$ and 100 kilometres per hour speed limits. Speed limits on all roads reduce as they approach the Princes Highway from the proposed development.

Typically, overtaking lanes are provided on high speed rural roads or where there are significant grades that could result in slow moving vehicles. The routes to and from the development from Princes Highway are mostly flat with minor grades with a single lane in each direction.

The longest stretch of existing rural road is 12.2 kilometres and that is through a previously upgraded section of Forest Road. For the vast majority of this road, double barrier lines are in place and sight distance is not sufficient for any overtaking.


## RURAL ROAD ASSESSMENT

The longest stretch of 100 kilometres per hour speed limit on Culburra Road is 5 kilometres and a review of that stretch shows there are only a couple of short sections (approximately 500 metres long) without barrier lines. None of those sections of Culburra Road are considered appropriate for an overtaking lane.

Coonemia Road are also a 100 kilometres per hour road, but its length is not considered long enough to warrant overtaking lanes.

Given the existing geometry and speed zones on the study roads, and the typical guidelines which suggest providing overtaking lanes every 10 to 15 kilometres, overtaking lanes are not considered necessary for any of the study roads and are not proposed to be provided.

### 8.5. Summary

The following conclusions from this rural road analysis can be drawn:

- The existing shoulder widths, including sealed shoulders, of the rural roads assessed do not meet the current Austroads guidance, which may have a road safety implication.
- Notwithstanding, the roads and intersections assessed are operating satisfactorily (analysis indicates that the intersections will continue to operate satisfactorily following completion of the development).
- An assessment of overtaking lanes on higher speed roads did not identify any locations where such treatments could be required (or easily introduced).


## 9. CONCLUSION



### 9.1. Conclusion

Based on the analysis and discussions presented within this report, the following conclusions are made:

1. The West Culburra Beach subdivision development involves approximately 47 ha on land west of the established area of Culburra Beach.
2. The subdivision comprises three precincts. On completion, the West Culburra Beach Development will include low and medium density residential dwellings, shop top housing, industrial lots and a sporting facility. While the residential subdivision is anticipated to be constructed first, construction of each development product is expected to be in response to market demand, noting all three stages (or parts thereof) can be constructed independently and concurrently if needed (from a traffic and transport perspective).
3. Access to the development is proposed from Culburra Road at three new roundabouts and the upgraded industrial area priority-controlled intersection.
4. GTA Consultants completed a design assessment of the three Culburra Road roundabouts proposed as part of the development. For improved road safety outcomes generally, it is recommended that the existing $100 \mathrm{~km} / \mathrm{h}$ speed limit is reduced to $80 \mathrm{~km} / \mathrm{h}$ to the west of the roundabout, with appropriate urban area entry treatment(s) provided. On this basis, the proposed roundabouts are suitable in both location and design.
5. The dedicated pedestrian/ cycle route proposed as part of the development is an east-west route along the foreshore area providing access to Culburra Beach shops. Connections to this route will be provided along the subdivision road network and the Culburra Road shared path.
6. It is recommended that all new bus stops provide shelter, seating, lighting, timetable information as a minimum
7. A minimum of a 1.2-metre-wide footpath is required on local and collector streets within a subdivision in line with SDCP 2014 Chapter G11, reference A38.2.
8. For the pedestrian and cycle path along the Crookhaven River foreshore reserve associated within the development, it is recommended to provide a minimum 3 metre width given their potential as recreational routes.
9. The pedestrian and bicycle network has been designed to allow pedestrian and cycling access to all key origins and destinations within, and outside the vicinity of the site.
10. It is anticipated that refuse collection for the new development areas will be undertaken by a standard 12.5-metre-long Council garbage vehicle.
11. In total, 174, 186 and 105 trips are expected to be generated from both the residential and non-residential developments onto the wider road network in the Friday AM, Friday PM and Saturday peaks respectively.
12. In assessing intersection performance on the road network surrounding the site, growth factors were applied to the recorded traffic volumes (May 2012) to represent the equivalent $120^{\text {th }}$ Highest Annual Hour $(\mathrm{HH})$. This was done to reflect the significant seasonal increases in traffic volumes in the region.
13. Under equivalent $120^{\text {th }} \mathrm{HH}$ traffic volumes the performance, the additional development traffic has marginal impact on intersections surrounding the site.
14. Under equivalent $120^{\text {th }} \mathrm{HH}$ traffic volumes the Princes Highway intersections at Kalandar Street and Moss Street currently operate at poor levels, particularly during the Friday AM and Friday PM peak periods. The addition of development traffic at these intersections (which would compromise only $2 \%$ of the flow at these intersections) would not result in any discernible change in intersection performance.
15. Marginal increase in traffic is expected due to the development. However, local intersections are expected to operate at satisfactory levels.

16. The existing shoulder widths, including sealed shoulders, of the rural roads assessed do not accord with current Austroads guidance.
17. Notwithstanding, the roads and intersections assessed are operating satisfactorily (Section 7 shows that the intersections will continue to operate satisfactorily following completion of the development).
18. An assessment of overtaking lanes on higher speed roads did not identify any locations where such treatments could be required (or easily introduced).

In conclusion, the additional development traffic has a marginal impact on the surrounding road network and the proposed access to each of the development precincts and the intersections within the regional network will continue to operate at satisfactory levels.

## A.SURVEY RESULTS



 $\uparrow$



| Job No. | : N790 |
| :--- | :--- |
| Client | : Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $:$ 1. Culburra Rd / Coonamia Rd |
|  |  |
| Day/Date | : Sat, 5th May 2012 |
| Weather | : Fine |
| Description | :Classified Intersection Count |
|  | $:$ Hourly Summary |



SKYHIGH - THE TRAFFIC SURVEY COMPANY




| Approach | Gulburra Rd |  |  |  |  |  |  |  |  | Mayfield Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 8 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 9 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | Direction 9U （U Turn） |  |  | $\begin{gathered} \hline \begin{array}{c} \text { Direction } 10 \\ \text { (Left Turn) } \end{array} \\ \hline \end{gathered}$ |  |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{array}{\|c\|} \hline \begin{array}{c} \text { Direction } 120 \\ \text { (U Turn) } \end{array} \\ \hline \end{array}$ |  |  |
| Time Period | 河 | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { di } \\ \hline \end{array}$ | $\begin{gathered} \overline{\mathrm{I}} \\ \stackrel{y}{\circ} \end{gathered}$ | $\begin{array}{\|c} \hline \mathbf{5} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\text { 㐅⿳亠丷厂犬土}} \\ \text { n } \end{array}$ | $\begin{array}{\|l\|} \hline \mathrm{I} \\ \hline 1 \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 总 } \end{array}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { à } \end{array}$ | $\begin{array}{\|l\|} \hline \mathrm{\Xi} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\begin{aligned} & \overline{\mathrm{g}} \\ & \stackrel{y}{\circ} \end{aligned}$ |  | $\begin{array}{\|c} \stackrel{\rightharpoonup}{9} \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \hline \mathrm{O} \end{array}$ | $\begin{array}{\|c} \stackrel{\rightharpoonup}{\mathrm{F}} \\ \hline \end{array}$ |  | $\stackrel{\text { ¢ }}{\stackrel{\text { ¢ }}{\circ}}$ |
| 12：00 10 12：15 | 47 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 | 0 | 1 | 0 | 0 | 0 |
| $12: 15$ 10 $12: 30$ | 45 | 0 | 45 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 |
| $12: 30$ 10 $12: 45$ <br> 1   <br> 1   | 57 | 0 | 57 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 | 0 | 1 | 0 | 0 | 0 |
| 12：45 to 13：00 | 39 | 1 | 40 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 |
| 13：00 10－13：15 | 44 | 1 | 45 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 | 0 | 1 | 0 | 0 | 0 |
| 13：15 to 13：30 | 53 | 0 | 53 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 |
| $13: 30$ to $13: 45$ | 60 | 0 | 60 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 | 0 | 1 | 0 | 0 | 0 |
| 13：45 to 14：00 | 46 | 0 | 46 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 | 0 | 1 | 0 | 0 | 0 |
| Totals | 391 | 2 | 393 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 5 | 0 | 5 | 0 | 0 | 0 |



| Approach | Gulburra Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction |  | ectio <br> eft T |  |  | $\begin{aligned} & \text { rection } \\ & \text { hroua } \end{aligned}$ |  |  | Tur |  |  |
| Time Period | $\begin{array}{\|l\|} \hline \text { 觅 } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \substack{\text { İ } \\ \text { İ工 } \\ \hline} \\ \hline \end{array}$ |  | $\begin{array}{\|c} \hline \text { 总 } \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \substack{3 \\ \text { xim }} \\ \hline \end{array}$ | $\frac{\overline{\mathrm{a}}}{\underline{\mathrm{o}}}$ | 营 |  | $\begin{aligned} & \stackrel{\overline{\mathrm{o}}}{\stackrel{1}{\circ}} \\ & \hline \end{aligned}$ |  |
| 12：00 to 13：00 | 2 | 0 | 2 | 209 | 1 | 210 | 0 | 0 | 0 |  |
| 12：15 to 13：15 | 2 | 0 | 2 | 204 | 0 | 204 | 0 | 0 | 0 |  |
| 12：30 to 13：30 | 2 | 0 | 2 | 174 | 0 | 174 | 0 | 0 | 0 |  |
| 12：45 to 13：45 | 1 | 0 | 1 | 139 | 1 | 140 | 0 | 0 | 0 |  |
| $13: 00$ to $14: 00$ | 1 | 0 | 1 | 139 | 1 | 140 | 0 | 0 | 0 |  |
| Totals | 3 | 0 | 3 | 348 | 2 | 350 | 0 | 0 | 0 |  |


| Approach | Gulburra Rd |  |  |  |  |  |  |  |  | Mayfield Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | $\begin{aligned} & \begin{array}{l} \text { Direction } 8 \\ \text { (Through) } \end{array} \end{aligned}$ |  |  | $\begin{gathered} \text { Direction } 9 \\ \text { (Right Turn) } \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \text { Direction } 10 \\ \text { (Left Turn) } \end{gathered}$ |  |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{gathered} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{array}{\|l\|} \hline \text { 吡 } \end{array}$ |  | $\begin{array}{\|l\|l\|} \hline \mathrm{I} \\ \hline 1 \end{array}$ | $\begin{array}{\|l\|} \hline \frac{5}{9} \\ \hline \end{array}$ | $\begin{aligned} & \text { 言 } \\ & \text { İ } \\ & \hline \end{aligned}$ |  | 皆 |  | $\begin{array}{\|l\|l\|} \hline \frac{\mathrm{⿺⿻}}{\mathrm{o}} \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 学 } \end{array}$ |  | $\begin{array}{\|l\|l\|} \hline \frac{\overline{\mathrm{g}}}{\mathrm{D}} \end{array}$ |  | $\begin{array}{\|l\|l\|} \hline \stackrel{\rightharpoonup}{9} \\ \hline \end{array}$ | $\begin{aligned} & \text { 䧺 } \\ & \text { In } \end{aligned}$ | $\begin{array}{\|l\|} \hline \mathrm{I} \\ \hline \mathrm{O} \end{array}$ | $\begin{array}{\|l\|l\|} \hline \text { 喜 } \\ \hline \end{array}$ | $\begin{aligned} & \text { 訔 } \\ & \text { İ } \end{aligned}$ | － |
| 12：00 to 13：00 | 188 | 1 | 189 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 2 | 0 | 2 | 0 | 0 | 0 |
| 12：15 to 13：15 | 185 | 2 | 187 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 2 | 0 | 2 | 0 | 0 | 0 |
| 12：30 to 13：30 | 193 | 2 | 195 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 2 | 0 | 2 | 0 | 0 | 0 |
| 12：45 to $13: 45$ | 196 | 2 | 198 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 2 | 0 | 2 | 0 | 0 | 0 |
| 13：00 to 14：00 | 203 | 1 | 204 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 3 | 0 | 3 | 0 | 0 | 0 |
| Totals | 391 | 2 | 393 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 5 | 0 | 5 | 0 | 0 | 0 |











$\uparrow$



| Job No. | : N790 |
| :--- | :--- |
| Client | : Realty Realizations |
| Suburb | : Nowra |
| Location | $: 5$. Greenwell Point Rd / Mayfield Rd |
|  |  |
| Day/Date | : Sat, 5th May 2012 |
| Weather | : Fine |
| Description | : Classified Intersection Count |
|  | : Hourly Summary |



Mayfield Rd




|  |  |  | Millbank Rd |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Job No． | ：N790 |  | $94.98{ }^{8}{ }^{7}$ |  |  |
| Client | ：Realty Realizations | ¢ |  | 할 |  |
| Suburb | ：Nowra | ！ |  | ！ |  |
| Location | ：6．Greenwell Point Rd／Millbank Rd | $\frac{\bar{\circ}}{\stackrel{\circ}{0}}$ |  | $\begin{aligned} & \text { ò } \\ & \hline \overline{\mathrm{o}} \end{aligned}$ |  |
| Day／Date | ：Sat，5th May 2012 | 気 | 気 ${ }^{\text {c }}$ | ${ }_{0}$ | SKYHIGH－THE TRAFFIC SURUEY COMPANY |
| Weather | ：Fine | － |  | O |  |
| Description | ：Classified Intersection Count |  | $\left\|\begin{array}{llll}1 & 2 & 3 & 3 U\end{array}\right\|$ |  |  |
|  | ： 15 mins Data |  | Millbank Rd |  |  |


| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7（Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 9 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | Direction 9U （U Turn） |  |  | $\begin{gathered} \hline \begin{array}{c} \text { Direction } 10 \\ \text { (Left Turn) } \end{array} \\ \hline \end{gathered}$ |  |  | Direction 11 （Through） |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{gathered} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{array}{\|l\|l} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \frac{\mathrm{x}}{\mathrm{⿺}} \\ \hline \end{array}$ | 苛 |  | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \stackrel{1}{2} \end{array}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{5} \\ & \hline \mathrm{~J} \end{aligned}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\text { 㐅⿳亠丷厂犬土}} \\ \text { n } \end{array}$ | $\begin{array}{\|l\|} \hline \mathrm{I} \\ \hline 1 \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 总 } \end{array}$ |  | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \stackrel{1}{2} \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \frac{\mathrm{⿺⿻} 丷 冖 ⿱ 丶 万 ⿱ ⿰ ㇒ 一 乂 。 ~}{2} \end{array}$ | $\begin{aligned} & \text { 蒿 } \end{aligned}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { di } \end{array}$ | $\stackrel{\overline{\mathrm{I}}}{\stackrel{\mathrm{I}}{\circ}}$ | $\begin{array}{\|c} \stackrel{5}{9} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { an } \end{array}$ | $\begin{array}{\|l\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心.} \end{array}$ | 苛 |  | $\stackrel{\text { ¢ }}{\text { ¢ }}$ |
| 12：00 to 12：15 | 0 | 0 | 0 | 5 | 2 | 7 | 5 | 0 | 5 | 0 | 0 | 0 | 4 | 0 | 4 | 55 | 0 | 55 | 14 | 0 | 14 | 0 | 0 | 0 |
| $\begin{array}{lllll}12: 15 & \text { to } & 12: 30\end{array}$ | 2 | 0 | 2 | 2 | 0 | 2 | 4 | 0 | 4 | 0 | 0 | 0 | 1 | 0 | 1 | 52 | 0 | 52 | 7 | 0 | 7 | 0 | 0 | 0 |
| 12：30 to 12：45 | 0 | 0 | 0 | 3 | 0 | 3 | 2 | 0 | 2 | 0 | 0 | 0 | 3 | 1 | 4 | 47 | 2 | 49 | 8 | 0 | 8 | 0 | 0 | 0 |
| 12：45 to 13：00 | 3 | 1 | 4 | 6 | 0 | 6 | 1 | 0 | 1 | 0 | 0 | 0 | 7 | 0 | 7 | 36 | 0 | 36 | 12 | 0 | 12 | 0 | 0 | 0 |
| 13：00 to 13：15 | 1 | 0 | 1 | 2 | 0 | 2 | 2 | 0 | 2 | 0 | 0 | 0 | 3 | 0 | 3 | 42 | 1 | ${ }^{43}$ | 9 | 0 | 9 | 0 | 0 | 0 |
| 13：15 to 13：30 | 2 | 0 | 2 | 6 | 0 | 6 | 3 | 0 | 3 | 0 | 0 | 0 | 7 | 0 | 7 | 47 | 1 | 48 | 10 | 0 | 10 | 0 | 0 | 0 |
| 13：30 to 13：45 | 1 | 0 | 1 | 2 | 0 | 2 | 2 | 0 | 2 | 0 | 0 | 0 | 8 | 0 | 8 | 57 | 2 | 59 | 11 | 0 | 11 | 0 | 0 | 0 |
| 13：45 to 14：00 | 1 | 0 | 1 | 3 | 0 | 3 | 3 | 0 | 3 | 0 | 0 | 0 | 4 | 0 | 4 | 49 | 1 | 50 | 11 | 0 | 11 | 0 | 0 | 0 |
| Totals | 10 | 1 | 11 | 29 | 2 | 31 | 22 | 0 | 22 | 0 | 0 | 0 | 37 | 1 | 38 | 385 | 7 | 392 | 82 | 0 | 82 | 0 | 0 | 0 |


|  |  |  |  | Millbank Rd |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Job No． | ：N790 |  |  |  |  |
| Client | ：Realty Realizations | 『 | ${ }^{\mathrm{D}}$－ | $\uparrow \downarrow \downarrow$ ¢ | \％ |
| Suburb | ：Nowra | 号 | 家 | $\overrightarrow{+}$ | 䓂 |
| Location | ：6．Greenwell Point Rd／Millbank Rd | $\stackrel{\text { 잉 }}{ }$ | $\pm$ | $\uparrow$ | $\stackrel{\circ}{\circ}$ |
|  |  | $\stackrel{0}{3}$ | $\stackrel{\rightharpoonup}{\sim}$ | $\stackrel{+}{\square}$ | $\stackrel{0}{3}$ |
| Day／Date | Sat，5th May 2012 | む̀ | $\stackrel{\rightharpoonup}{\sim}$ | $\checkmark$ | む |
| Weather | Fine | － |  | $\dot{\nu}_{B}$ | 0 |
| Description | ：Classified Intersection Count |  |  | $1{ }_{1}^{1} 23300$ |  |
|  | Hourly Summary |  |  | Millbank Rd |  |


| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 （Through） |  |  | $\begin{gathered} \text { Direction } 3 \text { 3 } \\ \text { (Right Turn) } \end{gathered}$ |  |  | $\begin{gathered} \text { Direction 3U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 6 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 6U } \\ \text { (U Turn) } \end{gathered}$ |  |  |
| Time Period | 总 |  | $\begin{array}{\|l\|} \hline \stackrel{\text { ® }}{\mathrm{o}} \\ \hline \end{array}$ | 营 | $\begin{aligned} & 3 \\ & \begin{array}{l} \text { In } \\ \text { In } \\ \hline \end{array} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 厄⿳一巛口口口阝 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l} \hline \text { 寽 } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { In } \\ \hline \end{array} \\ \hline \end{array}$ |  | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 3 } \\ & \text { I⿷匚⿳ } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 厄⿳一巛口口口阝 } \\ & \hline \end{aligned}$ | 总 | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { In } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \stackrel{\text { 玉̈ }}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 喜 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline 3 \\ \text { 㙜 } \\ \hline \end{array}$ | $\begin{aligned} & \overline{\mathrm{o}} \\ & \stackrel{\rightharpoonup}{\mathrm{o}} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { In } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{array}{\|} \stackrel{\text { ® }}{\stackrel{\circ}{\circ}} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline 5 \\ \hline \end{array}$ |  | － |
| 12：00 to 13：00 | 58 | 0 | 58 | 21 | 1 | 22 | 28 | 1 | 29 | 0 | 0 | 0 | 14 | 1 | 15 | 181 | 2 | 183 | 12 | 0 | 12 | 0 | 0 | 0 |
| 12：15 to 13：15 | 45 | 0 | 45 | 21 | 1 | 22 | 28 | 1 | 29 | 0 | 0 | 0 | 15 | 1 | 16 | 177 | 0 | 177 | 13 | 0 | 13 | 0 | 0 | 0 |
| 12：30 10 13：30 | 41 | 0 | 41 | 27 | 1 | 28 | ${ }^{30}$ | 1 | 31 | 0 | 0 | 0 | 15 | 1 | 16 | 155 | 1 | 156 | 6 | 0 | 6 | 0 | 0 | 0 |
| 12：45 to $13: 45$ | 35 | 0 | 35 | 28 | 2 | 30 | 25 | 1 | 26 | 0 | 0 | 0 | 16 | 0 | 16 | 130 | 2 | 132 | 7 | 1 | 8 | 0 | 0 | 0 |
| 13：00 to 14：00 | 31 | 1 | 32 | 28 | 1 | 29 | 25 | 1 | 26 | 0 | 0 | 0 | 15 | 14 | 29 | 139 | 2 | 141 | 7 | 1 | 8 | 0 | 0 | 0 |
| Totals | 89 | 1 | 90 | 49 | 2 | 51 | 53 | 2 | 55 | 0 | 0 | 0 | 29 | 15 | 44 | 320 | 4 | 324 | 19 | 1 | 20 | 0 | 0 | 0 |


| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 <br> （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （ U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12 U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 入 } \\ & \text { 崖 } \end{aligned}$ |  | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ |  |  | 喜 |  |  | 总 |  |  | $\begin{array}{\|l\|l} \hline \text { 总 } \\ \hline \end{array}$ |  | $\begin{aligned} & \stackrel{\overline{\mathrm{g}}}{\stackrel{\circ}{\circ}} \end{aligned}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ |  | $\stackrel{\text { 항 }}{ }$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ |  |  | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 各 } \\ & \text { I⿷匚⿳ } \end{aligned}$ |  |
| 12：00 to 13：00 | 5 | 1 | 6 | 16 | 2 | 8 | 12 | 0 | 12 | 0 | 0 | 0 | 15 | 1 | 16 | 190 | 2 | 192 | 41 | 0 | 41 | 0 | 0 | 0 |
| 12：15 to 13：15 | 6 | 1 | 7 | 13 | 0 | 13 | 9 | 0 | 9 | 0 | 0 | 0 | 14 | 1 | 15 | 177 | 3 | 180 | 36 | 0 | 36 | 0 | 0 | 0 |
| 12：30 to 13：30 | 6 | 1 | 7 | 17 | 0 | 17 | 8 | 0 | 8 | 0 | 0 | 0 | 20 | 1 | 21 | 172 | 4 | 176 | 39 | 0 | 39 | 0 | 0 | 0 |
| 12：45 to 13：45 | 7 | 1 | 8 | 16 | 0 | 16 | 8 | 0 | 8 | 0 | 0 | 0 | 25 | 0 | 25 | 182 | 4 | 186 | 42 | 0 | 42 | 0 | 0 | 0 |
| 13：00 to 14：00 | 5 | 0 | 5 | 13 | 0 | 13 | 10 | 0 | 10 | 0 | 0 | 0 | 22 | 0 | 22 | 195 | 5 | 200 | 41 | 0 | 41 | 0 | 0 | 0 |
| Totals | 10 | 1 | 11 | 29 | 2 | 31 | 22 | 0 | 22 | 0 | 0 | 0 | 37 | 1 | 38 | 385 | 7 | 392 | 82 | 0 | 82 | 0 | 0 | 0 |



|  |  |  | Princes Hwy |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Job No． | ：N790 |  |  |  |  |
| Client | ：Realty Realizations |  | $\mathrm{D}_{\text {¢ }}$ ¢ $\downarrow \downarrow \downarrow \square$ |  |  |
| Suburb | ：Nowra | $\stackrel{\square}{\omega}$ | $\overrightarrow{\overrightarrow{0} \uparrow} \xrightarrow{+1}$ | あ | 7 |
| Location | ：7．Kalandar St／Princes Hwy | 䟴 | $\stackrel{\text { ！}}{\substack{\text { ¢ }}}$ | 年 | 2） |
| Day／Date | ：Sat，5th May 2012 | $\stackrel{\text { ¢ }}{\text { ¢ }}$ | $\overrightarrow{\text { co }}$ | $\stackrel{\text { ¢ }}{\text { ¢ }}$ | SKYHIGH－THE TRAFFIC SURUEY COMPANY |
| Weather | ：Fine |  | $\dot{\dot{*}}_{\text {B }}$ |  |  |
| Description | ：Classified Intersection Count |  | A $\left\|\begin{array}{llll}1 & 2 & 3 & 3 U\end{array}\right\|$ |  |  |
|  | ： 15 mins Data |  | Princes Hwy |  |  |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 <br> （Left Turn） |  |  | $\begin{aligned} & \text { Direction } 88 \\ & \text { (Through) } \end{aligned}$ |  |  | Direction 9 （Right Turn） |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction } 10 \\ \text { (Left Turn) } \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction } 11 \\ \text { (Through) } \\ \hline \end{gathered}$ |  |  | Direction 12 <br> （Right Turn） |  |  | $\begin{array}{\|c\|} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{array}$ |  |  |
| Time Period | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \begin{array}{c} \text { Iam } \\ \text { In } \end{array} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \hline \end{array}$ | $\begin{array}{\|l} \hline \frac{5}{9} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{c} \text { àd } \\ \text { an } \\ \hline \end{array} \\ \hline \end{array}$ | $\frac{\text { 镸 }}{}$ | $\begin{array}{\|l} \hline \mathrm{F} \\ \hline \mathrm{I} \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} \text { Ia } \\ \text { an } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{\sigma}} \\ \hline \end{array}$ | $\begin{array}{\|c} \hline \stackrel{\text { F }}{\mathrm{I}} \\ \hline \end{array}$ |  |  | $\begin{array}{\|l} \hline \frac{\mathrm{F}}{9} \\ \hline \end{array}$ | $\begin{array}{\|c} \substack{\stackrel{\rightharpoonup}{I} \\ \stackrel{\rightharpoonup}{I} \\ \hline} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \hline \end{array}$ | $\begin{aligned} & \text { 嘉 } \end{aligned}$ |  | $\frac{\text { 硕 }}{}$ | $\begin{array}{\|l} \hline \frac{5}{9} \\ \hline \end{array}$ |  |  | $\begin{array}{\|c} \stackrel{5}{5} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { dix } \\ \hline \end{array}$ | 产 |
| 12：00 to 12：15 | 98 | 0 | 98 | 253 | 2 | 255 | 12 | 2 | 14 | 0 | 0 | 0 | 10 | 0 | 10 | 40 | 0 | 40 | ${ }^{35}$ | 0 | 35 | 0 | 0 | 0 |
| 12：15 to 12：30 | 95 | 1 | 96 | 298 | 3 | 301 | 10 | 2 | 12 | 0 | 0 | 0 | 20 | 0 | 20 | 19 | 0 | 19 | 24 | 0 | 24 | 0 | 0 | 0 |
| 12：30 to 12：45 | 97 | 0 | 97 | 314 | 4 | 318 | 17 | 4 | 21 | 0 | 0 | 0 | 10 | 1 | 11 | 34 | 0 | 34 | 39 | 1 | 40 | 0 | 0 | 0 |
| 12：45 to 13：00 | 86 | 0 | 86 | 240 | 2 | 242 | 22 | 0 | 22 | 0 | 0 | 0 | 9 | 0 | 9 | 35 | 0 | 35 | 32 | 0 | 32 | 0 | 0 | 0 |
| 13：00 to 13：15 | ${ }^{112}$ | 0 | 112 | 231 | 2 | 233 | 24 | 1 | 25 | 0 | 0 | 0 | 6 | 0 | 6 | 42 | 0 | 42 | 26 | 0 | 26 | 0 | 0 | 0 |
| 13：15 to 13：30 | 79 | 0 | 79 | 275 | 8 | 283 | 27 | 0 | 27 | 0 | 0 | 0 | 7 | 1 | 8 | 34 | 0 | 34 | 36 | 2 | 38 | 0 | 0 | 0 |
| 13：30 to 13：45 | 94 | 1 | 95 | 253 | 3 | 256 | 11 | 0 | 11 | 0 | 0 | 0 | 10 | 0 | 10 | 25 | 1 | 26 | 27 | 0 | 27 | 0 | 0 | 0 |
| 13：45 to 14：00 | 84 | 2 | 86 | 271 | 2 | 273 | 17 | 0 | 17 | 0 | 0 | 0 | 7 | 1 | 8 | 21 | 0 | 21 | 29 | 1 | 30 | 0 | 0 | 0 |
| Totals | 745 | 4 | 749 | 2135 | 26 | 2161 | 140 | 9 | 149 | 0 | 0 | 0 | 79 | 3 | 82 | 250 | 1 | 251 | 248 | 4 | 252 | 0 | 0 | 0 |


|  |  |  |  | Princes Hwy |
| :---: | :---: | :---: | :---: | :---: |
| Job No． | ：N790 |  |  | 9U $9.9 .8 .{ }^{8}$ |
| Client | ：Realty Realizations |  | 今 | $\uparrow \downarrow \downarrow$ |
| Suburb | ：Nowra | あ | 䎡 |  |
| Location | ：7．Kalandar St／Princes Hwy | \％ | $\stackrel{+}{\square}$ | 4 ¢ |
|  |  |  | $\vec{\sim}$ | ＋${ }^{+}$ |
| Day／Date | ：Sat，5th May 2012 | $\stackrel{\text { ® }}{ }$ | 式 | $\downarrow$ |
| Weather | ：Fine |  |  |  |
| Description | ：Classified Intersection Count |  |  | $\left\|\begin{array}{llll}1 & 2 & 3 & 3 U\end{array}\right\|$ |
|  | Hourly Summary |  |  | Princes Hw |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 （Through） |  |  | $\begin{gathered} \begin{array}{c} \text { Direction } 3 \\ \text { (Right Turn) } \end{array} \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 3U } \\ \text { (U Turn) } \end{gathered}$ |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | Direction 6（Right Turn） |  |  | $\begin{gathered} \hline \text { Direction 6U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { 玉̈x } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \text { 등 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \frac{5}{9} \\ \hline \end{array}$ |  | $\begin{gathered} \stackrel{\text { ® }}{\stackrel{\circ}{6}} \\ \hline \end{gathered}$ | $\begin{array}{\|l} \hline \frac{\mathrm{F}}{9} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \text { B } \\ \text { 玉्工 } \\ \hline \end{array}$ | $\begin{aligned} & \text { 응 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l} \hline \text { E } \\ \hline \end{array}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \hline 1 \end{aligned}$ | $\begin{array}{\|l} \hline \frac{5}{9} \\ \hline \end{array}$ | $$ |  | 总 | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { 区 } \\ \hline \end{array} \\ \hline \end{array}$ |  | $\begin{array}{\|l\|l\|} \hline \text { 亳 } \\ \hline \end{array}$ |  | $\begin{array}{\|l} \stackrel{\text { ® }}{\stackrel{1}{6}} \\ \hline \end{array}$ | 总 | $\begin{array}{r} \text { 3 } \\ \text { 圌 } \\ \hline \end{array}$ | － |
| 12：00 to 13：00 | 3 | 1 | 4 | 753 | 20 | 773 | 84 | 4 | 88 | 0 | 0 | 0 | 68 | 0 | 68 | 159 | 1 | 160 | 409 | 4 | 413 | 0 | 0 | 0 |
| 12：15 to 13：15 | 6 | 1 | 7 | 724 | 16 | 740 | 92 | 3 | 95 | 0 | 0 | 0 | 64 | 0 | 64 | 134 | 1 | 135 | 377 | 3 | 380 | 0 | 0 | 0 |
| 12：30 to 13：30 | 4 | 0 | 4 | 699 | 12 | 711 | 95 | 2 | 97 | 0 | 0 | 0 | 61 | 0 | 61 | 122 | 1 | 123 | 332 | 3 | 335 | 0 | 0 | 0 |
| 12：45 to 13：45 | 7 | 0 | 7 | 695 | 9 | 704 | 87 | 1 | 88 | 0 | 0 | 0 | 57 | 0 | 57 | 129 | 1 | 130 | 341 | 1 | 342 | 0 | 0 | 0 |
| 13：00 to 14：00 | 7 | 0 | 7 | 681 | 7 | 688 | 73 | 0 | 73 | 0 | 0 | 0 | 53 | 0 | 53 | 113 | 2 | 115 | 326 | 0 | 326 | 0 | 0 | 0 |
| Totals | 10 | 1 | 11 | 1434 | 27 | 1461 | 157 | 4 | 161 | 0 | 0 | 0 | 121 | 0 | 121 | 272 | 3 | 275 | 735 | 4 | 739 | － | 0 | 0 |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{gathered} \begin{array}{c} \text { Direction } 9 \\ \text { (Right Turn) } \end{array} \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | Direction 10（Left Turn） （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） <br> （Right Turn） |  |  | $\begin{array}{\|c\|} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{array}$ |  |  |
| Time Period | $\begin{array}{\|l\|} \hline \text { 䜳 } \\ \hline \end{array}$ |  | $\stackrel{\text { 픙 }}{ }$ | $\begin{aligned} & \text { 吡 } \end{aligned}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { as } \\ \hline \end{array} \\ \hline \end{array}$ |  | $\begin{array}{\|l} \hline \text { 寽 } \\ \hline \end{array}$ |  | $\stackrel{\text { 高 }}{\circ}$ | $\begin{aligned} & \text { 畐 } \\ & \hline \end{aligned}$ | $$ | $\begin{array}{\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心.} \end{array}$ | $\begin{array}{\|l\|l\|} \hline \text { 吡 } \\ \hline \end{array}$ |  |  | $\begin{array}{\|l\|l\|} \hline \text { 吡 } \end{array}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { am } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \frac{\overline{\mathrm{g}}}{\mathrm{i}} \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|l} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \frac{\overline{\mathrm{x}}}{\mathrm{i}} \\ \hline \end{array}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { im } \\ \hline \end{array} \\ \hline \end{array}$ | $\stackrel{\text { ¢ }}{\text { ¢ }}$ |
| 12：00 to 13：00 | 376 | 1 | 377 | 1105 | 11 | 1116 | 61 | 8 | 69 | 0 | 0 | 0 | 49 | 1 | 50 | 128 | 0 | 128 | 130 | 1 | 131 | 0 | 0 | 0 |
| 12：15 to 13：15 | 390 | 1 | 391 | 1083 | 11 | 1094 | 73 | 7 | 80 | 0 | 0 | 0 | 45 | 1 | 46 | 130 | 0 | 130 | 121 | 1 | 122 | 0 | 0 | 0 |
| $\begin{array}{llll}12: 30 & 10 & 13: 30\end{array}$ | 374 | 0 | 374 | 1060 | 16 | 1076 | 90 | 5 | 95 | 0 | 0 | 0 | 32 | 2 | 34 | 145 | 0 | 145 | 133 | 3 | 136 | 0 | 0 | 0 |
| 12：45 to 13：45 | 371 | 1 | 372 | 999 | 15 | 1014 | 84 | 1 | 85 | 0 | 0 | 0 | 32 | 1 | 33 | 136 | 1 | 137 | 121 | 2 | 123 | 0 | 0 | 0 |
| 13：00 to 14：00 | 369 | 3 | 372 | 1030 | 15 | 1045 | 79 | 1 | 80 | 0 | 0 | 0 | 30 | 2 | 32 | 122 | 1 | 123 | 118 | 3 | 121 | 0 | 0 | 0 |
| Totals | 745 | 4 | 749 | 2135 | 26 | 2161 | 140 | 9 | 149 | 0 | 0 | 0 | 79 | 3 | 82 | 250 | 1 | 251 | 248 | 4 | 252 | 0 | 0 | 0 |






| Approach |  | Currarong Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction |  |  | Direction 5 (Through) |  |  | $\begin{gathered} \text { Direction } 6 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 6U } \\ \text { (U Turn) } \end{gathered}$ |  |  |
| Time Period |  |  | $\begin{aligned} & \text { 镸 } \\ & \hline \end{aligned}$ |  | $\begin{array}{\|l\|} \hline \stackrel{\text { Ï }}{\circ} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \text { 喜 } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { In } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \overline{\mathrm{\circ}} \mathrm{i} \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|l} \hline 5 \\ \hline \end{array}$ |  | ¢ |
| $12: 00$ 10 $13: 00$ |  |  | 8 | 0 | 8 | 20 | 0 | 20 | 0 | 0 | 0 |
| $12: 15$ to $13: 15$ <br> 12 lo  |  |  | 9 | 0 | 9 | 18 | 0 | 18 | 0 | 0 | 0 |
| $\begin{array}{llll}12: 30 & 10 & 13: 30\end{array}$ |  |  | 7 | 0 | 7 | 17 | 0 | 17 | 0 | 0 | 0 |
| 12:45 to $13: 45$ |  |  | 6 | 0 | 6 | 19 | 0 | 19 | 0 | 0 | 0 |
|  |  |  | 8 | 0 | 8 | 20 | 0 | 20 | 0 | 0 | 0 |
| Totals |  |  | 16 | 0 | 16 | 40 | 0 | 40 | 0 | 0 | 0 |





| Approach | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 9 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (UTurn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \begin{array}{c} \text { Direction } 10 \\ \text { (Left Turn) } \end{array} \\ \hline \end{gathered}$ |  |  | Direction 11 （Through） |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{gathered} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{array}{\|c} \hline \stackrel{\rightharpoonup}{\Xi} \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \mathrm{y} \\ \hline 1 \end{array}$ | 苛 | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { di } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \stackrel{1}{2} \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 吡 } \end{array}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\text { 㐅⿳亠丷厂犬土}} \\ \text { n } \end{array}$ | $\begin{array}{\|l\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心} \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 总 } \end{array}$ |  | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \stackrel{1}{2} \end{array}$ | $\begin{array}{\|c} \hline \stackrel{\rightharpoonup}{\Xi} \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \bar{\circ} \mathrm{I} \\ \hline \end{array}$ |  | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { di } \end{array}$ | $\stackrel{\overline{\mathrm{I}}}{\stackrel{\mathrm{I}}{\circ}}$ | $\begin{array}{\|c} \stackrel{5}{9} \\ \hline \end{array}$ | $\begin{aligned} & \text { ? } \\ & \text { Ï } \\ & \text { In } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心.} \end{array}$ |  |  | － |
| 12：00 to 12：15 | 32 | 1 | 33 | 3 | 0 | 3 | 21 | 1 | 22 | 1 | 0 | 1 | 24 | 0 | 24 | 41 | 0 | 41 | 2 | 1 | 3 | 1 | 0 | 1 |
| $\begin{array}{lllll}12: 15 & \text { to } & 12: 30\end{array}$ | 28 | 0 | 28 | 4 | 0 | 4 | 27 | 0 | 27 | 0 | 0 | 0 | 28 | 1 | 29 | 34 | 0 | 34 | 3 | 0 | 3 | 0 | 0 | 0 |
| 12：30 to 12：45 | 27 | 0 | 27 | 6 | 0 | 6 | 29 | 2 | 31 | 1 | 1 | 2 | 32 | 0 | 32 | 45 | 1 | 46 | 5 | 0 | 5 | 1 | 1 | 2 |
| 12：45 to 13：00 | 31 | 0 | 31 | 3 | 0 | 3 | 35 | 0 | 35 | 0 | 0 | 0 | 36 | 1 | 37 | 41 | 0 | 41 | 3 | 1 | 4 | 0 | 0 | 0 |
| 13：00 to 13：15 | 27 | 0 | 27 | 2 | 0 | 2 | 26 | 1 | 27 | 2 | 0 | 2 | 27 | 1 | 28 | 40 | 1 | 41 | 5 | 0 | 5 | 2 | 0 | 2 |
| 13：15 to 13：30 | 26 | 0 | 26 | 5 | 0 | 5 | 28 | 2 | 30 | 0 | 1 | 1 | 26 | 0 | 26 | ${ }^{33}$ | 1 | 34 | 4 | 0 | 4 | 0 | 1 | 1 |
| 13：30 to 13：45 | 26 | 0 | 26 | 3 | 0 | 3 | 33 | 0 | 33 | 2 | 0 | 2 | 25 | 1 | 26 | 29 | 1 | 30 | 3 | 0 | 3 | 2 | 0 | 2 |
| 13：45 to 14：00 | 27 | 1 | 28 | 3 | 0 | 3 | 29 | 1 | 30 | 1 | 0 | 1 | 30 | 1 | 31 | 27 | 1 | 28 | 5 | 1 | 6 | 1 | 0 | 1 |
| Totals | 224 | 2 | 226 | 29 | 0 | 29 | 228 | 7 | 235 | 7 | 2 | 9 | 228 | 5 | 233 | 290 | 5 | 295 | 30 | 3 | 33 | 7 | 2 | 9 |



| Approach | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 9 \\ \text { (Right Turn) } \end{gathered}$ |  |  | Direction 9U （U Turn） |  |  | $\begin{gathered} \hline \text { Direction } 10 \\ \text { (Left Turn) } \\ \hline \end{gathered}$ |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） <br> （Right Turn） |  |  | $\begin{array}{\|c\|} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{array}$ |  |  |
| Time Period | $\begin{array}{\|l\|} \hline \text { 䜳 } \\ \hline \end{array}$ |  | $\stackrel{\text { 玉َ }}{\stackrel{\rightharpoonup}{\circ}}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { as } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \mathrm{y} \\ \hline 1 \end{array}$ | $\begin{array}{\|l} \hline \frac{5}{9} \\ \hline \end{array}$ | $$ | $\stackrel{\text { 高 }}{\circ}$ | $\begin{array}{\|l\|} \hline \text { 总 } \\ \hline \end{array}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { an } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心.} \end{array}$ | $\begin{array}{\|l} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\stackrel{\text { 흥 }}{ }$ | $\begin{array}{\|l\|l\|} \hline \text { 吡 } \end{array}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { am } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \frac{\overline{\mathrm{g}}}{\mathrm{i}} \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \text { 寽 } \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \frac{\overline{\mathrm{x}}}{\mathrm{i}} \\ \hline \end{array}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $$ | 产 |
| 12：00 to 13：00 | 118 | 1 | 119 | 16 | 0 | 16 | 112 | 3 | 115 | 2 | 1 | 3 | 120 | 2 | 122 | 161 | 1 | 162 | 13 | 2 | 15 | 2 | 1 | 3 |
| 12：15 to 13：15 | 113 | 0 | 113 | 15 | 0 | 15 | 117 | 3 | 120 | 3 | 1 | 4 | 123 | 3 | 126 | 160 | 2 | 162 | 16 | 1 | 17 | 3 | 1 | 4 |
| 12：30 | 111 | 0 | 111 | 16 | 0 | 16 | 118 | 5 | 123 | 3 | 2 | 5 | 121 | 2 | 123 | 159 | 3 | 162 | 17 | 1 | 18 | 3 | 2 | 5 |
| 12：45 to 13：45 | 110 | 0 | 110 | 13 | 0 | 13 | 122 | 3 | 125 | 4 | 1 | 5 | 114 | 3 | 117 | 143 | 3 | 146 | 15 | 1 | 16 | 4 | 1 | 5 |
| 13：00 to 14：00 | 106 | 1 | 107 | 13 | 0 | 13 | 116 | 4 | 120 | 5 | 1 | 6 | 108 | 3 | 111 | 129 | 4 | 133 | 17 | 1 | 18 | 5 | 1 | 6 |
| Totals | 224 | 2 | 226 | 29 | 0 | 29 | 228 | 7 | 235 | 7 | 2 | 9 | 228 | 5 | 233 | 290 | 5 | 295 | 30 | 3 | 33 | 7 | 2 | 9 |





| Approach | Princes Hwy |  |  |  |  |  |  |  |  | Forest Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 2 （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  |  | $\begin{gathered} \hline \text { Direction } 6 \\ \text { (Right Turn) } \end{gathered}$ |  |  | Direction 6U （U Turn） |  |  |
| Time Period | 营 | $\begin{array}{\|l\|l\|} \substack{3 \\ \text { In } \\ \hline} \\ \hline \end{array}$ | 坒 | 总 |  |  | 夁 |  | 家 | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 㽞 } \\ & \underline{\text { n }} \end{aligned}$ |  | 宕 | 夁 |  | 年 | 苛 | $\begin{aligned} & \underline{\mathrm{ax}} \\ & \underline{1} \\ & \hline \end{aligned}$ | 产 |
| $\begin{array}{llll}12.00 & \text { to } & 13.00\end{array}$ | ${ }^{642}$ | 11 | 653 | 30 | 0 | \％ | 0 | 0 | － | 34 | 0 |  | 34 | 65 | 2 | 67 | 0 | 0 | 0 |
| 12.15 to 13.15 <br> 123   | 629 | 10 | 639 | 26 | 0 | 26 | 0 | 0 | － | 36 | 0 |  | 36 | 70 | 2 | 72 | 0 | 0 | 0 |
| $\begin{array}{lllll}1230 & \text { 10 } & 13.30\end{array}$ | 621 | 11 | 632 | 31 | 0 | 1 | 0 | 0 | － | 28 | 0 |  | 28 | 59 | 0 | 59 | 0 | 0 | 0 |
| 12.45 10 13：45 | 618 | 12 | 630 | 31 | 0 | 1 | 0 | 0 | 0 | 32 | 1 |  | 33 | 59 | 0 | 59 | 0 | 0 | 0 |
| 13.00 to 14.00 | 604 | 18 | 622 | 29 | 0 | 9 | 0 | 0 | 0 | 41 | 2 |  | 43 | 70 | 0 | 70 | 0 | 0 | 0 |
| Totals | 1246 | 29 | 1275 | 59 | 0 | 9 | 0 | 0 | 0 | 75 | 2 |  | 77 | 135 | 2 | 137 | 0 | 0 | 0 |




| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{array}{\|c} \hline \begin{array}{c} \text { Direction } 9 \\ \text { (Right Turn) } \end{array} \\ \hline \end{array}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{gathered} \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{array}{\|l\|l} \hline \frac{\mathrm{F}}{\mathrm{I}} \\ \hline \end{array}$ | $$ | $\begin{array}{\|l\|} \hline \mathrm{y} \\ \hline 1 \end{array}$ | $\begin{aligned} & \text { 总 } \end{aligned}$ |  | $\begin{gathered} \overline{\mathrm{g}} \mathrm{⿺} \\ \hline \end{gathered}$ | $\begin{array}{\|l} \hline \mathrm{F} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { ix } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{g}} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \stackrel{\rightharpoonup}{\mathrm{I}} \\ \hline \end{array}$ |  | $\begin{aligned} & \overline{\mathrm{g}} \\ & \hline 1 \end{aligned}$ | $\begin{array}{\|l} \hline \frac{5}{9} \\ \hline \end{array}$ | $\begin{array}{\|l} \hline \stackrel{3}{3} \\ \text { id } \\ \hline \end{array}$ | $\begin{array}{\|l} \overline{\mathrm{g}} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|l} \hline \text { 坒 } \end{array}$ | $\begin{array}{\|l\|} \hline \stackrel{\rightharpoonup}{3} \\ \text { à } \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \overline{\mathrm{I}} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathrm{g}} \\ \hline \end{array}$ | $\begin{aligned} & \text { ? } \\ & \text { İ } \\ & \text { İ } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{g}} \\ & \hline \stackrel{y}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|l\|} \hline ⿳ 亠 㐅 \\ \text { İ } \\ \text { In } \\ \hline \end{array}$ | $\stackrel{\text { ¢ }}{\text { ¢ }}$ |
| 12：00 10－12：15 | ${ }^{26}$ | 2 | 28 | ${ }^{288}$ | 6 | 294 | 47 | 0 | 47 | 0 | 0 | 0 | 61 | 0 | 61 | 41 | 0 | 41 | 21 | 0 | 21 | 0 | 0 | 0 |
| 12：15 to 12：30 | 21 | 0 | 21 | 245 | 5 | 250 | 65 | 2 | 67 | 0 | 0 | 0 | 58 | 0 | 58 | 35 | 0 | 35 | 22 | 0 | 22 | 0 | 0 | 0 |
| 12：30 to 12：45 | 20 | 0 | 20 | 271 | 2 | 273 | 44 | 0 | 44 | 0 | 0 | 0 | 57 | 0 | 57 | 17 | 0 | 17 | ${ }^{15}$ | 1 | 16 | 0 | 0 | 0 |
| 12：45 to 13：00 | 18 | 0 | 18 | 229 | 3 | 232 | 38 | 0 | 38 | 0 | 0 | 0 | 60 | 0 | 60 | 24 | 0 | 24 | 19 | 0 | 19 | 0 | 0 | 0 |
| 13：00 to 13：15 | 27 | 0 | 27 | 234 | 2 | 236 | 58 | 1 | 59 | 0 | 0 | 0 | 73 | 0 | 73 | 34 | 0 | 34 | 23 | 1 | 24 | 0 | 0 | 0 |
| $13: 15$ to $13: 30$ | 17 | 0 | 17 | 271 | 6 | 277 | 54 | 0 | 54 | 0 | 0 | 0 | 57 | 0 | 57 | 18 | 0 | 18 | 13 | 2 | 15 | 0 | 0 | 0 |
| 13：30 to 13：45 | 25 | 0 | 25 | 277 | 4 | 281 | 37 | 0 | 37 | 0 | 0 | 0 | 46 | 0 | 46 | 20 | 0 | 20 | 10 | 1 | 11 | 0 | 0 | 0 |
| 13：45 to 14：00 | 25 | 0 | 25 | 271 | 3 | 274 | 58 | 0 | 58 | 0 | 0 | 0 | 51 | 0 | 51 | 34 | 0 | 34 | 11 | 0 | 11 | 0 | 0 | 0 |
| Totals | 179 | 2 | 181 | 2086 | 31 | 2117 | 401 | 3 | 404 | 0 | 0 | 0 | 463 | 0 | 463 | 223 | 0 | 223 | 134 | 5 | 139 | 0 | 0 | 0 |



| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 <br> （Left Turn） |  |  | Direction 2 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 3 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | Direction 3 U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | $\begin{gathered} \text { Direction } 6 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 6U } \\ \text { (U Turn) } \end{gathered}$ |  |  |
| Time Period | 喜 |  |  | 皆 |  | $\stackrel{\text { 틍 }}{ }$ | $\begin{array}{\|l\|} \hline \text { 总 } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { 区 } \\ \text { In } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \stackrel{\text { II }}{\circ} \end{array}$ | 喜 | $\begin{array}{\|l\|} \hline \begin{array}{l} \text { İ } \\ \text { İ工 } \\ \hline \end{array} \\ \hline \end{array}$ | $\stackrel{\text { 틍 }}{ }$ | $\begin{array}{\|l\|} \hline \text { 寽 } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} \text { 3 } \\ \text { İ } \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \text { 厄⿳一巛口亍刂。 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \stackrel{\rightharpoonup}{\Xi} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \text { B } \\ \text { 区ix } \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \stackrel{\text { IIO}}{\circ} \\ \hline \end{array}$ | $\begin{array}{\|l} \hline \mathrm{F} \\ \hline \end{array}$ |  | $\begin{aligned} & \overline{\mathrm{g}} \\ & \stackrel{1}{\circ} \\ & \hline \end{aligned}$ | 总 | $\begin{array}{\|l\|l\|} \hline \begin{array}{l} 3 \\ \text { ax } \\ \hline \end{array} \\ \hline \end{array}$ | － |
| 12：00 to 13：00 | 9 | 0 | 9 | 915 | 25 | 940 | 79 | 0 | 79 | 0 | 0 | 0 | 24 | 0 | 24 | ${ }^{61}$ | 0 | 61 | 109 | 2 | 111 | 0 | 0 | 0 |
| 12：15 to 13：15 | 7 | 0 | 7 | 856 | 17 | 873 | 74 | 0 | 74 | 0 | 0 | 0 | 24 | 0 | 24 | 63 | 0 | 63 | 109 | 2 | 111 | 0 | 0 | 0 |
| 12：30 to 13：30 | 6 | 0 | 6 | 823 | 16 | 839 | 67 | 1 | 68 | 1 | 0 | 1 | 21 | 0 | 21 | 47 | 0 | 47 | 111 | 2 | 113 | 0 | 0 | 0 |
| 12：45 to 13：45 | 9 | 0 | 9 | 811 | 15 | 826 | 65 | 1 | 66 | 2 | 0 | 2 | 23 | 0 | 23 | 47 | 0 | 47 | 106 | 2 | 108 | 0 | 0 | 0 |
| 13：00 to 14：00 | 9 | 0 | 9 | 764 | 12 | 776 | 67 | 1 | 68 | 2 | 0 | 2 | 26 | 0 | 26 | 49 | 0 | 49 | 102 | 2 | 104 | 0 | 0 | 0 |
| Totals | 18 | 0 | 18 | 1679 | 37 | 1716 | 146 | 1 | 147 | 2 | 0 | 2 | 50 | 0 | 50 | 110 | 0 | 110 | 211 | 4 | 215 | 0 | 0 | 0 |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 <br> （Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{array}{c\|} \hline \text { Direction } 9 \\ \text { (Right Turn) } \end{array}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \text { Direction } 10 \\ \text { (Left Turn) } \\ \hline \end{gathered}$ |  |  | Direction 11 （Through） |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{gathered} \hline \text { Direction } 12 \mathrm{U} \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{array}{\|l\|l\|} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\stackrel{\text { 픙 }}{ }$ | $\begin{aligned} & \text { 皆 } \end{aligned}$ |  |  | $\begin{array}{\|l\|} \hline \text { 寽 } \\ \hline \end{array}$ | $$ | $\begin{array}{\|l\|} \hline \mathrm{I} \\ \hline 1 \end{array}$ | $\begin{array}{\|l\|} \hline \text { 总 } \\ \hline \end{array}$ |  | $\begin{aligned} & \overline{\mathrm{g}} \\ & \hline 1 \end{aligned}$ | $\begin{array}{\|l} \hline \mathbf{F} \\ \hline \end{array}$ | $$ | $\stackrel{\overline{\mathrm{g}}}{\stackrel{\mathrm{I}}{\circ}}$ | $\begin{array}{\|l\|l\|} \hline \text { 吡 } \\ \hline \end{array}$ | $$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 学 } \\ \hline \end{array}$ | $$ | $\begin{array}{\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心.} \\ \hline \end{array}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ |  | － |
| 12：00 to 13：00 | 85 | 2 | 87 | 1033 | 16 | 1049 | 194 | 2 | 196 | 0 | 0 | 0 | 236 | 0 | 236 | 117 | 0 | 117 | 77 | 1 | 78 | 0 | 0 | 0 |
| 12：15 to 13：15 | 86 | 0 | 86 | 979 | 12 | 991 | 205 | 3 | 208 | 0 | 0 | 0 | 248 | 0 | 248 | 110 | 0 | 110 | 79 | 2 | 81 | 0 | 0 | 0 |
| 12：30 | 82 | 0 | 82 | 1005 | 13 | 1018 | 194 | 1 | 195 | 0 | 0 | 0 | 247 | 0 | 247 | 93 | 0 | 93 | 70 | 4 | 74 | 0 | 0 | 0 |
| 12：45 to 13：45 | 87 | 0 | 87 | 1011 | 15 | 1026 | 187 | 1 | 188 | 0 | 0 | 0 | 236 | 0 | 236 | 96 | 0 | 96 | 65 | 4 | 69 | 0 | 0 | 0 |
| 13：00 to 14：00 | 94 | 0 | 94 | 1053 | 15 | 1068 | 207 | 1 | 208 | 0 | 0 | 0 | 227 | 0 | 227 | 106 | 0 | 106 | 57 | 4 | 61 | 0 | 0 | 0 |
| Totals | 179 | 2 | 181 | 2086 | 31 | 2117 | 401 | 3 | 404 | 0 | 0 | 0 | 463 | 0 | 463 | 223 | 0 | 223 | 134 | 5 | 139 | 0 | 0 | 0 |







| Job No. | : N790 |
| :--- | :--- |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $:$ 1. Culburra Rd / Coonamia Rd |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | : Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |








| Approach | Gulburra Rd |  |  |  |  |  |  |  |  | Mayfield Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  |  | Direction 12 <br> （Right Turn） |  | Direction 12U <br> （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{\circ}} \\ & \hline \end{aligned}$ | $$ |  | $\begin{aligned} & \text { ज口 } \\ & \stackrel{1}{0} \end{aligned}$ |  |  |  | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \text { ভ } \\ & \stackrel{\rightharpoonup}{\circ} \\ & \hline \end{aligned}$ | 皆 |  | $\begin{array}{r} \overline{\boxed{\circ}} \\ \stackrel{\rightharpoonup}{\circ} \\ \hline \end{array}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | ¢ |
| 7：00 to 7：15 | 11 | 9 | 20 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 3 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7：15 to $7: 30$ | 16 | 10 | 26 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7：30 to 7：45 | 12 | 1 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 |
| 7：45 to 8：00 | 18 | 1 | 19 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8：00 to 8：15 | 18 | 4 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8：15 to 8：30 | 28 | 1 | 29 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 |
| 8：30 to $8: 45$ | 34 | 1 | 35 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8：45 to 9：00 | 20 | 1 | 21 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 1 | 0 | 0 | 0 |
| AM Totals | 157 | 28 | 185 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 2 | 4 | 3 | 0 | 3 | 0 | 0 | 0 |
| 16：00 to 16：15 | 62 | 3 | 65 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 |
| 16：15 to 16：30 | 71 | 1 | 72 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 16：30 to 16：45 | 76 | 1 | 77 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 16：45 to 17：00 | 63 | 0 | 63 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 17：00 to 17：15 | 84 | 0 | 84 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 |


| 17:15 to 17:30 | 89 | 0 | 89 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 17:30 to 17:45 | 83 | 1 | 84 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 |
| 17:45 to 18:00 | 76 | 1 | 77 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| PM Totals | 604 | 7 | 611 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 1 | 3 | 0 | 0 | 0 |


| Job No. | : N790 |
| :--- | :--- |
| Client | : Realty Realizations |
| Suburb | : Nowra |
| Location | : 2. Gulburra Rd / Mayfield Rd |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | : Fine |
| Description | : Classified Intersection Count |
|  | : Hourly Summary |






SKYHIGH - THE TRAFFIC SURVEY COMPANY





| Job No. | : N790 |
| :--- | :--- |
| Client | : Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $: 3$. Greenwell Point Rd / Pyree Ln |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | : Fine |
| Description | : Classified Intersection Count |
|  | : Hourly Summary |



SKYHIGH - THE TRAFFIC SURVEY COMPANY






Description ：Classified Intersection Count
： 15 mins Data

| Approach | Jindy Andy Ln |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 <br> （Through） |  |  | Direction 12 U （U Turn） |  |  |
| Time Period | $$ | ふ <br> $\substack{\text { en } \\ \text { ¢ }}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\circ}{1} \\ & \hline \end{aligned}$ | 菏 |  | $\begin{aligned} & \overline{\oplus 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline 1 \end{aligned}$ |  | $\begin{aligned} & \overline{\oplus 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  |  | 董 | 入 <br> $\substack{0 \\ \text { ¢ }}$ | \} |
| 7：00 to 7：15 | 3 | 0 | 3 | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 1 | 2 | 15 | 17 | 32 | 0 | 0 | 0 |
| 7：15 to 7：30 | 5 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 2 | 20 | 14 | 34 | 0 | 0 | 0 |
| 7：30 to 7：45 | 1 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 16 | 3 | 19 | 0 | 0 | 0 |
| 7：45 to 8：00 | 3 | 0 | 3 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 22 | 5 | 27 | 0 | 0 | 0 |
| 8：00 to 8：15 | 8 | 0 | 8 | 3 | 1 | 4 | 0 | 0 | 0 | 2 | 0 | 2 | 19 | 5 | 24 | 0 | 0 | 0 |
| $8: 15$ to $8: 30$ | 5 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 32 | 0 | 32 | 0 | 0 | 0 |
| 8：30 to 8：45 | 10 | 1 | 11 | 1 | 0 | 1 | 0 | 0 | 0 | 2 | 2 | 4 | 33 | 2 | 35 | 0 | 0 | 0 |
| 8：45 to 9：00 | 9 | 0 | 9 | 3 | 2 | 5 | 1 | 0 | 1 | 0 | 1 | 1 | 21 | 3 | 24 | 0 | 0 | 0 |
| AM Totals | 44 | 2 | 46 | 8 | 4 | 12 | 1 | 0 | 1 | 7 | 6 | 13 | 178 | 49 | 227 | 0 | 0 | 0 |


| 16:00 to 16:15 | 27 | 1 | 28 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 65 | 2 | 67 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16:15 to 16:30 | 24 | 1 | 25 | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 1 | 2 | 67 | 0 | 67 | 0 | 0 | 0 |
| 16:30 to 16:45 | 28 | 0 | 28 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 73 | 3 | 76 | 0 | 0 | 0 |
| 16:45 to 17:00 | 27 | 0 | 27 | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 59 | 0 | 59 | 0 | 0 | 0 |
| 17:00 to 17:15 | 32 | 0 | 32 | 3 | 0 | 3 | 0 | 0 | 0 | 1 | 0 | 1 | 77 | 0 | 77 | 0 | 0 | 0 |
| 17:15 to 17:30 | 37 | 0 | 37 | 2 | 0 | 2 | 0 | 0 | 0 | 1 | 0 | 1 | 79 | 0 | 79 | 0 | 0 | 0 |
| 17:30 to 17:45 | 26 | 0 | 26 | 1 | 0 | 1 | 0 | 0 | 0 | 3 | 0 | 3 | 77 | 0 | 77 | 0 | 0 | 0 |
| 17:45 to 18:00 | 23 | 1 | 24 | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 64 | 2 | 66 | 0 | 0 | 0 |
| PM Totals | 224 | 3 | 227 | 9 | 0 | 9 | 0 | 0 | 0 | 14 | 1 | 15 | 561 | 7 | 568 | 0 | 0 | 0 |









| Job No. | : N790 |
| :--- | :--- |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $: 5$. Greenwell Point Rd / Mayfield Rd |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |



SKYHIGH - THE TRAFFIC SURVEY COMPANY


| Approach |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction |  |  | Direction 11 (Through) |  |  | Direction 12 (Right Turn) |  |  | Direction 12U (U Turn) |  |  |
| Time Period |  |  |  | ¢ | $\begin{aligned} & \overline{5} \\ & \stackrel{\circ}{1} \end{aligned}$ | 苛 | त § ¢ ¢ | ¢ |  | $\xrightarrow{\substack{\text { ¢ } \\ \text { ¢ }}}$ | 픈 |
| 7:00 to 8:00 |  |  | 75 | 41 | 116 | 3 | 0 | 3 | 0 | 0 | 0 |
| 7:15 to $8: 15$ |  |  | 79 | 28 | 107 | 3 | 0 | 3 | 0 | 0 | 0 |
| 7:30 to 8:30 |  |  | 92 | 13 | 105 | 2 | 0 | 2 | 0 | 0 | 0 |
| 7:45 to 8:45 |  |  | 110 | 12 | 122 | 4 | 0 | 4 | 0 | 0 | 0 |
| 8:00 to 9:00 |  |  | 109 | 11 | 120 | 5 | 0 | 5 | 0 | 0 | 0 |
| AM Totals |  |  | 184 | 52 | 236 | 8 | 0 | 8 | 0 | 0 | 0 |
| 16:00 to 17:00 |  |  | 274 | 4 | 278 | 10 | 0 | 10 | 0 | 0 | 0 |
| 16:15 to 17:15 |  |  | 285 | 2 | 287 | 12 | 0 | 12 | 0 | 0 | 0 |
| 16:30 to 17:30 |  |  | 292 | 2 | 294 | 9 | 0 | 9 | 0 | 0 | 0 |
| 16:45 to 17:45 |  |  | 302 | 0 | 302 | 10 | 0 | 10 | 0 | 0 | 0 |
| 17:00 to 18:00 |  |  | 300 | 2 | 302 | 10 | 0 | 10 | 0 | 0 | 0 |
| PM Totals |  |  | 574 | 6 | 580 | 20 | 0 | 20 | 0 | 0 | 0 |



| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 岩 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $$ | $\begin{aligned} & 3 \\ & \begin{array}{l} 3 \\ \frac{0}{x} \end{array} \end{aligned}$ | $\begin{aligned} & \overline{\text { In }} \\ & \text { ○ } \end{aligned}$ |  |  | $\begin{aligned} & \overline{\mathrm{W}} \\ & \stackrel{-}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 芌 } \\ & \stackrel{O}{J} \end{aligned}$ | $\begin{aligned} & 3 \\ & \begin{array}{c} 3 \\ \text { I } \end{array} \end{aligned}$ | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{\rightharpoonup}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 들 } \\ & \\ & \hline \end{aligned}$ | ¢ | ¢ |
| 7：00 to 7：15 | 6 | 0 | 6 | 4 | 2 | 6 | 0 | 2 | 2 | 0 | 0 | 0 | 5 | 0 | 5 | 47 | 2 | 49 | 7 | 0 | 7 | 0 | 0 | 0 |
| 7：15 to $7: 30$ | 7 | 1 | 8 | 4 | 0 | 4 | 1 | 1 | 2 | 0 | 0 | 0 | 5 | 0 | 5 | 43 | 0 | 43 | 8 | 1 | 9 | 0 | 0 | 0 |
| 7：30 to 7：45 | 9 | 1 | 10 | 2 | 0 | 2 | 3 | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 3 | 81 | 6 | 87 | 7 | 0 | 7 | 0 | 0 | 0 |
| 7：45 to 8：00 | 21 | 1 | 22 | 9 | 0 | 9 | 2 | 0 | 2 | 0 | 0 | 0 | 4 | 3 | 7 | 73 | 4 | 77 | 3 | 1 | 4 | 0 | 0 | 0 |
| 8：00 to 8：15 | 18 | 2 | 20 | 19 | 3 | 22 | 4 | 0 | 4 | 0 | 0 | 0 | 3 | 0 | 3 | 100 | 1 | 101 | 4 | 2 | 6 | 0 | 0 | 0 |
| 8：15 to 8：30 | 15 | 1 | 16 | 30 | 0 | 30 | 8 | 0 | 8 | 0 | 0 | 0 | 1 | 0 | 1 | 59 | 2 | 61 | 5 | 0 | 5 | 0 | 0 | 0 |
| $8: 30$ to $8: 45$ | 8 | 0 | 8 | 35 | 0 | 35 | 4 | 0 | 4 | 0 | 0 | 0 | 2 | 0 | 2 | 70 | 3 | 73 | 13 | 0 | 13 | 0 | 0 | 0 |
| 8：45 to 9：00 | 15 | 0 | 15 | 13 | 0 | 13 | 2 | 0 | 2 | 0 | 0 | 0 | 4 | 0 | 4 | 65 | 3 | 68 | 3 | 0 | 3 | 0 | 0 | 0 |
| AM Totals | 99 | 6 | 105 | 116 | 5 | 121 | 24 | 3 | 27 | 0 | 0 | 0 | 27 | 3 | 30 | 538 | 21 | 559 | 50 | 4 | 54 | 0 | 0 | 0 |
| 16：00 to 16：15 | 10 | 0 | 10 | 4 | 0 | 4 | 2 | 0 | 2 | 0 | 0 | 0 | 5 | 0 | 5 | 38 | 3 | 41 | 2 | 0 | 2 | 0 | 0 | 0 |
| 16：15 to $16: 30$ | 9 | 0 | 9 | 12 | 0 | 12 | 6 | 0 | 6 | 0 | 0 | 0 | 3 | 0 | 3 | 21 | 2 | 23 | 3 | 0 | 3 | 0 | 0 | 0 |
| 16：30 to 16：45 | 7 | 0 | 7 | 7 | 0 | 7 | 7 | 1 | 8 | 0 | 0 | 0 | 4 | 0 | 4 | 22 | 1 | 23 | 2 | 0 | 2 | 0 | 0 | 0 |
| 16：45 to 17：00 | 13 | 0 | 13 | 9 | 0 | 9 | 3 | 0 | 3 | 0 | 0 | 0 | 1 | 1 | 2 | 23 | 0 | 23 | 2 | 0 | 2 | 0 | 0 | 0 |
| 17：00 to 17：15 | 9 | 1 | 10 | 7 | 0 | 7 | 10 | 0 | 10 | 0 | 0 | 0 | 8 | 0 | 8 | 26 | 0 | 26 | 4 | 0 | 4 | 0 | 0 | 0 |
| 17：15 to 17：30 | 5 | 0 | 5 | 4 | 0 | 4 | 6 | 0 | 6 | 0 | 0 | 0 | 3 | 0 | 3 | 32 | 0 | 32 | 3 | 0 | 3 | 0 | 0 | 0 |
| 17：30 to 17：45 | 9 | 0 | 9 | 7 | 0 | 7 | 6 | 0 | 6 | 0 | 0 | 0 | 3 | 0 | 3 | 32 | 0 | 32 | 1 | 1 | 2 | 0 | 0 | 0 |
| 17：45 to 18：00 | 11 | 0 | 11 | 6 | 0 | 6 | 5 | 0 | 5 | 0 | 0 | 0 | 2 | 0 | 2 | 20 | 1 | 21 | 6 | 0 | 6 | 0 | 0 | 0 |
| PM Totals | 73 | 1 | 74 | 56 | 0 | 56 | 45 | 1 | 46 | 0 | 0 | 0 | 29 | 1 | 30 | 214 | 7 | 221 | 23 | 1 | 24 | 0 | 0 | 0 |

： 15 mins Data
Millbank Rd

| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 <br> （Right Turn） |  |  | Direction 12 U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ |  | $\begin{aligned} & \bar{\Pi} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{aligned} & \overline{\# ँ} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ |  | $\begin{aligned} & \overline{50} \\ & \stackrel{0}{1} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \text { IO } \end{aligned}$ |  | $\begin{aligned} & \overline{50} \\ & \text { 으 } \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\oplus 5} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ | $\frac{\stackrel{\rightharpoonup}{5}}{\underline{3}}$ |  | $\begin{aligned} & \bar{\Pi} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{IN}} \\ & \stackrel{0}{1} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | 듳 |
| 7：00 to 7：15 | 2 | 1 | 3 | 1 | 0 | 1 | 1 | 1 | 2 | 0 | 0 | 0 | 3 | 0 | 3 | 20 | 8 | 28 | 2 | 0 | 2 | 0 | 0 | 0 |
| 7：15 to 7：30 | 0 | 0 | 0 | 0 | 1 | 1 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 16 | 6 | 22 | 1 | 1 | 2 | 0 | 0 | 0 |
| 7：30 to 7：45 | 3 | 0 | 3 | 0 | 0 | 0 | 4 | 0 | 4 | 0 | 0 | 0 | 2 | 0 | 2 | 16 | 4 | 20 | 6 | 1 | 7 | 0 | 0 | 0 |
| 7：45 to 8：00 | 3 | 0 | 3 | 2 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 20 | 4 | 24 | 5 | 0 | 5 | 0 | 0 | 0 |
| 8：00 to 8：15 | 4 | 0 | 4 | 3 | 1 | 4 | 2 | 0 | 2 | 0 | 0 | 0 | 11 | 1 | 12 | 23 | 2 | 25 | 3 | 0 | 3 | 0 | 0 | 0 |
| 8：15 to 8：30 | 1 | 0 | 1 | 8 | 1 | 9 | 2 | 0 | 2 | 0 | 0 | 0 | 14 | 0 | 14 | 32 | 4 | 36 | 5 | 0 | 5 | 0 | 0 | 0 |
| 8：30 to 8：45 | 3 | 0 | 3 | 6 | 1 | 7 | 3 | 0 | 3 | 0 | 0 | 0 | 9 | 0 | 9 | 23 | 1 | 24 | 5 | 1 | 6 | 0 | 0 | 0 |
| 8：45 to 9：00 | 2 | 2 | 4 | 2 | 0 | 2 | 3 | 0 | 3 | 0 | 0 | 0 | 3 | 2 | 5 | 32 | 4 | 36 | 36 | 2 | 38 | 0 | 0 | 0 |
| AM Totals | 18 | 3 | 21 | 22 | 4 | 26 | 16 | 1 | 17 | 0 | 0 | 0 | 44 | 3 | 47 | 182 | 33 | 215 | 63 | 5 | 68 | 0 | 0 | 0 |


| 16：00 to 16：15 | 2 | 0 | 2 | 9 | 1 | 10 | 4 | 0 | 4 | 0 | 0 | 0 | 4 | 0 | 4 | 72 | 0 | 72 | 21 | 0 | 21 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16：15 to 16：30 | 5 | 0 | 5 | 9 | 0 | 9 | 3 | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 3 | 69 | 1 | 70 | 17 | 1 | 18 | 0 | 0 | 0 |
| 16：30 to 16：45 | 2 | 0 | 2 | 8 | 0 | 8 | 3 | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 3 | 67 | 1 | 68 | 19 | 0 | 19 | 0 | 0 | 0 |
| 16：45 to 17：00 | 6 | 0 | 6 | 9 | 0 | 9 | 2 | 0 | 2 | 0 | 0 | 0 | 1 | 0 | 1 | 66 | 0 | 66 | 19 | 0 | 19 | 0 | 0 | 0 |
| 17：00 to 17：15 | 4 | 0 | 4 | 8 | 0 | 8 | 2 | 0 | 2 | 0 | 0 | 0 | 4 | 0 | 4 | 64 | 1 | 65 | 21 | 0 | 21 | 0 | 0 | 0 |
| 17：15 to 17：30 | 2 | 0 | 2 | 8 | 0 | 8 | 4 | 0 | 4 | 0 | 0 | 0 | 2 | 0 | 2 | 83 | 0 | 83 | 22 | 1 | 23 | 0 | 0 | 0 |
| 17：30 to 17：45 | 2 | 0 | 2 | 8 | 0 | 8 | 4 | 0 | 4 | 0 | 0 | 0 | 1 | 0 | 1 | 71 | 5 | 76 | 15 | 0 | 15 | 0 | 0 | 0 |
| 17：45 to 18：00 | 4 | 0 | 4 | 6 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 57 | 0 | 57 | 17 | 0 | 17 | 0 | 0 | 0 |
| PM Totals | 27 | 0 | 27 | 65 | 1 | 66 | 22 | 0 | 22 | 0 | 0 | 0 | 21 | 0 | 21 | 549 | 8 | 557 | 151 | 2 | 153 | 0 | 0 | 0 |


| Job No． | $:$ N790 |
| :--- | :--- |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $: 6$. Greenwell Point Rd／Millbank Rd |
|  |  |
| Day／Date | ：Fri，4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |



SKYHICH－THE TRAFFIC SURVEY COMPANY

| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U <br> （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{aligned} & \bar{\Xi} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\frac{\stackrel{7}{0}}{\underline{I}}$ |  | $\begin{aligned} & \overline{๊ ๊} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{O}} \\ & \stackrel{y}{\mathrm{O}} \end{aligned}$ | $\begin{aligned} & \text { ふ } \\ & \text { オ } \\ & \text { d } \end{aligned}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \underline{3} \end{aligned}$ |  | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\text { Z }}{\substack{x}} \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline 1 \end{aligned}$ | $\begin{aligned} & \underset{\text { Z }}{\substack{\text { I }}} \end{aligned}$ | $\begin{gathered} \overline{\mathrm{I}} \\ \stackrel{-}{\circ} \end{gathered}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\text { Z }}{\substack{\text { I } \\ \hline}} \end{aligned}$ | ¢ |
| 7：00 to 8：00 | 43 | 3 | 46 | 19 | 2 | 21 | 6 | 3 | 9 | 0 | 0 | 0 | 17 | 3 | 20 | 244 | 12 | 256 | 25 | 2 | 27 | 0 | 0 | 0 |
| 7：15 to 8：15 | 55 | 5 | 60 | 34 | 3 | 37 | 10 | 1 | 11 | 0 | 0 | 0 | 15 | 3 | 18 | 297 | 11 | 308 | 22 | 4 | 26 | 0 | 0 | 0 |
| 7：30 to 8：30 | 63 | 5 | 68 | 60 | 3 | 63 | 17 | 0 | 17 | 0 | 0 | 0 | 11 | 3 | 14 | 313 | 13 | 326 | 19 | 3 | 22 | 0 | 0 | 0 |
| 7：45 to 8：45 | 62 | 4 | 66 | 93 | 3 | 96 | 18 | 0 | 18 | 0 | 0 | 0 | 10 | 3 | 13 | 302 | 10 | 312 | 25 | 3 | 28 | 0 | 0 | 0 |
| 8：00 to 9：00 | 56 | 3 | 59 | 97 | 3 | 100 | 18 | 0 | 18 | 0 | 0 | 0 | 10 | 0 | 10 | 294 | 9 | 303 | 25 | 2 | 27 | 0 | 0 | 0 |
| AM Totals | 99 | 6 | 105 | 116 | 5 | 121 | 24 | 3 | 27 | 0 | 0 | 0 | 27 | 3 | 30 | 538 | 21 | 559 | 50 | 4 | 54 | 0 | 0 | 0 |
| 16：00 to 17：00 | 39 | 0 | 39 | 32 | 0 | 32 | 18 | 1 | 19 | 0 | 0 | 0 | 13 | 1 | 14 | 104 | 6 | 110 | 9 | 0 | 9 | 0 | 0 | 0 |
| 16：15 to 17：15 | 38 | 1 | 39 | 35 | 0 | 35 | 26 | 1 | 27 | 0 | 0 | 0 | 16 | 1 | 17 | 92 | 3 | 95 | 11 | 0 | 11 | 0 | 0 | 0 |
| 16：30 to 17：30 | 34 | 1 | 35 | 27 | 0 | 27 | 26 | 1 | 27 | 0 | 0 | 0 | 16 | 1 | 17 | 103 | 1 | 104 | 11 | 0 | 11 | 0 | 0 | 0 |
| 16：45 to 17：45 | 36 | 1 | 37 | 27 | 0 | 27 | 25 | 0 | 25 | 0 | 0 | 0 | 15 | 1 | 16 | 113 | 0 | 113 | 10 | 1 | 11 | 0 | 0 | 0 |
| 17：00 to 18：00 | 34 | 1 | 35 | 24 | 0 | 24 | 27 | 0 | 27 | 0 | 0 | 0 | 16 | 0 | 16 | 110 | 1 | 111 | 14 | 1 | 15 | 0 | 0 | 0 |
| PM Totals | 73 | 1 | 74 | 56 | 0 | 56 | 45 | 1 | 46 | 0 | 0 | 0 | 29 | 1 | 30 | 214 | 7 | 221 | 23 | 1 | 24 | 0 | 0 | 0 |


| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline 1 \end{aligned}$ | $\begin{aligned} & 3 \\ & \begin{array}{l} 7 \\ \text { I } \end{array} \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\rightharpoonup}{1} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \underline{J} \end{aligned}$ | $\begin{aligned} & \sum_{\pi}^{入} \\ & \text { © } \end{aligned}$ | $\begin{aligned} & \overline{\boxed{\circ}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\frac{\stackrel{\rightharpoonup}{0}}{\underline{O}}$ |  | $\begin{gathered} \overline{\boxed{\circ}} \\ \stackrel{\circ}{\circ} \end{gathered}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \mathrm{I} \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \bar{Ð} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{V}} \\ & \stackrel{\rightharpoonup}{J} \end{aligned}$ | 入 <br> $\substack{\text { ¢ } \\ \text { ¢ }}$ <br>  | \} |
| 7：00 to 8：00 | 8 | 1 | 9 | 3 | 1 | 4 | 6 | 1 | 7 | 0 | 0 | 0 | 7 | 0 | 7 | 72 | 22 | 94 | 14 | 2 | 16 | 0 | 0 | 0 |
| 7：15 to 8：15 | 10 | 0 | 10 | 5 | 2 | 7 | 7 | 0 | 7 | 0 | 0 | 0 | 15 | 1 | 16 | 75 | 16 | 91 | 15 | 2 | 17 | 0 | 0 | 0 |
| 7：30 to 8：30 | 11 | 0 | 11 | 13 | 2 | 15 | 8 | 0 | 8 | 0 | 0 | 0 | 29 | 1 | 30 | 91 | 14 | 105 | 19 | 1 | 20 | 0 | 0 | 0 |
| 7：45 to 8：45 | 11 | 0 | 11 | 19 | 3 | 22 | 7 | 0 | 7 | 0 | 0 | 0 | 36 | 1 | 37 | 98 | 11 | 109 | 18 | 1 | 19 | 0 | 0 | 0 |
| 8：00 to 9：00 | 10 | 2 | 12 | 19 | 3 | 22 | 10 | 0 | 10 | 0 | 0 | 0 | 37 | 3 | 40 | 110 | 11 | 121 | 49 | 3 | 52 | 0 | 0 | 0 |
| AM Totals | 18 | 3 | 21 | 22 | 4 | 26 | 16 | 1 | 17 | 0 | 0 | 0 | 44 | 3 | 47 | 182 | 33 | 215 | 63 | 5 | 68 | 0 | 0 | 0 |
| 16：00 to 17：00 | 15 | 0 | 15 | 35 | 1 | 36 | 12 | 0 | 12 | 0 | 0 | 0 | 11 | 0 | 11 | 274 | 2 | 276 | 76 | 1 | 77 | 0 | 0 | 0 |
| 16：15 to 17：15 | 17 | 0 | 17 | 34 | 0 | 34 | 10 | 0 | 10 | 0 | 0 | 0 | 11 | 0 | 11 | 266 | 3 | 269 | 76 | 1 | 77 | 0 | 0 | 0 |
| 16：30 to 17：30 | 14 | 0 | 14 | 33 | 0 | 33 | 11 | 0 | 11 | 0 | 0 | 0 | 10 | 0 | 10 | 280 | 2 | 282 | 81 | 1 | 82 | 0 | 0 | 0 |
| 16：45 to 17：45 | 14 | 0 | 14 | 33 | 0 | 33 | 12 | 0 | 12 | 0 | 0 | 0 | 8 | 0 | 8 | 284 | 6 | 290 | 77 | 1 | 78 | 0 | 0 | 0 |
| 17：00 to 18：00 | 12 | 0 | 12 | 30 | 0 | 30 | 10 | 0 | 10 | 0 | 0 | 0 | 10 | 0 | 10 | 275 | 6 | 281 | 75 | 1 | 76 | 0 | 0 | 0 |
| PM Totals | 27 | 0 | 27 | 65 | 1 | 66 | 22 | 0 | 22 | 0 | 0 | 0 | 21 | 0 | 21 | 549 | 8 | 557 | 151 | 2 | 153 | 0 | 0 | 0 |



| Approach <br> Direction | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U <br> （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 <br> （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{gathered} \text { 历̈ } \\ \stackrel{\circ}{\circ} \end{gathered}$ | $$ |  | $\begin{aligned} & \bar{\leftrightarrows} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $$ | $\begin{aligned} & \underset{0}{\lambda} \\ & \text { ָin } \end{aligned}$ | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\boxed{\circ}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $$ |  | $\begin{aligned} & \text { 历̈ } \\ & \stackrel{\rightharpoonup}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { ㄷㅡㅡㄹ } \\ & \hline 0 \end{aligned}$ |  | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $$ | $\begin{aligned} & \underset{\substack{3 \\ \mathbf{x} \\ \text { In }}}{ } \end{aligned}$ | $\begin{aligned} & \stackrel{\pi}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & 3 \\ & \begin{array}{c} 3 \\ \text { I } \end{array} \end{aligned}$ | 픙 |
| 7：00 to 7：15 | 0 | 0 | 0 | 67 | 23 | 90 | 5 | 2 | 7 | 0 | 0 | 0 | 9 | 1 | 10 | 35 | 0 | 35 | 47 | 2 | 49 | 0 | 0 | 0 |
| 7：15 to 7：30 | 0 | 0 | 0 | 120 | 4 | 124 | 9 | 0 | 9 | 0 | 0 | 0 | 6 | 1 | 7 | 36 | 2 | 38 | 78 | 0 | 78 | 0 | 0 | 0 |
| 7：30 to 7：45 | 1 | 0 | 1 | 136 | 20 | 156 | 6 | 0 | 6 | 0 | 0 | 0 | 8 | 0 | 8 | 29 | 0 | 29 | 83 | 3 | 86 | 0 | 0 | 0 |
| 7：45 to 8：00 | 0 | 0 | 0 | 164 | 7 | 171 | 6 | 1 | 7 | 0 | 0 | 0 | 13 | 1 | 14 | 62 | 0 | 62 | 114 | 0 | 114 | 0 | 0 | 0 |
| 8：00 to 8：15 | 1 | 0 | 1 | 184 | 15 | 199 | 9 | 0 | 9 | 0 | 0 | 0 | 5 | 0 | 5 | 47 | 1 | 48 | 130 | 5 | 135 | 0 | 0 | 0 |
| 8：15 to 8：30 | 1 | 0 | 1 | 185 | 8 | 193 | 4 | 0 | 4 | 0 | 0 | 0 | 7 | 0 | 7 | 75 | 2 | 77 | 172 | 6 | 178 | 0 | 0 | 0 |
| 8：30 to 8：45 | 1 | 0 | 1 | 200 | 10 | 210 | 11 | 0 | 11 | 0 | 0 | 0 | 8 | 2 | 10 | 64 | 0 | 64 | 182 | 5 | 187 | 0 | 0 | 0 |
| 8：45 to 9：00 | 0 | 0 | 0 | 195 | 12 | 207 | 8 | 0 | 8 | 0 | 0 | 0 | 9 | 1 | 10 | 66 | 5 | 71 | 160 | 3 | 163 | 0 | 0 | 0 |
| AM Totals | 4 | 0 | 4 | 1251 | 99 | 1350 | 58 | 3 | 61 | 0 | 0 | 0 | 65 | 6 | 71 | 414 | 10 | 424 | 966 | 24 | 990 | 0 | 0 | 0 |
| 16：00 to 16：15 | 5 | 0 | 5 | 163 | 6 | 169 | 18 | 0 | 18 | 0 | 0 | 0 | 18 | 1 | 19 | 44 | 0 | 44 | 101 | 3 | 104 | 0 | 0 | 0 |
| 16：15 to $16: 30$ | 0 | 0 | 0 | 184 | 4 | 188 | 22 | 1 | 23 | 0 | 0 | 0 | 15 | 0 | 15 | 42 | 1 | 43 | 88 | 1 | 89 | 0 | 0 | 0 |
| 16：30 to $16: 45$ | 1 | 0 | 1 | 161 | 2 | 163 | 22 | 0 | 22 | 0 | 0 | 0 | 18 | 0 | 18 | 35 | 2 | 37 | 98 | 3 | 101 | 0 | 0 | 0 |
| 16：45 to 17：00 | 2 | 0 | 2 | 171 | 6 | 177 | 11 | 0 | 11 | 0 | 0 | 0 | 15 | 0 | 15 | 40 | 1 | 41 | 88 | 0 | 88 | 0 | 0 | 0 |
| 17：00 to 17：15 | 1 | 0 | 1 | 156 | 4 | 160 | 16 | 2 | 18 | 0 | 0 | 0 | 20 | 1 | 21 | 44 | 1 | 45 | 99 | 0 | 99 | 0 | 0 | 0 |
| 17：15 to 17：30 | 0 | 0 | 0 | 130 | 2 | 132 | 14 | 0 | 14 | 0 | 0 | 0 | 19 | 0 | 19 | 47 | 1 | 48 | 110 | 1 | 111 | 0 | 0 | 0 |
| 17：30 to 17：45 | 1 | 0 | 1 | 162 | 5 | 167 | 15 | 0 | 15 | 0 | 0 | 0 | 13 | 0 | 13 | 38 | 0 | 38 | 91 | 1 | 92 | 0 | 0 | 0 |
| 17：45 to 18：00 | 3 | 0 | 3 | 126 | 4 | 130 | 10 | 0 | 10 | 0 | 0 | 0 | 15 | 0 | 15 | 51 | 0 | 51 | 100 | 0 | 100 | 0 | 0 | 0 |
| PM Totals | 13 | 0 | 13 | 1253 | 33 | 1286 | 128 | 3 | 131 | 0 | 0 | 0 | 133 | 2 | 135 | 341 | 6 | 347 | 775 | 9 | 784 | 0 | 0 | 0 |



| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 <br> （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 <br> （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 鹍 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \sum_{J}^{\gtrless} \\ & \text { din } \end{aligned}$ | $\begin{aligned} & \text { 픈 } \\ & \hline \end{aligned}$ |  |  | $\begin{aligned} & \bar{\cong} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline .0 \end{aligned}$ | $\begin{aligned} & \text { 入 } \\ & \text { 조 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{50} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ | $\begin{aligned} & \text { 入 } \\ & \text { 조 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{5} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ | $\begin{aligned} & \sum_{J}^{7} \\ & \frac{0}{1} \end{aligned}$ | $\begin{aligned} & \overline{\ddagger 5} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ | $\begin{aligned} & \text { 入 } \\ & \text { 조 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \bar{\cong} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline .0 \end{aligned}$ | ス <br> $\substack{\text { ¢ } \\ \text { ¢ }}$ | $\begin{aligned} & \overline{5} \\ & \stackrel{\circ}{1} \\ & \hline \end{aligned}$ | $\frac{\text { 苛 }}{3}$ | 橘 | ¢ |
| 7：00 to 7：15 | 22 | 5 | 27 | 101 | 15 | 116 | 18 | 2 | 20 | 0 | 0 | 0 | 2 | 2 | 4 | 11 | 5 | 16 | 19 | 0 | 19 | 0 | 0 | 0 |
| 7：15 to 7：30 | 29 | 4 | 33 | 118 | 17 | 135 | 9 | 1 | 10 | 0 | 0 | 0 | 3 | 2 | 5 | 12 | 1 | 13 | 19 | 0 | 19 | 0 | 0 | 0 |
| 7：30 to 7：45 | 21 | 2 | 23 | 151 | 15 | 166 | 18 | 3 | 21 | 0 | 0 | 0 | 11 | 1 | 12 | 11 | 3 | 14 | 14 | 1 | 15 | 0 | 0 | 0 |
| 7：45 to 8：00 | 41 | 2 | 43 | 178 | 12 | 190 | 21 | 0 | 21 | 0 | 0 | 0 | 6 | 2 | 8 | 14 | 1 | 15 | 28 | 0 | 28 | 0 | 0 | 0 |
| 8：00 to 8：15 | 49 | 2 | 51 | 195 | 10 | 205 | 27 | 4 | 31 | 0 | 0 | 0 | 8 | 2 | 10 | 20 | 1 | 21 | 34 | 4 | 38 | 0 | 0 | 0 |
| 8：15 to 8：30 | 40 | 2 | 42 | 185 | 12 | 197 | 35 | 5 | 40 | 0 | 0 | 0 | 10 | 2 | 12 | 26 | 0 | 26 | 33 | 1 | 34 | 0 | 0 | 0 |
| 8：30 to 8：45 | 46 | 5 | 51 | 150 | 16 | 166 | 33 | 2 | 35 | 0 | 0 | 0 | 12 | 1 | 13 | 27 | 0 | 27 | 30 | 0 | 30 | 0 | 0 | 0 |
| 8：45 to 9：00 | 69 | 7 | 76 | 170 | 17 | 187 | 23 | 4 | 27 | 0 | 0 | 0 | 8 | 1 | 9 | 52 | 1 | 53 | 28 | 2 | 30 | 0 | 0 | 0 |
| AM Totals | 317 | 29 | 346 | 1248 | 114 | 1362 | 184 | 21 | 205 | 0 | 0 | 0 | 60 | 13 | 73 | 173 | 12 | 185 | 205 | 8 | 213 | 0 | 0 | 0 |


| 16：00 to 16：15 | 150 | 6 | 156 | 289 | 8 | 297 | 29 | 2 | 31 | 0 | 0 | 0 | 15 | 3 | 18 | 55 | 0 | 55 | 61 | 1 | 62 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16：15 to 16：30 | 128 | 0 | 128 | 259 | 8 | 267 | 20 | 1 | 21 | 0 | 0 | 0 | 24 | 1 | 25 | 49 | 0 | 49 | 53 | 0 | 53 | 0 | 0 | 0 |
| 16：30 to 16：45 | 128 | 0 | 128 | 268 | 7 | 275 | 22 | 2 | 24 | 0 | 0 | 0 | 15 | 3 | 18 | 50 | 0 | 50 | 50 | 1 | 51 | 0 | 0 | 0 |
| 16：45 to 17：00 | 148 | 0 | 148 | 304 | 13 | 317 | 17 | 0 | 17 | 0 | 0 | 0 | 13 | 0 | 13 | 52 | 0 | 52 | 38 | 0 | 38 | 0 | 0 | 0 |
| 17：00 to 17：15 | 149 | 1 | 150 | 284 | 4 | 288 | 15 | 1 | 16 | 0 | 0 | 0 | 16 | 0 | 16 | 67 | 0 | 67 | 45 | 0 | 45 | 0 | 0 | 0 |
| 17：15 to 17：30 | 156 | 2 | 158 | 282 | 6 | 288 | 12 | 0 | 12 | 0 | 0 | 0 | 8 | 0 | 8 | 47 | 1 | 48 | 44 | 0 | 44 | 0 | 0 | 0 |
| 17：30 to 17：45 | 157 | 4 | 161 | 280 | 7 | 287 | 22 | 1 | 23 | 0 | 0 | 0 | 8 | 0 | 8 | 51 | 2 | 53 | 38 | 0 | 38 | 0 | 0 | 0 |
| 17：45 to 18：00 | 127 | 3 | 130 | 258 | 7 | 265 | 10 | 1 | 11 | 0 | 0 | 0 | 10 | 2 | 12 | 52 | 0 | 52 | 41 | 0 | 41 | 0 | 0 | 0 |
| PM Totals | 1143 | 16 | 1159 | 2224 | 60 | 2284 | 147 | 8 | 155 | 0 | 0 | 0 | 109 | 9 | 118 | 423 | 3 | 426 | 370 | 2 | 372 | 0 | 0 | 0 |


| Job No． | ：N790 |
| :--- | :--- |
| Client | ：Realty Realizations |
| Suburb | ：Nowra |
| Location | $:$ 7．Kalandar St／Princes Hwy |
|  |  |
| Day／Date | ：Fri，4th May 2012 |
| Weather | ：Fine |
| Description | ：Classified Intersection Count |
|  | $:$ Hourly Summary |



Princes Hwy

| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ |  |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\frac{\stackrel{7}{0}}{\underline{I}}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{1} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ |  | $\begin{aligned} & \text { ふ } \\ & \text { ふ } \\ & \text { ¹ } \end{aligned}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ |  |  | $\begin{aligned} & \overline{\mathrm{O}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { ふ } \\ & \text { 㐅} \\ & \text { © } \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{O}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { ふ } \\ & \text { § } \\ & \text { ² } \end{aligned}$ | ¢ |
| 7：00 to 8：00 | 1 | 0 | 1 | 487 | 54 | 541 | 26 | 3 | 29 | 0 | 0 | 0 | 36 | 3 | 39 | 162 | 2 | 164 | 322 | 5 | 327 | 0 | 0 | 0 |
| 7：15 to 8：15 | 2 | 0 | 2 | 604 | 46 | 650 | 30 | 1 | 31 | 0 | 0 | 0 | 32 | 2 | 34 | 174 | 3 | 177 | 405 | 8 | 413 | 0 | 0 | 0 |
| 7：30 to 8：30 | 3 | 0 | 3 | 669 | 50 | 719 | 25 | 1 | 26 | 0 | 0 | 0 | 33 | 1 | 34 | 213 | 3 | 216 | 499 | 14 | 513 | 0 | 0 | 0 |
| 7：45 to 8：45 | 3 | 0 | 3 | 733 | 40 | 773 | 30 | 1 | 31 | 0 | 0 | 0 | 33 | 3 | 36 | 248 | 3 | 251 | 598 | 16 | 614 | 0 | 0 | 0 |
| 8：00 to 9：00 | 3 | 0 | 3 | 764 | 45 | 809 | 32 | 0 | 32 | 0 | 0 | 0 | 29 | 3 | 32 | 252 | 8 | 260 | 644 | 19 | 663 | 0 | 0 | 0 |
| AM Totals | 4 | 0 | 4 | 1251 | 99 | 1350 | 58 | 3 | 61 | 0 | 0 | 0 | 65 | 6 | 71 | 414 | 10 | 424 | 966 | 24 | 990 | 0 | 0 | 0 |
| 16：00 to 17：00 | 8 | 0 | 8 | 679 | 18 | 697 | 73 | 1 | 74 | 0 | 0 | 0 | 66 | 1 | 67 | 161 | 4 | 165 | 375 | 7 | 382 | 0 | 0 | 0 |
| 16：15 to 17：15 | 4 | 0 | 4 | 672 | 16 | 688 | 71 | 3 | 74 | 0 | 0 | 0 | 68 | 1 | 69 | 161 | 5 | 166 | 373 | 4 | 377 | 0 | 0 | 0 |
| 16：30 to $17: 30$ | 4 | 0 | 4 | 618 | 14 | 632 | 63 | 2 | 65 | 0 | 0 | 0 | 72 | 1 | 73 | 166 | 5 | 171 | 395 | 4 | 399 | 0 | 0 | 0 |
| 16：45 to 17：45 | 4 | 0 | 4 | 619 | 17 | 636 | 56 | 2 | 58 | 0 | 0 | 0 | 67 | 1 | 68 | 169 | 3 | 172 | 388 | 2 | 390 | 0 | 0 | 0 |
| 17：00 to 18：00 | 5 | 0 | 5 | 574 | 15 | 589 | 55 | 2 | 57 | 0 | 0 | 0 | 67 | 1 | 68 | 180 | 2 | 182 | 400 | 2 | 402 | 0 | 0 | 0 |
| PM Totals | 13 | 0 | 13 | 1253 | 33 | 1286 | 128 | 3 | 131 | 0 | 0 | 0 | 133 | 2 | 135 | 341 | 6 | 347 | 775 | 9 | 784 | 0 | 0 | 0 |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{O}} \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \mathrm{I} \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{\circ}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{I}} \\ & \hline \mathrm{~J} \end{aligned}$ | $\begin{aligned} & \text { ス } \\ & \text { त⿹丁口 } \\ & \text { in } \end{aligned}$ | $\begin{gathered} \overline{\mathrm{O}} \\ \stackrel{-}{\circ} \end{gathered}$ | $\frac{\stackrel{\rightharpoonup}{\mathrm{V}}}{\underline{\mathrm{~J}}}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{V}} \\ & \stackrel{\rightharpoonup}{\mathrm{O}} \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{I}} \\ & \hline \mathrm{~J} \end{aligned}$ | $\begin{aligned} & \text { } \\ & \text { त } \\ & \frac{0}{x} \end{aligned}$ | ¢ |
| 7：00 to 8：00 | 113 | 13 | 126 | 548 | 59 | 607 | 66 | 6 | 72 | 0 | 0 | 0 | 22 | 7 | 29 | 48 | 10 | 58 | 80 | 1 | 81 | 0 | 0 | 0 |
| 7：15 to 8：15 | 140 | 10 | 150 | 642 | 54 | 696 | 75 | 8 | 83 | 0 | 0 | 0 | 28 | 7 | 35 | 57 | 6 | 63 | 95 | 5 | 100 | 0 | 0 | 0 |
| 7：30 to 8：30 | 151 | 8 | 159 | 709 | 49 | 758 | 101 | 12 | 113 | 0 | 0 | 0 | 35 | 7 | 42 | 71 | 5 | 76 | 109 | 6 | 115 | 0 | 0 | 0 |
| 7：45 to 8：45 | 176 | 11 | 187 | 708 | 50 | 758 | 116 | 11 | 127 | 0 | 0 | 0 | 36 | 7 | 43 | 87 | 2 | 89 | 125 | 5 | 130 | 0 | 0 | 0 |
| 8：00 to 9：00 | 204 | 16 | 220 | 700 | 55 | 755 | 118 | 15 | 133 | 0 | 0 | 0 | 38 | 6 | 44 | 125 | 2 | 127 | 125 | 7 | 132 | 0 | 0 | 0 |
| AM Totals | 317 | 29 | 346 | 1248 | 114 | 1362 | 184 | 21 | 205 | 0 | 0 | 0 | 60 | 13 | 73 | 173 | 12 | 185 | 205 | 8 | 213 | 0 | 0 | 0 |
| 16：00 to 17：00 | 554 | 6 | 560 | 1120 | 36 | 1156 | 88 | 5 | 93 | 0 | 0 | 0 | 67 | 7 | 74 | 206 | 0 | 206 | 202 | 2 | 204 | 0 | 0 | 0 |
| 16：15 to 17：15 | 553 | 1 | 554 | 1115 | 32 | 1147 | 74 | 4 | 78 | 0 | 0 | 0 | 68 | 4 | 72 | 218 | 0 | 218 | 186 | 1 | 187 | 0 | 0 | 0 |
| 16：30 to $17: 30$ | 581 | 3 | 584 | 1138 | 30 | 1168 | 66 | 3 | 69 | 0 | 0 | 0 | 52 | 3 | 55 | 216 | 1 | 217 | 177 | 1 | 178 | 0 | 0 | 0 |
| 16：45 to 17：45 | 610 | 7 | 617 | 1150 | 30 | 1180 | 66 | 2 | 68 | 0 | 0 | 0 | 45 | 0 | 45 | 217 | 3 | 220 | 165 | 0 | 165 | 0 | 0 | 0 |
| 17：00 to 18：00 | 589 | 10 | 599 | 1104 | 24 | 1128 | 59 | 3 | 62 | 0 | 0 | 0 | 42 | 2 | 44 | 217 | 3 | 220 | 168 | 0 | 168 | 0 | 0 | 0 |
| PM Totals | 1143 | 16 | 1159 | 2224 | 60 | 2284 | 147 | 8 | 155 | 0 | 0 | 0 | 109 | 9 | 118 | 423 | 3 | 426 | 370 | 2 | 372 | 0 | 0 | 0 |




|  |  |
| :--- | :--- |
| Job No. | $:$ N790 |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $:$ 8. Forest Rd / Coonamia Rd |
| Day/Date | $:$ Fri, 4th May 2012 |
| Weather | : Fine |
| Description | $:$ Classified Intersection Count |
|  | $: 15$ mins Data |



| 16:00 to 16:15 | 9 | 0 | 9 | 32 | 2 | 34 | 0 | 0 | 0 | 32 | 2 | 34 | 9 | 0 | 9 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16:15 to 16:30 | 9 | 0 | 9 | 32 | 2 | 34 | 0 | 0 | 0 | 26 | 1 | 27 | 7 | 1 | 8 | 0 | 0 | 0 |
| 16:30 to 16:45 | 4 | 0 | 4 | 28 | 0 | 28 | 0 | 0 | 0 | 15 | 1 | 16 | 3 | 0 | 3 | 0 | 0 | 0 |
| 16:45 to 17:00 | 3 | 0 | 3 | 28 | 1 | 29 | 0 | 0 | 0 | 14 | 1 | 15 | 3 | 0 | 3 | 0 | 0 | 0 |
| 17:00 to 17:15 | 6 | 0 | 6 | 35 | 0 | 35 | 0 | 0 | 0 | 18 | 0 | 18 | 2 | 0 | 2 | 0 | 0 | 0 |
| 17:15 to 17:30 | 4 | 0 | 4 | 42 | 0 | 42 | 0 | 0 | 0 | 19 | 0 | 19 | 4 | 0 | 4 | 0 | 0 | 0 |
| 17:30 to 17:45 | 9 | 0 | 9 | 25 | 0 | 25 | 0 | 0 | 0 | 16 | 1 | 17 | 4 | 0 | 4 | 0 | 0 | 0 |
| 17:45 to 18:00 | 5 | 0 | 5 | 33 | 0 | 33 | 0 | 0 | 0 | 15 | 0 | 15 | 5 | 0 | 5 | 0 | 0 | 0 |
| PM Totals | 49 | 0 | 49 | 255 | 5 | 260 | 0 | 0 | 0 | 155 | 6 | 161 | 37 | 1 | 38 | 0 | 0 | 0 |


| Job No. | : N790 |
| :--- | :--- |
| Client | : Realty Realizations |
| Suburb | : Nowra |
| Location | $:$ 8. Forest Rd / Coonamia Rd |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | : Fine |
| Description | : Classified Intersection Count |
|  | $:$ Hourly Summary |


: Hourly Summary




| Approach <br> Direction | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U <br> （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 <br> （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{gathered} \text { 历̈ } \\ \stackrel{\circ}{\circ} \end{gathered}$ | $\begin{aligned} & \frac{7}{0} \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\pi}{3} \\ & \text { © } \\ & \text { In } \end{aligned}$ | $\begin{aligned} & \overline{\amalg ٓ} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{5} \\ & \stackrel{O}{J} \end{aligned}$ | $\begin{aligned} & \underset{0}{\lambda} \\ & \text { ָin } \end{aligned}$ | $\begin{aligned} & \bar{\leftrightarrows} \\ & \stackrel{\rightharpoonup}{\circ} \\ & \hline \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\boxed{\circ}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\pi}{\square} \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\text { In }} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { ㄷㅡㅡㄹ } \\ & \hline 0 \end{aligned}$ |  | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $$ | $\begin{aligned} & \underset{\substack{3 \\ \mathbf{x} \\ \text { In }}}{ } \end{aligned}$ | $\begin{aligned} & \stackrel{\pi}{\circ} \\ & \stackrel{1}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & 3 \\ & \begin{array}{c} 3 \\ \text { I } \end{array} \end{aligned}$ | 픙 |
| 7：00 to 7：15 | 3 | 2 | 5 | 21 | 1 | 22 | 6 | 1 | 7 | 0 | 0 | 0 | 1 | 0 | 1 | 47 | 2 | 49 | 4 | 0 | 4 | 0 | 0 | 0 |
| 7：15 to 7：30 | 5 | 3 | 8 | 37 | 0 | 37 | 8 | 0 | 8 | 0 | 0 | 0 | 2 | 0 | 2 | 34 | 2 | 36 | 11 | 1 | 12 | 0 | 0 | 0 |
| 7：30 to 7：45 | 6 | 0 | 6 | 55 | 1 | 56 | 4 | 1 | 5 | 0 | 0 | 0 | 8 | 0 | 8 | 28 | 3 | 31 | 11 | 0 | 11 | 0 | 0 | 0 |
| 7：45 to 8：00 | 6 | 0 | 6 | 71 | 2 | 73 | 5 | 1 | 6 | 0 | 0 | 0 | 6 | 0 | 6 | 49 | 0 | 49 | 27 | 0 | 27 | 0 | 0 | 0 |
| 8：00 to 8：15 | 3 | 1 | 4 | 87 | 3 | 90 | 11 | 0 | 11 | 0 | 0 | 0 | 5 | 0 | 5 | 49 | 4 | 53 | 21 | 1 | 22 | 0 | 0 | 0 |
| 8：15 to 8：30 | 7 | 0 | 7 | 106 | 4 | 110 | 10 | 0 | 10 | 0 | 0 | 0 | 5 | 0 | 5 | 70 | 5 | 75 | 41 | 1 | 42 | 0 | 0 | 0 |
| 8：30 to 8：45 | 7 | 1 | 8 | 109 | 2 | 111 | 14 | 0 | 14 | 0 | 0 | 0 | 3 | 0 | 3 | 54 | 2 | 56 | 41 | 0 | 41 | 0 | 0 | 0 |
| 8：45 to 9：00 | 7 | 0 | 7 | 94 | 5 | 99 | 15 | 0 | 15 | 0 | 0 | 0 | 12 | 0 | 12 | 49 | 9 | 58 | 26 | 1 | 27 | 0 | 0 | 0 |
| AM Totals | 44 | 7 | 51 | 580 | 18 | 598 | 73 | 3 | 76 | 0 | 0 | 0 | 42 | 0 | 42 | 380 | 27 | 407 | 182 | 4 | 186 | 0 | 0 | 0 |
| 16：00 to 16：15 | 3 | 0 | 3 | 69 | 0 | 69 | 15 | 0 | 15 | 0 | 0 | 0 | 9 | 0 | 9 | 43 | 4 | 47 | 24 | 0 | 24 | 0 | 0 | 0 |
| 16：15 to $16: 30$ | 7 | 1 | 8 | 73 | 1 | 74 | 21 | 0 | 21 | 0 | 0 | 0 | 8 | 0 | 8 | 35 | 2 | 37 | 20 | 0 | 20 | 0 | 0 | 0 |
| 16：30 to $16: 45$ | 4 | 0 | 4 | 44 | 0 | 44 | 9 | 0 | 9 | 1 | 0 | 1 | 5 | 1 | 6 | 39 | 2 | 41 | 13 | 0 | 13 | 0 | 0 | 0 |
| 16：45 to 17：00 | 8 | 0 | 8 | 33 | 0 | 33 | 7 | 0 | 7 | 0 | 0 | 0 | 4 | 0 | 4 | 35 | 0 | 35 | 19 | 1 | 20 | 1 | 0 | 1 |
| 17：00 to 17：15 | 3 | 1 | 4 | 40 | 1 | 41 | 10 | 0 | 10 | 0 | 0 | 0 | 5 | 0 | 5 | 27 | 2 | 29 | 23 | 0 | 23 | 0 | 0 | 0 |
| 17：15 to 17：30 | 8 | 0 | 8 | 43 | 0 | 43 | 10 | 0 | 10 | 0 | 0 | 0 | 7 | 0 | 7 | 32 | 2 | 34 | 15 | 0 | 15 | 0 | 0 | 0 |
| 17：30 to 17：45 | 3 | 0 | 3 | 32 | 0 | 32 | 13 | 1 | 14 | 0 | 0 | 0 | 14 | 0 | 14 | 36 | 1 | 37 | 18 | 0 | 18 | 1 | 0 | 1 |
| 17：45 to 18：00 | 1 | 0 | 1 | 32 | 0 | 32 | 15 | 0 | 15 | 0 | 0 | 0 | 6 | 0 | 6 | 30 | 1 | 31 | 23 | 0 | 23 | 1 | 0 | 1 |
| PM Totals | 37 | 2 | 39 | 366 | 2 | 368 | 100 | 1 | 101 | 1 | 0 | 1 | 58 | 1 | 59 | 277 | 14 | 291 | 155 | 1 | 156 | 3 | 0 | 3 |


| Job No． | $:$ N790 |
| :--- | :--- |
| Client | ：Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $: 9$. Kalandar St／Kinghorne St |
|  |  |
| Day／Date | ：Fri，4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |

： 15 mins Data
 15 лериегея


SKYHIGH－THE TRAFFIC SURVEY COMPANY

| Approach | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 <br> （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 哥 } \\ & \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\circ}{1} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & .0 \end{aligned}$ |  | $\begin{gathered} \overline{Ð 5} \\ \stackrel{\circ}{\circ} \\ \hline \end{gathered}$ | $\frac{\text { 苛 }}{3}$ | $\begin{aligned} & \underset{\substack{\lambda \\ \\ ~}}{ } \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \sum_{\nwarrow}^{\lambda} \\ & \text { © } \end{aligned}$ | $\begin{aligned} & \overline{\ddagger 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\mathrm{IN}} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ | $\begin{aligned} & \gtrless_{0}^{7} \\ & \text { 오 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $$ |  | П |
| 7：00 to 7：15 | 9 | 1 | 10 | 2 | 0 | 2 | 110 | 1 | 111 | 0 | 0 | 0 | 19 | 0 | 19 | 15 | 5 | 20 | 0 | 1 | 1 | 0 | 0 | 0 |
| 7：15 to 7：30 | 15 | 1 | 16 | 2 | 0 | 2 | 101 | 1 | 102 | 0 | 0 | 0 | 27 | 1 | 28 | 10 | 2 | 12 | 2 | 0 | 2 | 0 | 0 | 0 |
| 7：30 to 7：45 | 12 | 1 | 13 | 4 | 0 | 4 | 42 | 1 | 43 | 0 | 0 | 0 | 27 | 2 | 29 | 21 | 3 | 24 | 1 | 0 | 1 | 0 | 0 | 0 |
| 7：45 to 8：00 | 18 | 0 | 18 | 6 | 0 | 6 | 52 | 2 | 54 | 0 | 0 | 0 | 33 | 0 | 33 | 21 | 3 | 24 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8：00 to 8：15 | 19 | 1 | 20 | 7 | 0 | 7 | 35 | 1 | 36 | 0 | 0 | 0 | 43 | 1 | 44 | 45 | 7 | 52 | 1 | 1 | 2 | 0 | 0 | 0 |
| 8：15 to 8：30 | 16 | 0 | 16 | 3 | 0 | 3 | 39 | 2 | 41 | 2 | 0 | 2 | 54 | 3 | 57 | 35 | 3 | 38 | 1 | 0 | 1 | 0 | 0 | 0 |
| 8：30 to 8：45 | 20 | 0 | 20 | 7 | 0 | 7 | 38 | 1 | 39 | 1 | 0 | 1 | 63 | 3 | 66 | 38 | 1 | 39 | 3 | 0 | 3 | 0 | 0 | 0 |
| 8：45 to 9：00 | 49 | 3 | 52 | 9 | 0 | 9 | 32 | 1 | 33 | 2 | 0 | 2 | 51 | 3 | 54 | 44 | 2 | 46 | 2 | 1 | 3 | 0 | 0 | 0 |
| AM Totals | 158 | 7 | 165 | 40 | 0 | 40 | 449 | 10 | 459 | 5 | 0 | 5 | 317 | 13 | 330 | 229 | 26 | 255 | 10 | 3 | 13 | 0 | 0 | 0 |


| 16：00 to 16：15 | 60 | 1 | 61 | 14 | 1 | 15 | 52 | 2 | 54 | 1 | 0 | 1 | 65 | 0 | 65 | 53 | 2 | 55 | 2 | 0 | 2 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16：15 to 16：30 | 48 | 0 | 48 | 15 | 0 | 15 | 49 | 3 | 52 | 1 | 0 | 1 | 58 | 1 | 59 | 61 | 1 | 62 | 3 | 0 | 3 | 0 | 0 | 0 |
| 16：30 to 16：45 | 47 | 1 | 48 | 16 | 1 | 17 | 45 | 2 | 47 | 0 | 0 | 0 | 42 | 1 | 43 | 61 | 1 | 62 | 4 | 0 | 4 | 0 | 0 | 0 |
| 16：45 to 17：00 | 55 | 0 | 55 | 7 | 0 | 7 | 52 | 0 | 52 | 0 | 0 | 0 | 35 | 0 | 35 | 41 | 0 | 41 | 1 | 0 | 1 | 0 | 0 | 0 |
| 17：00 to 17：15 | 75 | 0 | 75 | 16 | 0 | 16 | 42 | 2 | 44 | 0 | 0 | 0 | 50 | 2 | 52 | 56 | 1 | 57 | 3 | 0 | 3 | 0 | 0 | 0 |
| 17：15 to 17：30 | 46 | 0 | 46 | 12 | 0 | 12 | 53 | 2 | 55 | 2 | 0 | 2 | 40 | 0 | 40 | 42 | 0 | 42 | 4 | 0 | 4 | 0 | 0 | 0 |
| 17：30 to 17：45 | 49 | 0 | 49 | 9 | 0 | 9 | 53 | 1 | 54 | 0 | 0 | 0 | 31 | 1 | 32 | 50 | 1 | 51 | 0 | 0 | 0 | 0 | 0 | 0 |
| 17：45 to 18：00 | 39 | 0 | 39 | 10 | 0 | 10 | 35 | 1 | 36 | 0 | 0 | 0 | 46 | 1 | 47 | 37 | 1 | 38 | 1 | 0 | 1 | 0 | 0 | 0 |
| PM Totals | 419 | 2 | 421 | 99 | 2 | 101 | 381 | 13 | 394 | 4 | 0 | 4 | 367 | 6 | 373 | 401 | 7 | 408 | 18 | 0 | 18 | 0 | 0 | 0 |


| Job No． | $:$ N790 |
| :--- | :--- |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $: 9$. Kalandar St／Kinghorne St |
|  |  |
| Day／Date | ：Fri，4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |



SKYHIGH－THE TRAFFIC SURVEY COMPANY

| Approach | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\text { 号 }}{3} \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{0} \\ & \hline \end{aligned}$ |  | 픈 | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{O}} \\ & \stackrel{y}{\mathrm{O}} \end{aligned}$ | $\begin{aligned} & \text { ふ } \\ & \text { オ } \\ & \text { d } \end{aligned}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \underline{3} \end{aligned}$ |  | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\text { Z }}{\substack{x}} \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ |  |  | 픈 | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\text { Z }}{\substack{\text { I } \\ \hline}} \end{aligned}$ | ¢ |
| 7：00 to 8：00 | 20 | 5 | 25 | 184 | 4 | 188 | 23 | 3 | 26 | 0 | 0 | 0 | 17 | 0 | 17 | 158 | 7 | 165 | 53 | 1 | 54 | 0 | 0 | 0 |
| 7：15 to 8：15 | 20 | 4 | 24 | 250 | 6 | 256 | 28 | 2 | 30 | 0 | 0 | 0 | 21 | 0 | 21 | 160 | 9 | 169 | 70 | 2 | 72 | 0 | 0 | 0 |
| 7：30 to 8：30 | 22 | 1 | 23 | 319 | 10 | 329 | 30 | 2 | 32 | 0 | 0 | 0 | 24 | 0 | 24 | 196 | 12 | 208 | 100 | 2 | 102 | 0 | 0 | 0 |
| 7：45 to 8：45 | 23 | 2 | 25 | 373 | 11 | 384 | 40 | 1 | 41 | 0 | 0 | 0 | 19 | 0 | 19 | 222 | 11 | 233 | 130 | 2 | 132 | 0 | 0 | 0 |
| 8：00 to 9：00 | 24 | 2 | 26 | 396 | 14 | 410 | 50 | 0 | 50 | 0 | 0 | 0 | 25 | 0 | 25 | 222 | 20 | 242 | 129 | 3 | 132 | 0 | 0 | 0 |
| AM Totals | 44 | 7 | 51 | 580 | 18 | 598 | 73 | 3 | 76 | 0 | 0 | 0 | 42 | 0 | 42 | 380 | 27 | 407 | 182 | 4 | 186 | 0 | 0 | 0 |
| 16：00 to 17：00 | 22 | 1 | 23 | 219 | 1 | 220 | 52 | 0 | 52 | 1 | 0 | 1 | 26 | 1 | 27 | 152 | 8 | 160 | 76 | 1 | 77 | 1 | 0 | 1 |
| 16：15 to 17：15 | 22 | 2 | 24 | 190 | 2 | 192 | 47 | 0 | 47 | 1 | 0 | 1 | 22 | 1 | 23 | 136 | 6 | 142 | 75 | 1 | 76 | 1 | 0 | 1 |
| 16：30 to 17：30 | 23 | 1 | 24 | 160 | 1 | 161 | 36 | 0 | 36 | 1 | 0 | 1 | 21 | 1 | 22 | 133 | 6 | 139 | 70 | 1 | 71 | 1 | 0 | 1 |
| 16：45 to 17：45 | 22 | 1 | 23 | 148 | 1 | 149 | 40 | 1 | 41 | 0 | 0 | 0 | 30 | 0 | 30 | 130 | 5 | 135 | 75 | 1 | 76 | 2 | 0 | 2 |
| 17：00 to 18：00 | 15 | 1 | 16 | 147 | 1 | 148 | 48 | 1 | 49 | 0 | 0 | 0 | 32 | 0 | 32 | 125 | 6 | 131 | 79 | 0 | 79 | 2 | 0 | 2 |
| PM Totals | 37 | 2 | 39 | 366 | 2 | 368 | 100 | 1 | 101 | 1 | 0 | 1 | 58 | 1 | 59 | 277 | 14 | 291 | 155 | 1 | 156 | 3 | 0 | 3 |


| Approach | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ | $\frac{\stackrel{7}{0}}{\underline{I}}$ | $\begin{aligned} & 3 \\ & \begin{array}{l} \lambda \\ \\ \hline \end{array} \end{aligned}$ | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{V}} \\ & \hline \mathrm{~J} \end{aligned}$ |  | $\begin{gathered} \overline{\boxed{\circ}} \\ \stackrel{-}{\circ} \end{gathered}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{I}} \\ & \hline \mathrm{~J} \end{aligned}$ | $\begin{aligned} & \lambda_{\pi}^{\lambda} \\ & \text { N్土 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & 7 \\ & \substack{0 \\ \text { N } \\ \hline} \end{aligned}$ | $\begin{gathered} \overline{\boxed{0}} \\ \stackrel{-}{\circ} \end{gathered}$ | $\frac{\stackrel{\rightharpoonup}{\mathrm{I}}}{\underline{\mathrm{I}}}$ |  | $\begin{gathered} \overline{\boxed{O}} \\ \stackrel{-}{\circ} \end{gathered}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{I}} \\ & \hline \mathrm{~J} \end{aligned}$ |  | 든 |
| 7：00 to 8：00 | 54 | 3 | 57 | 14 | 0 | 14 | 305 | 5 | 310 | 0 | 0 | 0 | 106 | 3 | 109 | 67 | 13 | 80 | 3 | 1 | 4 | 0 | 0 | 0 |
| 7：15 to 8：15 | 64 | 3 | 67 | 19 | 0 | 19 | 230 | 5 | 235 | 0 | 0 | 0 | 130 | 4 | 134 | 97 | 15 | 112 | 4 | 1 | 5 | 0 | 0 | 0 |
| 7：30 to 8：30 | 65 | 2 | 67 | 20 | 0 | 20 | 168 | 6 | 174 | 2 | 0 | 2 | 157 | 6 | 163 | 122 | 16 | 138 | 3 | 1 | 4 | 0 | 0 | 0 |
| 7：45 to 8：45 | 73 | 1 | 74 | 23 | 0 | 23 | 164 | 6 | 170 | 3 | 0 | 3 | 193 | 7 | 200 | 139 | 14 | 153 | 5 | 1 | 6 | 0 | 0 | 0 |
| 8：00 to 9：00 | 104 | 4 | 108 | 26 | 0 | 26 | 144 | 5 | 149 | 5 | 0 | 5 | 211 | 10 | 221 | 162 | 13 | 175 | 7 | 2 | 9 | 0 | 0 | 0 |
| AM Totals | 158 | 7 | 165 | 40 | 0 | 40 | 449 | 10 | 459 | 5 | 0 | 5 | 317 | 13 | 330 | 229 | 26 | 255 | 10 | 3 | 13 | 0 | 0 | 0 |
| 16：00 to 17：00 | 210 | 2 | 212 | 52 | 2 | 54 | 198 | 7 | 205 | 2 | 0 | 2 | 200 | 2 | 202 | 216 | 4 | 220 | 10 | 0 | 10 | 0 | 0 | 0 |
| 16：15 to 17：15 | 225 | 1 | 226 | 54 | 1 | 55 | 188 | 7 | 195 | 1 | 0 | 1 | 185 | 4 | 189 | 219 | 3 | 222 | 11 | 0 | 11 | 0 | 0 | 0 |
| 16：30 to 17：30 | 223 | 1 | 224 | 51 | 1 | 52 | 192 | 6 | 198 | 2 | 0 | 2 | 167 | 3 | 170 | 200 | 2 | 202 | 12 | 0 | 12 | 0 | 0 | 0 |
| 16：45 to 17：45 | 225 | 0 | 225 | 44 | 0 | 44 | 200 | 5 | 205 | 2 | 0 | 2 | 156 | 3 | 159 | 189 | 2 | 191 | 8 | 0 | 8 | 0 | 0 | 0 |
| 17：00 to 18：00 | 209 | 0 | 209 | 47 | 0 | 47 | 183 | 6 | 189 | 2 | 0 | 2 | 167 | 4 | 171 | 185 | 3 | 188 | 8 | 0 | 8 | 0 | 0 | 0 |
| PM Totals | 419 | 2 | 421 | 99 | 2 | 101 | 381 | 13 | 394 | 4 | 0 | 4 | 367 | 6 | 373 | 401 | 7 | 408 | 18 | 0 | 18 | 0 | 0 | 0 |


|  |  |
| :--- | :--- |
| Job No. | : N790 |
| Client | : Realty Realizations |
| Suburb | : Nowra |
| Location | $:$ 10. Forest Rd / Princes Hwy |
|  | $:$ Fri, 4th May 2012 |
| Day/Date | : Fine |
| Weather |  |
| Description | $:$ Classified Intersection Count |
|  | $: 15$ mins Data |






| 16:00 to 16:15 | 20 | 0 | 20 | 359 | 10 | 369 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16:15 to 16:30 | 28 | 2 | 30 | 306 | 10 | 316 | 0 | 0 | 0 |
| 16:30 to 16:45 | 29 | 0 | 29 | 279 | 5 | 284 | 1 | 0 | 1 |
| 16:45 to 17:00 | 21 | 0 | 21 | 339 | 1 | 340 | 0 | 0 | 0 |
| 17:00 to 17:15 | 32 | 1 | 33 | 340 | 8 | 348 | 0 | 0 | 0 |
| 17:15 to 17:30 | 29 | 0 | 29 | 324 | 7 | 331 | 0 | 0 | 0 |
| 17:30 to 17:45 | 30 | 0 | 30 | 294 | 5 | 299 | 0 | 0 | 0 |
| 17:45 to 18:00 | 16 | 1 | 17 | 263 | 4 | 267 | 0 | 0 | 0 |
| PM Totals | 205 | 4 | 209 | 2504 | 50 | 2554 | 1 | 0 | 1 |


| Job No. | $:$ N790 |
| :--- | :--- |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $:$ 10. Forest Rd / Princes Hwy |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |






| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 <br> （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\text { IN }} \\ & \stackrel{0}{1} \\ & \hline \end{aligned}$ |  | $\begin{array}{r} 3 \\ \substack{3 \\ \text { I } \\ \hline \\ \hline} \end{array}$ | $\begin{aligned} & \bar{\Pi} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\text { IN }} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $$ | $\begin{aligned} & \underset{\text { J }}{\substack{\text { In }}} \end{aligned}$ |  |  | $\begin{array}{r} \underset{\pi}{3} \\ \stackrel{\pi}{I} \\ \hline \end{array}$ | $\begin{aligned} & \overline{\mathrm{O}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{array}{r} \substack{3 \\ \vdots \\ \text { 0 } \\ \hline \\ \hline} \end{array}$ | $\begin{aligned} & \text { Г̄ } \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $$ | $\begin{array}{r} 3 \\ \begin{array}{c} 3 \\ \text { 0 } \\ \hline \end{array} \\ \hline \end{array}$ | 픙 |
| 7：00 to 7：15 | 0 | 0 | 0 | 95 | 26 | 121 | 3 | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 3 | 13 | 0 | 13 | 6 | 0 | 6 | 0 | 0 | 0 |
| 7：15 to 7：30 | 2 | 0 | 2 | 138 | 11 | 149 | 5 | 1 | 6 | 0 | 0 | 0 | 2 | 0 | 2 | 5 | 0 | 5 | 19 | 5 | 24 | 0 | 0 | 0 |
| 7：30 to 7：45 | 0 | 0 | 0 | 145 | 22 | 167 | 4 | 1 | 5 | 0 | 0 | 0 | 4 | 0 | 4 | 18 | 0 | 18 | 36 | 0 | 36 | 0 | 0 | 0 |
| 7：45 to 8：00 | 0 | 0 | 0 | 153 | 12 | 165 | 9 | 0 | 9 | 0 | 0 | 0 | 1 | 0 | 1 | 33 | 0 | 33 | 23 | 0 | 23 | 0 | 0 | 0 |
| 8：00 to 8：15 | 0 | 0 | 0 | 143 | 16 | 159 | 12 | 2 | 14 | 0 | 0 | 0 | 5 | 0 | 5 | 37 | 3 | 40 | 42 | 1 | 43 | 0 | 0 | 0 |
| $8: 15$ to $8: 30$ | 0 | 0 | 0 | 188 | 15 | 203 | 22 | 3 | 25 | 0 | 0 | 0 | 4 | 1 | 5 | 41 | 2 | 43 | 42 | 4 | 46 | 0 | 0 | 0 |
| $8: 30$ to $8: 45$ | 3 | 0 | 3 | 202 | 13 | 215 | 47 | 0 | 47 | 0 | 0 | 0 | 8 | 0 | 8 | 48 | 3 | 51 | 66 | 5 | 71 | 0 | 0 | 0 |
| 8：45 to 9：00 | 0 | 0 | 0 | 169 | 14 | 183 | 48 | 0 | 48 | 0 | 0 | 0 | 11 | 0 | 11 | 59 | 0 | 59 | 58 | 0 | 58 | 0 | 0 | 0 |
| AM Totals | 5 | 0 | 5 | 1233 | 129 | 1362 | 150 | 7 | 157 | 0 | 0 | 0 | 38 | 1 | 39 | 254 | 8 | 262 | 292 | 15 | 307 | 0 | 0 | 0 |
| 16：00 to 16：15 | 2 | 0 | 2 | 250 | 8 | 258 | 21 | 0 | 21 | 1 | 0 | 1 | 5 | 0 | 5 | 28 | 0 | 28 | 51 | 2 | 53 | 0 | 0 | 0 |
| 16：15 to 16：30 | 1 | 0 | 1 | 216 | 8 | 224 | 22 | 0 | 22 | 0 | 0 | 0 | 6 | 1 | 7 | 31 | 0 | 31 | 37 | 0 | 37 | 0 | 0 | 0 |
| 16：30 to 16：45 | 0 | 0 | 0 | 273 | 11 | 284 | 21 | 0 | 21 | 0 | 0 | 0 | 5 | 0 | 5 | 23 | 0 | 23 | 34 | 1 | 35 | 0 | 0 | 0 |
| 16：45 to 17：00 | 4 | 0 | 4 | 222 | 4 | 226 | 13 | 0 | 13 | 0 | 0 | 0 | 6 | 0 | 6 | 19 | 0 | 19 | 31 | 0 | 31 | 0 | 0 | 0 |
| 17：00 to 17：15 | 0 | 0 | 0 | 272 | 4 | 276 | 9 | 0 | 9 | 0 | 0 | 0 | 2 | 0 | 2 | 18 | 0 | 18 | 28 | 1 | 29 | 0 | 0 | 0 |
| 17：15 to 17：30 | 1 | 0 | 1 | 239 | 3 | 242 | 21 | 0 | 21 | 0 | 0 | 0 | 1 | 0 | 1 | 11 | 0 | 11 | 27 | 0 | 27 | 0 | 0 | 0 |
| 17：30 to 17：45 | 1 | 0 | 1 | 217 | 6 | 223 | 11 | 0 | 11 | 1 | 0 | 1 | 2 | 0 | 2 | 25 | 0 | 25 | 32 | 0 | 32 | 0 | 0 | 0 |
| 17：45 to 18：00 | 1 | 0 | 1 | 194 | 6 | 200 | 7 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 0 | 15 | 25 | 0 | 25 | 0 | 0 | 0 |
| PM Totals | 10 | 0 | 10 | 1883 | 50 | 1933 | 125 | 0 | 125 | 2 | 0 | 2 | 27 | 1 | 28 | 170 | 0 | 170 | 265 | 4 | 269 | 0 | 0 | 0 |


： 15 mins Data
Princes Hwy

| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 <br> （Left Turn） |  |  | Direction 8 <br> （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 <br> （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period |  |  | $\begin{aligned} & \overline{\# 5} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{5} \\ & \stackrel{\circ}{1} \\ & \hline \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{5} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline . . \end{aligned}$ |  | $\begin{aligned} & \overline{5} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\mathrm{N}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $$ | $\begin{aligned} & \sum_{J}^{\gtrless} \\ & \text { © } \end{aligned}$ | $\begin{aligned} & \overline{5} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | \} | $\begin{aligned} & \overline{\mathrm{IN}} \\ & \stackrel{0}{1} \end{aligned}$ |  | \} $\\ {\substack{\text { ¢ } \\ \text { ¢ }}} \\ {\hline}$ | П |
| 7：00 to 7：15 | 9 | 1 | 10 | 214 | 22 | 236 | 34 | 2 | 36 | 0 | 0 | 0 | 10 | 0 | 10 | 0 | 0 | 0 | 2 | 1 | 3 | 0 | 0 | 0 |
| 7：15 to 7：30 | 6 | 1 | 7 | 220 | 16 | 236 | 27 | 2 | 29 | 0 | 0 | 0 | 13 | 1 | 14 | 7 | 1 | 8 | 5 | 1 | 6 | 0 | 0 | 0 |
| 7：30 to 7：45 | 14 | 1 | 15 | 247 | 15 | 262 | 57 | 3 | 60 | 0 | 0 | 0 | 16 | 2 | 18 | 7 | 0 | 7 | 5 | 2 | 7 | 0 | 0 | 0 |
| 7：45 to 8：00 | 23 | 1 | 24 | 282 | 19 | 301 | 82 | 4 | 86 | 0 | 0 | 0 | 16 | 0 | 16 | 10 | 1 | 11 | 7 | 0 | 7 | 0 | 0 | 0 |
| 8：00 to 8：15 | 27 | 0 | 27 | 271 | 17 | 288 | 72 | 0 | 72 | 0 | 0 | 0 | 21 | 4 | 25 | 20 | 0 | 20 | 6 | 2 | 8 | 0 | 0 | 0 |
| 8：15 to 8：30 | 43 | 2 | 45 | 289 | 23 | 312 | 74 | 3 | 77 | 0 | 0 | 0 | 30 | 3 | 33 | 25 | 2 | 27 | 9 | 1 | 10 | 0 | 0 | 0 |
| 8：30 to 8：45 | 39 | 4 | 43 | 261 | 15 | 276 | 73 | 5 | 78 | 0 | 0 | 0 | 20 | 2 | 22 | 34 | 2 | 36 | 14 | 2 | 16 | 0 | 0 | 0 |
| 8：45 to 9：00 | 42 | 2 | 44 | 296 | 27 | 323 | 91 | 4 | 95 | 0 | 0 | 0 | 25 | 2 | 27 | 28 | 0 | 28 | 8 | 0 | 8 | 0 | 0 | 0 |
| AM Totals | 203 | 12 | 215 | 2080 | 154 | 2234 | 510 | 23 | 533 | 0 | 0 | 0 | 151 | 14 | 165 | 131 | 6 | 137 | 56 | 9 | 65 | 0 | 0 | 0 |


| 16：00 to 16：15 | 28 | 1 | 29 | 267 | 15 | 282 | 81 | 1 | 82 | 0 | 0 | 0 | 85 | 0 | 85 | 40 | 0 | 40 | 20 | 0 | 20 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16：15 to 16：30 | 36 | 2 | 38 | 273 | 6 | 279 | 69 | 1 | 70 | 0 | 0 | 0 | 91 | 2 | 93 | 42 | 0 | 42 | 28 | 1 | 29 | 0 | 0 | 0 |
| 16：30 to 16：45 | 31 | 1 | 32 | 343 | 13 | 356 | 62 | 0 | 62 | 0 | 0 | 0 | 65 | 2 | 67 | 27 | 0 | 27 | 14 | 0 | 14 | 0 | 0 | 0 |
| 16：45 to 17：00 | 34 | 1 | 35 | 247 | 7 | 254 | 61 | 0 | 61 | 0 | 0 | 0 | 96 | 0 | 96 | 56 | 0 | 56 | 28 | 0 | 28 | 1 | 0 | 1 |
| 17：00 to 17：15 | 35 | 1 | 36 | 308 | 7 | 315 | 74 | 0 | 74 | 0 | 0 | 0 | 83 | 0 | 83 | 51 | 0 | 51 | 35 | 1 | 36 | 0 | 0 | 0 |
| 17：15 to 17：30 | 25 | 0 | 25 | 273 | 10 | 283 | 48 | 1 | 49 | 0 | 0 | 0 | 70 | 1 | 71 | 32 | 0 | 32 | 17 | 0 | 17 | 0 | 0 | 0 |
| 17：30 to 17：45 | 33 | 0 | 33 | 280 | 7 | 287 | 58 | 1 | 59 | 0 | 0 | 0 | 78 | 0 | 78 | 32 | 0 | 32 | 25 | 0 | 25 | 0 | 0 | 0 |
| 17：45 to 18：00 | 19 | 0 | 19 | 268 | 10 | 278 | 47 | 0 | 47 | 0 | 0 | 0 | 60 | 0 | 60 | 26 | 0 | 26 | 15 | 0 | 15 | 0 | 0 | 0 |
| PM Totals | 241 | 6 | 247 | 2259 | 75 | 2334 | 500 | 4 | 504 | 0 | 0 | 0 | 628 | 5 | 633 | 306 | 0 | 306 | 182 | 2 | 184 | 1 | 0 | 1 |


| Job No． | $:$ N790 |
| :--- | :--- |
| Client | ：Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $:$ 11．Moss St／Princes Hwy |
|  |  |
| Day／Date | ：Fri，4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |
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SKYHIGH－THE TRAFFIC SURVEY COMPANY

| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 <br> （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period |  |  | $\begin{aligned} & \overline{\amalg ँ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \mathrm{J} \end{aligned}$ |  | $\begin{aligned} & \overline{\amalg ँ} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{I}} \\ & \hline \mathrm{~J} \end{aligned}$ |  | $\begin{aligned} & \overline{\text { In }} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\frac{\text { 苛 }}{3}$ | $\begin{aligned} & \underset{\nwarrow}{\lambda} \\ & \stackrel{\text { x }}{\mathbf{I}} \end{aligned}$ | $\begin{aligned} & \overline{\text { In }} \\ & \stackrel{1}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\pi}{\grave{~}} \\ & \stackrel{y}{x} \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{-}{\circ} \end{aligned}$ | 苛 | $\begin{aligned} & {\underset{刃}{\mathbf{J}}}_{\substack{\text { I }}} \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{O}} \\ & \stackrel{-}{\circ} \end{aligned}$ |  | $\begin{aligned} & \underset{\pi}{\grave{~ N}} \\ & \stackrel{y}{I} \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $$ | 入 <br> ¢ <br> ¢ | \} |
| 7：00 to 8：00 | 2 | 0 | 2 | 531 | 71 | 602 | 21 | 2 | 23 | 0 | 0 | 0 | 10 | 0 | 10 | 69 | 0 | 69 | 84 | 5 | 89 | 0 | 0 | 0 |
| 7：15 to 8：15 | 2 | 0 | 2 | 579 | 61 | 640 | 30 | 4 | 34 | 0 | 0 | 0 | 12 | 0 | 12 | 93 | 3 | 96 | 120 | 6 | 126 | 0 | 0 | 0 |
| 7：30 to 8：30 | 0 | 0 | 0 | 629 | 65 | 694 | 47 | 6 | 53 | 0 | 0 | 0 | 14 | 1 | 15 | 129 | 5 | 134 | 143 | 5 | 148 | 0 | 0 | 0 |
| 7：45 to 8：45 | 3 | 0 | 3 | 686 | 56 | 742 | 90 | 5 | 95 | 0 | 0 | 0 | 18 | 1 | 19 | 159 | 8 | 167 | 173 | 10 | 183 | 0 | 0 | 0 |
| 8：00 to 9：00 | 3 | 0 | 3 | 702 | 58 | 760 | 129 | 5 | 134 | 0 | 0 | 0 | 28 | 1 | 29 | 185 | 8 | 193 | 208 | 10 | 218 | 0 | 0 | 0 |
| AM Totals | 5 | 0 | 5 | 1233 | 129 | 1362 | 150 | 7 | 157 | 0 | 0 | 0 | 38 | 1 | 39 | 254 | 8 | 262 | 292 | 15 | 307 | 0 | 0 | 0 |
| 16：00 to 17：00 | 7 | 0 | 7 | 961 | 31 | 992 | 77 | 0 | 77 | 1 | 0 | 1 | 22 | 1 | 23 | 101 | 0 | 101 | 153 | 3 | 156 | 0 | 0 | 0 |
| 16：15 to 17：15 | 5 | 0 | 5 | 983 | 27 | 1010 | 65 | 0 | 65 | 0 | 0 | 0 | 19 | 1 | 20 | 91 | 0 | 91 | 130 | 2 | 132 | 0 | 0 | 0 |
| 16：30 to 17：30 | 5 | 0 | 5 | 1006 | 22 | 1028 | 64 | 0 | 64 | 0 | 0 | 0 | 14 | 0 | 14 | 71 | 0 | 71 | 120 | 2 | 122 | 0 | 0 | 0 |
| 16：45 to 17：45 | 6 | 0 | 6 | 950 | 17 | 967 | 54 | 0 | 54 | 1 | 0 | 1 | 11 | 0 | 11 | 73 | 0 | 73 | 118 | 1 | 119 | 0 | 0 | 0 |
| 17：00 to 18：00 | 3 | 0 | 3 | 922 | 19 | 941 | 48 | 0 | 48 | 1 | 0 | 1 | 5 | 0 | 5 | 69 | 0 | 69 | 112 | 1 | 113 | 0 | 0 | 0 |
| PM Totals | 10 | 0 | 10 | 1883 | 50 | 1933 | 125 | 0 | 125 | 2 | 0 | 2 | 27 | 1 | 28 | 170 | 0 | 170 | 265 | 4 | 269 | 0 | 0 | 0 |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ |  |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ |  |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline 1 \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\frac{\text { 岢 }}{3}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ | $\frac{\stackrel{7}{0}}{\underline{I}}$ | $\begin{aligned} & 3 \\ & \begin{array}{l} \lambda \\ \frac{0}{1} \end{array} \end{aligned}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\frac{\stackrel{7}{0}}{\underline{I}}$ | $\begin{aligned} & \underset{\sim}{\lambda} \\ & \stackrel{\pi}{\top} \end{aligned}$ | $\begin{aligned} & \bar{\Pi} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | ふ ¢ ¢ ¢ | 工 |
| 7：00 to 8：00 | 52 | 4 | 56 | 963 | 72 | 1035 | 200 | 11 | 211 | 0 | 0 | 0 | 55 | 3 | 58 | 24 | 2 | 26 | 19 | 4 | 23 | 0 | 0 | 0 |
| 7：15 to 8：15 | 70 | 3 | 73 | 1020 | 67 | 1087 | 238 | 9 | 247 | 0 | 0 | 0 | 66 | 7 | 73 | 44 | 2 | 46 | 23 | 5 | 28 | 0 | 0 | 0 |
| 7：30 to 8：30 | 107 | 4 | 111 | 1089 | 74 | 1163 | 285 | 10 | 295 | 0 | 0 | 0 | 83 | 9 | 92 | 62 | 3 | 65 | 27 | 5 | 32 | 0 | 0 | 0 |
| 7：45 to 8：45 | 132 | 7 | 139 | 1103 | 74 | 1177 | 301 | 12 | 313 | 0 | 0 | 0 | 87 | 9 | 96 | 89 | 5 | 94 | 36 | 5 | 41 | 0 | 0 | 0 |
| 8：00 to 9：00 | 151 | 8 | 159 | 1117 | 82 | 1199 | 310 | 12 | 322 | 0 | 0 | 0 | 96 | 11 | 107 | 107 | 4 | 111 | 37 | 5 | 42 | 0 | 0 | 0 |
| AM Totals | 203 | 12 | 215 | 2080 | 154 | 2234 | 510 | 23 | 533 | 0 | 0 | 0 | 151 | 14 | 165 | 131 | 6 | 137 | 56 | 9 | 65 | 0 | 0 | 0 |
| 16：00 to 17：00 | 129 | 5 | 134 | 1130 | 41 | 1171 | 273 | 2 | 275 | 0 | 0 | 0 | 337 | 4 | 341 | 165 | 0 | 165 | 90 | 1 | 91 | 1 | 0 | 1 |
| 16：15 to 17：15 | 136 | 5 | 141 | 1171 | 33 | 1204 | 266 | 1 | 267 | 0 | 0 | 0 | 335 | 4 | 339 | 176 | 0 | 176 | 105 | 2 | 107 | 1 | 0 | 1 |
| 16：30 to 17：30 | 125 | 3 | 128 | 1171 | 37 | 1208 | 245 | 1 | 246 | 0 | 0 | 0 | 314 | 3 | 317 | 166 | 0 | 166 | 94 | 1 | 95 | 1 | 0 | 1 |
| 16：45 to 17：45 | 127 | 2 | 129 | 1108 | 31 | 1139 | 241 | 2 | 243 | 0 | 0 | 0 | 327 | 1 | 328 | 171 | 0 | 171 | 105 | 1 | 106 | 1 | 0 | 1 |
| 17：00 to 18：00 | 112 | 1 | 113 | 1129 | 34 | 1163 | 227 | 2 | 229 | 0 | 0 | 0 | 291 | 1 | 292 | 141 | 0 | 141 | 92 | 1 | 93 | 0 | 0 | 0 |
| PM Totals | 241 | 6 | 247 | 2259 | 75 | 2334 | 500 | 4 | 504 | 0 | 0 | 0 | 628 | 5 | 633 | 306 | 0 | 306 | 182 | 2 | 184 | 1 | 0 | 1 |


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| Job No. | : N790 |
| :--- | :--- |
| Client | : Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $:$ 1. Culburra Rd / Coonamia Rd |
|  |  |
| Day/Date | : Sat, 5th May 2012 |
| Weather | : Fine |
| Description | :Classified Intersection Count |
|  | $:$ Hourly Summary |



SKYHIGH - THE TRAFFIC SURVEY COMPANY




| Approach | Gulburra Rd |  |  |  |  |  |  |  |  | Mayfield Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 8 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 9 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | Direction 9U （U Turn） |  |  | $\begin{gathered} \hline \begin{array}{c} \text { Direction } 10 \\ \text { (Left Turn) } \end{array} \\ \hline \end{gathered}$ |  |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{array}{\|c\|} \hline \begin{array}{c} \text { Direction } 120 \\ \text { (U Turn) } \end{array} \\ \hline \end{array}$ |  |  |
| Time Period | 河 | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { di } \\ \hline \end{array}$ | $\begin{gathered} \overline{\mathrm{I}} \\ \stackrel{y}{\circ} \end{gathered}$ | $\begin{array}{\|c} \hline \mathbf{5} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\text { 㐅⿳亠丷厂犬土}} \\ \text { n } \end{array}$ | $\begin{array}{\|l\|} \hline \mathrm{I} \\ \hline 1 \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 总 } \end{array}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { à } \end{array}$ | $\begin{array}{\|l\|} \hline \mathrm{\Xi} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\begin{aligned} & \overline{\mathrm{g}} \\ & \stackrel{y}{\circ} \end{aligned}$ |  | $\begin{array}{\|c} \stackrel{\rightharpoonup}{9} \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \hline \mathrm{O} \end{array}$ | $\begin{array}{\|c} \stackrel{\rightharpoonup}{\mathrm{F}} \\ \hline \end{array}$ |  | $\stackrel{\text { ¢ }}{\stackrel{\text { ¢ }}{\circ}}$ |
| 12：00 10 12：15 | 47 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 | 0 | 1 | 0 | 0 | 0 |
| $12: 15$ 10 $12: 30$ | 45 | 0 | 45 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 |
| $12: 30$ 10 $12: 45$ <br> 1   <br> 1   | 57 | 0 | 57 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 | 0 | 1 | 0 | 0 | 0 |
| 12：45 to 13：00 | 39 | 1 | 40 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 |
| 13：00 10－13：15 | 44 | 1 | 45 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 | 0 | 1 | 0 | 0 | 0 |
| 13：15 to 13：30 | 53 | 0 | 53 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 |
| $13: 30$ to $13: 45$ | 60 | 0 | 60 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 | 0 | 1 | 0 | 0 | 0 |
| 13：45 to 14：00 | 46 | 0 | 46 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 | 0 | 1 | 0 | 0 | 0 |
| Totals | 391 | 2 | 393 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 5 | 0 | 5 | 0 | 0 | 0 |



| Approach | Gulburra Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction |  | ectio <br> eft T |  |  | $\begin{aligned} & \text { rection } \\ & \text { hroua } \end{aligned}$ |  |  | Tur |  |  |
| Time Period | $\begin{array}{\|l\|} \hline \text { 觅 } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \substack{\text { İ } \\ \text { İ工 } \\ \hline} \\ \hline \end{array}$ |  | $\begin{array}{\|c} \hline \text { 总 } \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \substack{3 \\ \text { xim }} \\ \hline \end{array}$ | $\frac{\overline{\mathrm{a}}}{\underline{\mathrm{o}}}$ | 营 |  | $\begin{aligned} & \stackrel{\overline{\mathrm{o}}}{\stackrel{1}{\circ}} \\ & \hline \end{aligned}$ |  |
| 12：00 to 13：00 | 2 | 0 | 2 | 209 | 1 | 210 | 0 | 0 | 0 |  |
| 12：15 to 13：15 | 2 | 0 | 2 | 204 | 0 | 204 | 0 | 0 | 0 |  |
| 12：30 to 13：30 | 2 | 0 | 2 | 174 | 0 | 174 | 0 | 0 | 0 |  |
| 12：45 to 13：45 | 1 | 0 | 1 | 139 | 1 | 140 | 0 | 0 | 0 |  |
| $13: 00$ to $14: 00$ | 1 | 0 | 1 | 139 | 1 | 140 | 0 | 0 | 0 |  |
| Totals | 3 | 0 | 3 | 348 | 2 | 350 | 0 | 0 | 0 |  |


| Approach | Gulburra Rd |  |  |  |  |  |  |  |  | Mayfield Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | $\begin{aligned} & \begin{array}{l} \text { Direction } 8 \\ \text { (Through) } \end{array} \end{aligned}$ |  |  | $\begin{gathered} \text { Direction } 9 \\ \text { (Right Turn) } \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \text { Direction } 10 \\ \text { (Left Turn) } \end{gathered}$ |  |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{gathered} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{array}{\|l\|} \hline \text { 吡 } \end{array}$ |  | $\begin{array}{\|l\|l\|} \hline \mathrm{I} \\ \hline 1 \end{array}$ | $\begin{array}{\|l\|} \hline \frac{5}{9} \\ \hline \end{array}$ | $\begin{aligned} & \text { 言 } \\ & \text { İ } \\ & \hline \end{aligned}$ |  | 皆 |  | $\begin{array}{\|l\|l\|} \hline \frac{\mathrm{⿺⿻}}{\mathrm{o}} \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 学 } \end{array}$ |  | $\begin{array}{\|l\|l\|} \hline \frac{\overline{\mathrm{g}}}{\mathrm{D}} \end{array}$ |  | $\begin{array}{\|l\|l\|} \hline \stackrel{\rightharpoonup}{9} \\ \hline \end{array}$ | $\begin{aligned} & \text { 䧺 } \\ & \text { In } \end{aligned}$ | $\begin{array}{\|l\|} \hline \mathrm{I} \\ \hline \mathrm{O} \end{array}$ | $\begin{array}{\|l\|l\|} \hline \text { 喜 } \\ \hline \end{array}$ | $\begin{aligned} & \text { 訔 } \\ & \text { İ } \end{aligned}$ | － |
| 12：00 to 13：00 | 188 | 1 | 189 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 2 | 0 | 2 | 0 | 0 | 0 |
| 12：15 to 13：15 | 185 | 2 | 187 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 2 | 0 | 2 | 0 | 0 | 0 |
| 12：30 to 13：30 | 193 | 2 | 195 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 2 | 0 | 2 | 0 | 0 | 0 |
| 12：45 to $13: 45$ | 196 | 2 | 198 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 2 | 0 | 2 | 0 | 0 | 0 |
| 13：00 to 14：00 | 203 | 1 | 204 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 3 | 0 | 3 | 0 | 0 | 0 |
| Totals | 391 | 2 | 393 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 5 | 0 | 5 | 0 | 0 | 0 |











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| Job No. | : N790 |
| :--- | :--- |
| Client | : Realty Realizations |
| Suburb | : Nowra |
| Location | $: 5$. Greenwell Point Rd / Mayfield Rd |
|  |  |
| Day/Date | : Sat, 5th May 2012 |
| Weather | : Fine |
| Description | : Classified Intersection Count |
|  | : Hourly Summary |



Mayfield Rd




|  |  |  | Millbank Rd |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Job No． | ：N790 |  | $94.98{ }^{8}{ }^{7}$ |  |  |
| Client | ：Realty Realizations | ¢ |  | 할 |  |
| Suburb | ：Nowra | ！ |  | ！ |  |
| Location | ：6．Greenwell Point Rd／Millbank Rd | $\frac{\bar{\circ}}{\stackrel{\circ}{0}}$ |  | $\begin{aligned} & \text { ò } \\ & \hline \overline{\mathrm{o}} \end{aligned}$ |  |
| Day／Date | ：Sat，5th May 2012 | 気 | 気 ${ }^{\text {c }}$ | ${ }_{0}$ | SKYHIGH－THE TRAFFIC SURUEY COMPANY |
| Weather | ：Fine | － |  | O |  |
| Description | ：Classified Intersection Count |  | $\left\|\begin{array}{llll}1 & 2 & 3 & 3 U\end{array}\right\|$ |  |  |
|  | ： 15 mins Data |  | Millbank Rd |  |  |


| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7（Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 9 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | Direction 9U （U Turn） |  |  | $\begin{gathered} \hline \begin{array}{c} \text { Direction } 10 \\ \text { (Left Turn) } \end{array} \\ \hline \end{gathered}$ |  |  | Direction 11 （Through） |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{gathered} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{array}{\|l\|l} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \frac{\mathrm{x}}{\mathrm{⿺}} \\ \hline \end{array}$ | 苛 |  | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \stackrel{1}{2} \end{array}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{5} \\ & \hline \mathrm{~J} \end{aligned}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\text { 㐅⿳亠丷厂犬土}} \\ \text { n } \end{array}$ | $\begin{array}{\|l\|} \hline \mathrm{I} \\ \hline 1 \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 总 } \end{array}$ |  | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \stackrel{1}{2} \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \frac{\mathrm{⿺⿻} 丷 冖 ⿱ 丶 万 ⿱ ⿰ ㇒ 一 乂 。 ~}{2} \end{array}$ | $\begin{aligned} & \text { 蒿 } \end{aligned}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { di } \end{array}$ | $\stackrel{\overline{\mathrm{I}}}{\stackrel{\mathrm{I}}{\circ}}$ | $\begin{array}{\|c} \stackrel{5}{9} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { an } \end{array}$ | $\begin{array}{\|l\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心.} \end{array}$ | 苛 |  | $\stackrel{\text { ¢ }}{\text { ¢ }}$ |
| 12：00 to 12：15 | 0 | 0 | 0 | 5 | 2 | 7 | 5 | 0 | 5 | 0 | 0 | 0 | 4 | 0 | 4 | 55 | 0 | 55 | 14 | 0 | 14 | 0 | 0 | 0 |
| $\begin{array}{lllll}12: 15 & \text { to } & 12: 30\end{array}$ | 2 | 0 | 2 | 2 | 0 | 2 | 4 | 0 | 4 | 0 | 0 | 0 | 1 | 0 | 1 | 52 | 0 | 52 | 7 | 0 | 7 | 0 | 0 | 0 |
| 12：30 to 12：45 | 0 | 0 | 0 | 3 | 0 | 3 | 2 | 0 | 2 | 0 | 0 | 0 | 3 | 1 | 4 | 47 | 2 | 49 | 8 | 0 | 8 | 0 | 0 | 0 |
| 12：45 to 13：00 | 3 | 1 | 4 | 6 | 0 | 6 | 1 | 0 | 1 | 0 | 0 | 0 | 7 | 0 | 7 | 36 | 0 | 36 | 12 | 0 | 12 | 0 | 0 | 0 |
| 13：00 to 13：15 | 1 | 0 | 1 | 2 | 0 | 2 | 2 | 0 | 2 | 0 | 0 | 0 | 3 | 0 | 3 | 42 | 1 | ${ }^{43}$ | 9 | 0 | 9 | 0 | 0 | 0 |
| 13：15 to 13：30 | 2 | 0 | 2 | 6 | 0 | 6 | 3 | 0 | 3 | 0 | 0 | 0 | 7 | 0 | 7 | 47 | 1 | 48 | 10 | 0 | 10 | 0 | 0 | 0 |
| 13：30 to 13：45 | 1 | 0 | 1 | 2 | 0 | 2 | 2 | 0 | 2 | 0 | 0 | 0 | 8 | 0 | 8 | 57 | 2 | 59 | 11 | 0 | 11 | 0 | 0 | 0 |
| 13：45 to 14：00 | 1 | 0 | 1 | 3 | 0 | 3 | 3 | 0 | 3 | 0 | 0 | 0 | 4 | 0 | 4 | 49 | 1 | 50 | 11 | 0 | 11 | 0 | 0 | 0 |
| Totals | 10 | 1 | 11 | 29 | 2 | 31 | 22 | 0 | 22 | 0 | 0 | 0 | 37 | 1 | 38 | 385 | 7 | 392 | 82 | 0 | 82 | 0 | 0 | 0 |


|  |  |  |  | Millbank Rd |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Job No． | ：N790 |  |  |  |  |
| Client | ：Realty Realizations | 『 | ${ }^{\mathrm{D}}$－ | $\uparrow \downarrow \downarrow$ ¢ | \％ |
| Suburb | ：Nowra | 号 | 家 | $\overrightarrow{+}$ | 䓂 |
| Location | ：6．Greenwell Point Rd／Millbank Rd | $\stackrel{\text { 잉 }}{ }$ | $\pm$ | $\uparrow$ | $\stackrel{\circ}{\circ}$ |
|  |  | $\stackrel{0}{3}$ | $\stackrel{\rightharpoonup}{\sim}$ | $\stackrel{+}{\square}$ | $\stackrel{0}{3}$ |
| Day／Date | Sat，5th May 2012 | む̀ | $\stackrel{\rightharpoonup}{\sim}$ | $\checkmark$ | む |
| Weather | Fine | － |  | $\dot{\nu}_{B}$ | 0 |
| Description | ：Classified Intersection Count |  |  | $1{ }_{1}^{1} 23300$ |  |
|  | Hourly Summary |  |  | Millbank Rd |  |


| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 （Through） |  |  | $\begin{gathered} \text { Direction } 3 \text { 3 } \\ \text { (Right Turn) } \end{gathered}$ |  |  | $\begin{gathered} \text { Direction 3U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 6 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 6U } \\ \text { (U Turn) } \end{gathered}$ |  |  |
| Time Period | 总 |  | $\begin{array}{\|l\|} \hline \stackrel{\text { ® }}{\mathrm{o}} \\ \hline \end{array}$ | 营 | $\begin{aligned} & 3 \\ & \begin{array}{l} \text { In } \\ \text { In } \\ \hline \end{array} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 厄⿳一巛口口口阝 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l} \hline \text { 寽 } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { In } \\ \hline \end{array} \\ \hline \end{array}$ |  | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 3 } \\ & \text { I⿷匚⿳ } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 厄⿳一巛口口口阝 } \\ & \hline \end{aligned}$ | 总 | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { In } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \stackrel{\text { 玉̈ }}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 喜 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline 3 \\ \text { 㙜 } \\ \hline \end{array}$ | $\begin{aligned} & \overline{\mathrm{o}} \\ & \stackrel{\rightharpoonup}{\mathrm{o}} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { In } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{array}{\|} \stackrel{\text { ® }}{\stackrel{\circ}{\circ}} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline 5 \\ \hline \end{array}$ |  | － |
| 12：00 to 13：00 | 58 | 0 | 58 | 21 | 1 | 22 | 28 | 1 | 29 | 0 | 0 | 0 | 14 | 1 | 15 | 181 | 2 | 183 | 12 | 0 | 12 | 0 | 0 | 0 |
| 12：15 to 13：15 | 45 | 0 | 45 | 21 | 1 | 22 | 28 | 1 | 29 | 0 | 0 | 0 | 15 | 1 | 16 | 177 | 0 | 177 | 13 | 0 | 13 | 0 | 0 | 0 |
| 12：30 10 13：30 | 41 | 0 | 41 | 27 | 1 | 28 | ${ }^{30}$ | 1 | 31 | 0 | 0 | 0 | 15 | 1 | 16 | 155 | 1 | 156 | 6 | 0 | 6 | 0 | 0 | 0 |
| 12：45 to $13: 45$ | 35 | 0 | 35 | 28 | 2 | 30 | 25 | 1 | 26 | 0 | 0 | 0 | 16 | 0 | 16 | 130 | 2 | 132 | 7 | 1 | 8 | 0 | 0 | 0 |
| 13：00 to 14：00 | 31 | 1 | 32 | 28 | 1 | 29 | 25 | 1 | 26 | 0 | 0 | 0 | 15 | 14 | 29 | 139 | 2 | 141 | 7 | 1 | 8 | 0 | 0 | 0 |
| Totals | 89 | 1 | 90 | 49 | 2 | 51 | 53 | 2 | 55 | 0 | 0 | 0 | 29 | 15 | 44 | 320 | 4 | 324 | 19 | 1 | 20 | 0 | 0 | 0 |


| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 <br> （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （ U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12 U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 入 } \\ & \text { 崖 } \end{aligned}$ |  | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ |  |  | 喜 |  |  | 总 |  |  | $\begin{array}{\|l\|l} \hline \text { 总 } \\ \hline \end{array}$ |  | $\begin{aligned} & \stackrel{\overline{\mathrm{g}}}{\stackrel{\circ}{\circ}} \end{aligned}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ |  | $\stackrel{\text { 항 }}{ }$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ |  |  | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 各 } \\ & \text { I⿷匚⿳ } \end{aligned}$ |  |
| 12：00 to 13：00 | 5 | 1 | 6 | 16 | 2 | 8 | 12 | 0 | 12 | 0 | 0 | 0 | 15 | 1 | 16 | 190 | 2 | 192 | 41 | 0 | 41 | 0 | 0 | 0 |
| 12：15 to 13：15 | 6 | 1 | 7 | 13 | 0 | 13 | 9 | 0 | 9 | 0 | 0 | 0 | 14 | 1 | 15 | 177 | 3 | 180 | 36 | 0 | 36 | 0 | 0 | 0 |
| 12：30 to 13：30 | 6 | 1 | 7 | 17 | 0 | 17 | 8 | 0 | 8 | 0 | 0 | 0 | 20 | 1 | 21 | 172 | 4 | 176 | 39 | 0 | 39 | 0 | 0 | 0 |
| 12：45 to 13：45 | 7 | 1 | 8 | 16 | 0 | 16 | 8 | 0 | 8 | 0 | 0 | 0 | 25 | 0 | 25 | 182 | 4 | 186 | 42 | 0 | 42 | 0 | 0 | 0 |
| 13：00 to 14：00 | 5 | 0 | 5 | 13 | 0 | 13 | 10 | 0 | 10 | 0 | 0 | 0 | 22 | 0 | 22 | 195 | 5 | 200 | 41 | 0 | 41 | 0 | 0 | 0 |
| Totals | 10 | 1 | 11 | 29 | 2 | 31 | 22 | 0 | 22 | 0 | 0 | 0 | 37 | 1 | 38 | 385 | 7 | 392 | 82 | 0 | 82 | 0 | 0 | 0 |



|  |  |  | Princes Hwy |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Job No． | ：N790 |  |  |  |  |
| Client | ：Realty Realizations |  | $\mathrm{D}_{\text {¢ }}$ ¢ $\downarrow \downarrow \downarrow \square$ |  |  |
| Suburb | ：Nowra | $\stackrel{\square}{\omega}$ | $\overrightarrow{\overrightarrow{0} \uparrow} \xrightarrow{+1}$ | あ | 7 |
| Location | ：7．Kalandar St／Princes Hwy | 䟴 | $\stackrel{\text { ！}}{\substack{\text { ¢ }}}$ | 年 | 2） |
| Day／Date | ：Sat，5th May 2012 | $\stackrel{\text { ¢ }}{\text { ¢ }}$ | $\overrightarrow{\text { co }}$ | $\stackrel{\text { ¢ }}{\text { ¢ }}$ | SKYHIGH－THE TRAFFIC SURUEY COMPANY |
| Weather | ：Fine |  | $\dot{\dot{*}}_{\text {B }}$ |  |  |
| Description | ：Classified Intersection Count |  | A $\left\|\begin{array}{llll}1 & 2 & 3 & 3 U\end{array}\right\|$ |  |  |
|  | ： 15 mins Data |  | Princes Hwy |  |  |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 <br> （Left Turn） |  |  | $\begin{aligned} & \text { Direction } 88 \\ & \text { (Through) } \end{aligned}$ |  |  | Direction 9 （Right Turn） |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction } 10 \\ \text { (Left Turn) } \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction } 11 \\ \text { (Through) } \\ \hline \end{gathered}$ |  |  | Direction 12 <br> （Right Turn） |  |  | $\begin{array}{\|c\|} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{array}$ |  |  |
| Time Period | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \begin{array}{c} \text { Iam } \\ \text { In } \end{array} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \hline \end{array}$ | $\begin{array}{\|l} \hline \frac{5}{9} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{c} \text { àd } \\ \text { an } \\ \hline \end{array} \\ \hline \end{array}$ | $\frac{\text { 镸 }}{}$ | $\begin{array}{\|l} \hline \mathrm{F} \\ \hline \mathrm{I} \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} \text { Ia } \\ \text { an } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{\sigma}} \\ \hline \end{array}$ | $\begin{array}{\|c} \hline \stackrel{\text { F }}{\mathrm{I}} \\ \hline \end{array}$ |  |  | $\begin{array}{\|l} \hline \frac{\mathrm{F}}{9} \\ \hline \end{array}$ | $\begin{array}{\|c} \substack{\stackrel{\rightharpoonup}{I} \\ \stackrel{\rightharpoonup}{I} \\ \hline} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \hline \end{array}$ | $\begin{aligned} & \text { 嘉 } \end{aligned}$ |  | $\frac{\text { 硕 }}{}$ | $\begin{array}{\|l} \hline \frac{5}{9} \\ \hline \end{array}$ |  |  | $\begin{array}{\|c} \stackrel{5}{5} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { dix } \\ \hline \end{array}$ | 产 |
| 12：00 to 12：15 | 98 | 0 | 98 | 253 | 2 | 255 | 12 | 2 | 14 | 0 | 0 | 0 | 10 | 0 | 10 | 40 | 0 | 40 | ${ }^{35}$ | 0 | 35 | 0 | 0 | 0 |
| 12：15 to 12：30 | 95 | 1 | 96 | 298 | 3 | 301 | 10 | 2 | 12 | 0 | 0 | 0 | 20 | 0 | 20 | 19 | 0 | 19 | 24 | 0 | 24 | 0 | 0 | 0 |
| 12：30 to 12：45 | 97 | 0 | 97 | 314 | 4 | 318 | 17 | 4 | 21 | 0 | 0 | 0 | 10 | 1 | 11 | 34 | 0 | 34 | 39 | 1 | 40 | 0 | 0 | 0 |
| 12：45 to 13：00 | 86 | 0 | 86 | 240 | 2 | 242 | 22 | 0 | 22 | 0 | 0 | 0 | 9 | 0 | 9 | 35 | 0 | 35 | 32 | 0 | 32 | 0 | 0 | 0 |
| 13：00 to 13：15 | ${ }^{112}$ | 0 | 112 | 231 | 2 | 233 | 24 | 1 | 25 | 0 | 0 | 0 | 6 | 0 | 6 | 42 | 0 | 42 | 26 | 0 | 26 | 0 | 0 | 0 |
| 13：15 to 13：30 | 79 | 0 | 79 | 275 | 8 | 283 | 27 | 0 | 27 | 0 | 0 | 0 | 7 | 1 | 8 | 34 | 0 | 34 | 36 | 2 | 38 | 0 | 0 | 0 |
| 13：30 to 13：45 | 94 | 1 | 95 | 253 | 3 | 256 | 11 | 0 | 11 | 0 | 0 | 0 | 10 | 0 | 10 | 25 | 1 | 26 | 27 | 0 | 27 | 0 | 0 | 0 |
| 13：45 to 14：00 | 84 | 2 | 86 | 271 | 2 | 273 | 17 | 0 | 17 | 0 | 0 | 0 | 7 | 1 | 8 | 21 | 0 | 21 | 29 | 1 | 30 | 0 | 0 | 0 |
| Totals | 745 | 4 | 749 | 2135 | 26 | 2161 | 140 | 9 | 149 | 0 | 0 | 0 | 79 | 3 | 82 | 250 | 1 | 251 | 248 | 4 | 252 | 0 | 0 | 0 |


|  |  |  |  | Princes Hwy |
| :---: | :---: | :---: | :---: | :---: |
| Job No． | ：N790 |  |  | 9U $9.9 .8 .{ }^{8}$ |
| Client | ：Realty Realizations |  | 今 | $\uparrow \downarrow \downarrow$ |
| Suburb | ：Nowra | あ | 䎡 |  |
| Location | ：7．Kalandar St／Princes Hwy | \％ | $\stackrel{+}{\square}$ | 4 ¢ |
|  |  |  | $\vec{\sim}$ | ＋${ }^{+}$ |
| Day／Date | ：Sat，5th May 2012 | $\stackrel{\text { ® }}{ }$ | 式 | $\downarrow$ |
| Weather | ：Fine |  |  |  |
| Description | ：Classified Intersection Count |  |  | $\left\|\begin{array}{llll}1 & 2 & 3 & 3 U\end{array}\right\|$ |
|  | Hourly Summary |  |  | Princes Hw |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 （Through） |  |  | $\begin{gathered} \begin{array}{c} \text { Direction } 3 \\ \text { (Right Turn) } \end{array} \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 3U } \\ \text { (U Turn) } \end{gathered}$ |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | Direction 6（Right Turn） |  |  | $\begin{gathered} \hline \text { Direction 6U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { 玉̈x } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \text { 등 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \frac{5}{9} \\ \hline \end{array}$ |  | $\begin{gathered} \stackrel{\text { ® }}{\stackrel{\circ}{6}} \\ \hline \end{gathered}$ | $\begin{array}{\|l} \hline \frac{\mathrm{F}}{9} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \text { B } \\ \text { 玉्工 } \\ \hline \end{array}$ | $\begin{aligned} & \text { 응 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l} \hline \text { E } \\ \hline \end{array}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \hline 1 \end{aligned}$ | $\begin{array}{\|l} \hline \frac{5}{9} \\ \hline \end{array}$ | $$ |  | 总 | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { 区 } \\ \hline \end{array} \\ \hline \end{array}$ |  | $\begin{array}{\|l\|l\|} \hline \text { 亳 } \\ \hline \end{array}$ |  | $\begin{array}{\|l} \stackrel{\text { ® }}{\stackrel{1}{6}} \\ \hline \end{array}$ | 总 | $\begin{array}{r} \text { 3 } \\ \text { 圌 } \\ \hline \end{array}$ | － |
| 12：00 to 13：00 | 3 | 1 | 4 | 753 | 20 | 773 | 84 | 4 | 88 | 0 | 0 | 0 | 68 | 0 | 68 | 159 | 1 | 160 | 409 | 4 | 413 | 0 | 0 | 0 |
| 12：15 to 13：15 | 6 | 1 | 7 | 724 | 16 | 740 | 92 | 3 | 95 | 0 | 0 | 0 | 64 | 0 | 64 | 134 | 1 | 135 | 377 | 3 | 380 | 0 | 0 | 0 |
| 12：30 to 13：30 | 4 | 0 | 4 | 699 | 12 | 711 | 95 | 2 | 97 | 0 | 0 | 0 | 61 | 0 | 61 | 122 | 1 | 123 | 332 | 3 | 335 | 0 | 0 | 0 |
| 12：45 to 13：45 | 7 | 0 | 7 | 695 | 9 | 704 | 87 | 1 | 88 | 0 | 0 | 0 | 57 | 0 | 57 | 129 | 1 | 130 | 341 | 1 | 342 | 0 | 0 | 0 |
| 13：00 to 14：00 | 7 | 0 | 7 | 681 | 7 | 688 | 73 | 0 | 73 | 0 | 0 | 0 | 53 | 0 | 53 | 113 | 2 | 115 | 326 | 0 | 326 | 0 | 0 | 0 |
| Totals | 10 | 1 | 11 | 1434 | 27 | 1461 | 157 | 4 | 161 | 0 | 0 | 0 | 121 | 0 | 121 | 272 | 3 | 275 | 735 | 4 | 739 | － | 0 | 0 |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{gathered} \begin{array}{c} \text { Direction } 9 \\ \text { (Right Turn) } \end{array} \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | Direction 10（Left Turn） （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） <br> （Right Turn） |  |  | $\begin{array}{\|c\|} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{array}$ |  |  |
| Time Period | $\begin{array}{\|l\|} \hline \text { 䜳 } \\ \hline \end{array}$ |  | $\stackrel{\text { 픙 }}{ }$ | $\begin{aligned} & \text { 吡 } \end{aligned}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { as } \\ \hline \end{array} \\ \hline \end{array}$ |  | $\begin{array}{\|l} \hline \text { 寽 } \\ \hline \end{array}$ |  | $\stackrel{\text { 高 }}{\circ}$ | $\begin{aligned} & \text { 畐 } \\ & \hline \end{aligned}$ | $$ | $\begin{array}{\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心.} \end{array}$ | $\begin{array}{\|l\|l\|} \hline \text { 吡 } \\ \hline \end{array}$ |  |  | $\begin{array}{\|l\|l\|} \hline \text { 吡 } \end{array}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { am } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \frac{\overline{\mathrm{g}}}{\mathrm{i}} \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|l} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \frac{\overline{\mathrm{x}}}{\mathrm{i}} \\ \hline \end{array}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { im } \\ \hline \end{array} \\ \hline \end{array}$ | $\stackrel{\text { ¢ }}{\text { ¢ }}$ |
| 12：00 to 13：00 | 376 | 1 | 377 | 1105 | 11 | 1116 | 61 | 8 | 69 | 0 | 0 | 0 | 49 | 1 | 50 | 128 | 0 | 128 | 130 | 1 | 131 | 0 | 0 | 0 |
| 12：15 to 13：15 | 390 | 1 | 391 | 1083 | 11 | 1094 | 73 | 7 | 80 | 0 | 0 | 0 | 45 | 1 | 46 | 130 | 0 | 130 | 121 | 1 | 122 | 0 | 0 | 0 |
| $\begin{array}{llll}12: 30 & 10 & 13: 30\end{array}$ | 374 | 0 | 374 | 1060 | 16 | 1076 | 90 | 5 | 95 | 0 | 0 | 0 | 32 | 2 | 34 | 145 | 0 | 145 | 133 | 3 | 136 | 0 | 0 | 0 |
| 12：45 to 13：45 | 371 | 1 | 372 | 999 | 15 | 1014 | 84 | 1 | 85 | 0 | 0 | 0 | 32 | 1 | 33 | 136 | 1 | 137 | 121 | 2 | 123 | 0 | 0 | 0 |
| 13：00 to 14：00 | 369 | 3 | 372 | 1030 | 15 | 1045 | 79 | 1 | 80 | 0 | 0 | 0 | 30 | 2 | 32 | 122 | 1 | 123 | 118 | 3 | 121 | 0 | 0 | 0 |
| Totals | 745 | 4 | 749 | 2135 | 26 | 2161 | 140 | 9 | 149 | 0 | 0 | 0 | 79 | 3 | 82 | 250 | 1 | 251 | 248 | 4 | 252 | 0 | 0 | 0 |






| Approach |  | Currarong Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction |  |  | Direction 5 (Through) |  |  | $\begin{gathered} \text { Direction } 6 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 6U } \\ \text { (U Turn) } \end{gathered}$ |  |  |
| Time Period |  |  | $\begin{aligned} & \text { 镸 } \\ & \hline \end{aligned}$ |  | $\begin{array}{\|l\|} \hline \stackrel{\text { Ï }}{\circ} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \text { 喜 } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { In } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \overline{\mathrm{\circ}} \mathrm{i} \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|l} \hline 5 \\ \hline \end{array}$ |  | ¢ |
| $12: 00$ 10 $13: 00$ |  |  | 8 | 0 | 8 | 20 | 0 | 20 | 0 | 0 | 0 |
| $12: 15$ to $13: 15$ <br> 12 lo  |  |  | 9 | 0 | 9 | 18 | 0 | 18 | 0 | 0 | 0 |
| $\begin{array}{llll}12: 30 & 10 & 13: 30\end{array}$ |  |  | 7 | 0 | 7 | 17 | 0 | 17 | 0 | 0 | 0 |
| 12:45 to $13: 45$ |  |  | 6 | 0 | 6 | 19 | 0 | 19 | 0 | 0 | 0 |
|  |  |  | 8 | 0 | 8 | 20 | 0 | 20 | 0 | 0 | 0 |
| Totals |  |  | 16 | 0 | 16 | 40 | 0 | 40 | 0 | 0 | 0 |





| Approach | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 9 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (UTurn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \begin{array}{c} \text { Direction } 10 \\ \text { (Left Turn) } \end{array} \\ \hline \end{gathered}$ |  |  | Direction 11 （Through） |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{gathered} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{array}{\|c} \hline \stackrel{\rightharpoonup}{\Xi} \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \mathrm{y} \\ \hline 1 \end{array}$ | 苛 | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { di } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \stackrel{1}{2} \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 吡 } \end{array}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\text { 㐅⿳亠丷厂犬土}} \\ \text { n } \end{array}$ | $\begin{array}{\|l\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心} \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 总 } \end{array}$ |  | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \stackrel{1}{2} \end{array}$ | $\begin{array}{\|c} \hline \stackrel{\rightharpoonup}{\Xi} \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \bar{\circ} \mathrm{I} \\ \hline \end{array}$ |  | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { di } \end{array}$ | $\stackrel{\overline{\mathrm{I}}}{\stackrel{\mathrm{I}}{\circ}}$ | $\begin{array}{\|c} \stackrel{5}{9} \\ \hline \end{array}$ | $\begin{aligned} & \text { ? } \\ & \text { Ï } \\ & \text { In } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心.} \end{array}$ |  |  | － |
| 12：00 to 12：15 | 32 | 1 | 33 | 3 | 0 | 3 | 21 | 1 | 22 | 1 | 0 | 1 | 24 | 0 | 24 | 41 | 0 | 41 | 2 | 1 | 3 | 1 | 0 | 1 |
| $\begin{array}{lllll}12: 15 & \text { to } & 12: 30\end{array}$ | 28 | 0 | 28 | 4 | 0 | 4 | 27 | 0 | 27 | 0 | 0 | 0 | 28 | 1 | 29 | 34 | 0 | 34 | 3 | 0 | 3 | 0 | 0 | 0 |
| 12：30 to 12：45 | 27 | 0 | 27 | 6 | 0 | 6 | 29 | 2 | 31 | 1 | 1 | 2 | 32 | 0 | 32 | 45 | 1 | 46 | 5 | 0 | 5 | 1 | 1 | 2 |
| 12：45 to 13：00 | 31 | 0 | 31 | 3 | 0 | 3 | 35 | 0 | 35 | 0 | 0 | 0 | 36 | 1 | 37 | 41 | 0 | 41 | 3 | 1 | 4 | 0 | 0 | 0 |
| 13：00 to 13：15 | 27 | 0 | 27 | 2 | 0 | 2 | 26 | 1 | 27 | 2 | 0 | 2 | 27 | 1 | 28 | 40 | 1 | 41 | 5 | 0 | 5 | 2 | 0 | 2 |
| 13：15 to 13：30 | 26 | 0 | 26 | 5 | 0 | 5 | 28 | 2 | 30 | 0 | 1 | 1 | 26 | 0 | 26 | ${ }^{33}$ | 1 | 34 | 4 | 0 | 4 | 0 | 1 | 1 |
| 13：30 to 13：45 | 26 | 0 | 26 | 3 | 0 | 3 | 33 | 0 | 33 | 2 | 0 | 2 | 25 | 1 | 26 | 29 | 1 | 30 | 3 | 0 | 3 | 2 | 0 | 2 |
| 13：45 to 14：00 | 27 | 1 | 28 | 3 | 0 | 3 | 29 | 1 | 30 | 1 | 0 | 1 | 30 | 1 | 31 | 27 | 1 | 28 | 5 | 1 | 6 | 1 | 0 | 1 |
| Totals | 224 | 2 | 226 | 29 | 0 | 29 | 228 | 7 | 235 | 7 | 2 | 9 | 228 | 5 | 233 | 290 | 5 | 295 | 30 | 3 | 33 | 7 | 2 | 9 |



| Approach | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 9 \\ \text { (Right Turn) } \end{gathered}$ |  |  | Direction 9U （U Turn） |  |  | $\begin{gathered} \hline \text { Direction } 10 \\ \text { (Left Turn) } \\ \hline \end{gathered}$ |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） <br> （Right Turn） |  |  | $\begin{array}{\|c\|} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{array}$ |  |  |
| Time Period | $\begin{array}{\|l\|} \hline \text { 䜳 } \\ \hline \end{array}$ |  | $\stackrel{\text { 玉َ }}{\stackrel{\rightharpoonup}{\circ}}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { as } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \mathrm{y} \\ \hline 1 \end{array}$ | $\begin{array}{\|l} \hline \frac{5}{9} \\ \hline \end{array}$ | $$ | $\stackrel{\text { 高 }}{\circ}$ | $\begin{array}{\|l\|} \hline \text { 总 } \\ \hline \end{array}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { an } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心.} \end{array}$ | $\begin{array}{\|l} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\stackrel{\text { 흥 }}{ }$ | $\begin{array}{\|l\|l\|} \hline \text { 吡 } \end{array}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { am } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \frac{\overline{\mathrm{g}}}{\mathrm{i}} \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \text { 寽 } \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \frac{\overline{\mathrm{x}}}{\mathrm{i}} \\ \hline \end{array}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $$ | 产 |
| 12：00 to 13：00 | 118 | 1 | 119 | 16 | 0 | 16 | 112 | 3 | 115 | 2 | 1 | 3 | 120 | 2 | 122 | 161 | 1 | 162 | 13 | 2 | 15 | 2 | 1 | 3 |
| 12：15 to 13：15 | 113 | 0 | 113 | 15 | 0 | 15 | 117 | 3 | 120 | 3 | 1 | 4 | 123 | 3 | 126 | 160 | 2 | 162 | 16 | 1 | 17 | 3 | 1 | 4 |
| 12：30 | 111 | 0 | 111 | 16 | 0 | 16 | 118 | 5 | 123 | 3 | 2 | 5 | 121 | 2 | 123 | 159 | 3 | 162 | 17 | 1 | 18 | 3 | 2 | 5 |
| 12：45 to 13：45 | 110 | 0 | 110 | 13 | 0 | 13 | 122 | 3 | 125 | 4 | 1 | 5 | 114 | 3 | 117 | 143 | 3 | 146 | 15 | 1 | 16 | 4 | 1 | 5 |
| 13：00 to 14：00 | 106 | 1 | 107 | 13 | 0 | 13 | 116 | 4 | 120 | 5 | 1 | 6 | 108 | 3 | 111 | 129 | 4 | 133 | 17 | 1 | 18 | 5 | 1 | 6 |
| Totals | 224 | 2 | 226 | 29 | 0 | 29 | 228 | 7 | 235 | 7 | 2 | 9 | 228 | 5 | 233 | 290 | 5 | 295 | 30 | 3 | 33 | 7 | 2 | 9 |





| Approach | Princes Hwy |  |  |  |  |  |  |  |  | Forest Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 2 （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  |  | $\begin{gathered} \hline \text { Direction } 6 \\ \text { (Right Turn) } \end{gathered}$ |  |  | Direction 6U （U Turn） |  |  |
| Time Period | 营 | $\begin{array}{\|l\|l\|} \substack{3 \\ \text { In } \\ \hline} \\ \hline \end{array}$ | 坒 | 总 |  |  | 夁 |  | 家 | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 㽞 } \\ & \underline{\text { n }} \end{aligned}$ |  | 宕 | 夁 |  | 年 | 苛 | $\begin{aligned} & \underline{\mathrm{ax}} \\ & \underline{1} \\ & \hline \end{aligned}$ | 产 |
| $\begin{array}{llll}12.00 & \text { to } & 13.00\end{array}$ | ${ }^{642}$ | 11 | 653 | 30 | 0 | \％ | 0 | 0 | － | 34 | 0 |  | 34 | 65 | 2 | 67 | 0 | 0 | 0 |
| 12.15 to 13.15 <br> 123   | 629 | 10 | 639 | 26 | 0 | 26 | 0 | 0 | － | 36 | 0 |  | 36 | 70 | 2 | 72 | 0 | 0 | 0 |
| $\begin{array}{lllll}1230 & \text { 10 } & 13.30\end{array}$ | 621 | 11 | 632 | 31 | 0 | 1 | 0 | 0 | － | 28 | 0 |  | 28 | 59 | 0 | 59 | 0 | 0 | 0 |
| 12.45 10 13：45 | 618 | 12 | 630 | 31 | 0 | 1 | 0 | 0 | 0 | 32 | 1 |  | 33 | 59 | 0 | 59 | 0 | 0 | 0 |
| 13.00 to 14.00 | 604 | 18 | 622 | 29 | 0 | 9 | 0 | 0 | 0 | 41 | 2 |  | 43 | 70 | 0 | 70 | 0 | 0 | 0 |
| Totals | 1246 | 29 | 1275 | 59 | 0 | 9 | 0 | 0 | 0 | 75 | 2 |  | 77 | 135 | 2 | 137 | 0 | 0 | 0 |




| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{array}{\|c} \hline \begin{array}{c} \text { Direction } 9 \\ \text { (Right Turn) } \end{array} \\ \hline \end{array}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{gathered} \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{array}{\|l\|l} \hline \frac{\mathrm{F}}{\mathrm{I}} \\ \hline \end{array}$ | $$ | $\begin{array}{\|l\|} \hline \mathrm{y} \\ \hline 1 \end{array}$ | $\begin{aligned} & \text { 总 } \end{aligned}$ |  | $\begin{gathered} \overline{\mathrm{g}} \mathrm{⿺} \\ \hline \end{gathered}$ | $\begin{array}{\|l} \hline \mathrm{F} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { ix } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{g}} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \stackrel{\rightharpoonup}{\mathrm{I}} \\ \hline \end{array}$ |  | $\begin{aligned} & \overline{\mathrm{g}} \\ & \hline 1 \end{aligned}$ | $\begin{array}{\|l} \hline \frac{5}{9} \\ \hline \end{array}$ | $\begin{array}{\|l} \hline \stackrel{3}{3} \\ \text { id } \\ \hline \end{array}$ | $\begin{array}{\|l} \overline{\mathrm{g}} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|l} \hline \text { 坒 } \end{array}$ | $\begin{array}{\|l\|} \hline \stackrel{\rightharpoonup}{3} \\ \text { à } \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \overline{\mathrm{I}} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathrm{g}} \\ \hline \end{array}$ | $\begin{aligned} & \text { ? } \\ & \text { İ } \\ & \text { İ } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{g}} \\ & \hline \stackrel{y}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|l\|} \hline ⿳ 亠 㐅 \\ \text { İ } \\ \text { In } \\ \hline \end{array}$ | $\stackrel{\text { ¢ }}{\text { ¢ }}$ |
| 12：00 10－12：15 | ${ }^{26}$ | 2 | 28 | ${ }^{288}$ | 6 | 294 | 47 | 0 | 47 | 0 | 0 | 0 | 61 | 0 | 61 | 41 | 0 | 41 | 21 | 0 | 21 | 0 | 0 | 0 |
| 12：15 to 12：30 | 21 | 0 | 21 | 245 | 5 | 250 | 65 | 2 | 67 | 0 | 0 | 0 | 58 | 0 | 58 | 35 | 0 | 35 | 22 | 0 | 22 | 0 | 0 | 0 |
| 12：30 to 12：45 | 20 | 0 | 20 | 271 | 2 | 273 | 44 | 0 | 44 | 0 | 0 | 0 | 57 | 0 | 57 | 17 | 0 | 17 | ${ }^{15}$ | 1 | 16 | 0 | 0 | 0 |
| 12：45 to 13：00 | 18 | 0 | 18 | 229 | 3 | 232 | 38 | 0 | 38 | 0 | 0 | 0 | 60 | 0 | 60 | 24 | 0 | 24 | 19 | 0 | 19 | 0 | 0 | 0 |
| 13：00 to 13：15 | 27 | 0 | 27 | 234 | 2 | 236 | 58 | 1 | 59 | 0 | 0 | 0 | 73 | 0 | 73 | 34 | 0 | 34 | 23 | 1 | 24 | 0 | 0 | 0 |
| $13: 15$ to $13: 30$ | 17 | 0 | 17 | 271 | 6 | 277 | 54 | 0 | 54 | 0 | 0 | 0 | 57 | 0 | 57 | 18 | 0 | 18 | 13 | 2 | 15 | 0 | 0 | 0 |
| 13：30 to 13：45 | 25 | 0 | 25 | 277 | 4 | 281 | 37 | 0 | 37 | 0 | 0 | 0 | 46 | 0 | 46 | 20 | 0 | 20 | 10 | 1 | 11 | 0 | 0 | 0 |
| 13：45 to 14：00 | 25 | 0 | 25 | 271 | 3 | 274 | 58 | 0 | 58 | 0 | 0 | 0 | 51 | 0 | 51 | 34 | 0 | 34 | 11 | 0 | 11 | 0 | 0 | 0 |
| Totals | 179 | 2 | 181 | 2086 | 31 | 2117 | 401 | 3 | 404 | 0 | 0 | 0 | 463 | 0 | 463 | 223 | 0 | 223 | 134 | 5 | 139 | 0 | 0 | 0 |



| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 <br> （Left Turn） |  |  | Direction 2 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 3 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | Direction 3 U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | $\begin{gathered} \text { Direction } 6 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 6U } \\ \text { (U Turn) } \end{gathered}$ |  |  |
| Time Period | 喜 |  |  | 皆 |  | $\stackrel{\text { 틍 }}{ }$ | $\begin{array}{\|l\|} \hline \text { 总 } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { 区 } \\ \text { In } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \stackrel{\text { II }}{\circ} \end{array}$ | 喜 | $\begin{array}{\|l\|} \hline \begin{array}{l} \text { İ } \\ \text { İ工 } \\ \hline \end{array} \\ \hline \end{array}$ | $\stackrel{\text { 틍 }}{ }$ | $\begin{array}{\|l\|} \hline \text { 寽 } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} \text { 3 } \\ \text { İ } \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \text { 厄⿳一巛口亍刂。 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \stackrel{\rightharpoonup}{\Xi} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \text { B } \\ \text { 区ix } \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \stackrel{\text { IIO}}{\circ} \\ \hline \end{array}$ | $\begin{array}{\|l} \hline \mathrm{F} \\ \hline \end{array}$ |  | $\begin{aligned} & \overline{\mathrm{g}} \\ & \stackrel{1}{\circ} \\ & \hline \end{aligned}$ | 总 | $\begin{array}{\|l\|l\|} \hline \begin{array}{l} 3 \\ \text { ax } \\ \hline \end{array} \\ \hline \end{array}$ | － |
| 12：00 to 13：00 | 9 | 0 | 9 | 915 | 25 | 940 | 79 | 0 | 79 | 0 | 0 | 0 | 24 | 0 | 24 | ${ }^{61}$ | 0 | 61 | 109 | 2 | 111 | 0 | 0 | 0 |
| 12：15 to 13：15 | 7 | 0 | 7 | 856 | 17 | 873 | 74 | 0 | 74 | 0 | 0 | 0 | 24 | 0 | 24 | 63 | 0 | 63 | 109 | 2 | 111 | 0 | 0 | 0 |
| 12：30 to 13：30 | 6 | 0 | 6 | 823 | 16 | 839 | 67 | 1 | 68 | 1 | 0 | 1 | 21 | 0 | 21 | 47 | 0 | 47 | 111 | 2 | 113 | 0 | 0 | 0 |
| 12：45 to 13：45 | 9 | 0 | 9 | 811 | 15 | 826 | 65 | 1 | 66 | 2 | 0 | 2 | 23 | 0 | 23 | 47 | 0 | 47 | 106 | 2 | 108 | 0 | 0 | 0 |
| 13：00 to 14：00 | 9 | 0 | 9 | 764 | 12 | 776 | 67 | 1 | 68 | 2 | 0 | 2 | 26 | 0 | 26 | 49 | 0 | 49 | 102 | 2 | 104 | 0 | 0 | 0 |
| Totals | 18 | 0 | 18 | 1679 | 37 | 1716 | 146 | 1 | 147 | 2 | 0 | 2 | 50 | 0 | 50 | 110 | 0 | 110 | 211 | 4 | 215 | 0 | 0 | 0 |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 <br> （Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{array}{c\|} \hline \text { Direction } 9 \\ \text { (Right Turn) } \end{array}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \text { Direction } 10 \\ \text { (Left Turn) } \\ \hline \end{gathered}$ |  |  | Direction 11 （Through） |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{gathered} \hline \text { Direction } 12 \mathrm{U} \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{array}{\|l\|l\|} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\stackrel{\text { 픙 }}{ }$ | $\begin{aligned} & \text { 皆 } \end{aligned}$ |  |  | $\begin{array}{\|l\|} \hline \text { 寽 } \\ \hline \end{array}$ | $$ | $\begin{array}{\|l\|} \hline \mathrm{I} \\ \hline 1 \end{array}$ | $\begin{array}{\|l\|} \hline \text { 总 } \\ \hline \end{array}$ |  | $\begin{aligned} & \overline{\mathrm{g}} \\ & \hline 1 \end{aligned}$ | $\begin{array}{\|l} \hline \mathbf{F} \\ \hline \end{array}$ | $$ | $\stackrel{\overline{\mathrm{g}}}{\stackrel{\mathrm{I}}{\circ}}$ | $\begin{array}{\|l\|l\|} \hline \text { 吡 } \\ \hline \end{array}$ | $$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 学 } \\ \hline \end{array}$ | $$ | $\begin{array}{\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心.} \\ \hline \end{array}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ |  | － |
| 12：00 to 13：00 | 85 | 2 | 87 | 1033 | 16 | 1049 | 194 | 2 | 196 | 0 | 0 | 0 | 236 | 0 | 236 | 117 | 0 | 117 | 77 | 1 | 78 | 0 | 0 | 0 |
| 12：15 to 13：15 | 86 | 0 | 86 | 979 | 12 | 991 | 205 | 3 | 208 | 0 | 0 | 0 | 248 | 0 | 248 | 110 | 0 | 110 | 79 | 2 | 81 | 0 | 0 | 0 |
| 12：30 | 82 | 0 | 82 | 1005 | 13 | 1018 | 194 | 1 | 195 | 0 | 0 | 0 | 247 | 0 | 247 | 93 | 0 | 93 | 70 | 4 | 74 | 0 | 0 | 0 |
| 12：45 to 13：45 | 87 | 0 | 87 | 1011 | 15 | 1026 | 187 | 1 | 188 | 0 | 0 | 0 | 236 | 0 | 236 | 96 | 0 | 96 | 65 | 4 | 69 | 0 | 0 | 0 |
| 13：00 to 14：00 | 94 | 0 | 94 | 1053 | 15 | 1068 | 207 | 1 | 208 | 0 | 0 | 0 | 227 | 0 | 227 | 106 | 0 | 106 | 57 | 4 | 61 | 0 | 0 | 0 |
| Totals | 179 | 2 | 181 | 2086 | 31 | 2117 | 401 | 3 | 404 | 0 | 0 | 0 | 463 | 0 | 463 | 223 | 0 | 223 | 134 | 5 | 139 | 0 | 0 | 0 |







| Job No. | : N790 |
| :--- | :--- |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $:$ 1. Culburra Rd / Coonamia Rd |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | : Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |








| Approach | Gulburra Rd |  |  |  |  |  |  |  |  | Mayfield Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  |  | Direction 12 <br> （Right Turn） |  | Direction 12U <br> （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{\circ}} \\ & \hline \end{aligned}$ | $$ |  | $\begin{aligned} & \text { ज口 } \\ & \stackrel{1}{0} \end{aligned}$ |  |  |  | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \text { ভ } \\ & \stackrel{\rightharpoonup}{\circ} \\ & \hline \end{aligned}$ | 皆 |  | $\begin{array}{r} \overline{\boxed{\circ}} \\ \stackrel{\rightharpoonup}{\circ} \\ \hline \end{array}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | ¢ |
| 7：00 to 7：15 | 11 | 9 | 20 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 3 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7：15 to $7: 30$ | 16 | 10 | 26 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7：30 to 7：45 | 12 | 1 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 |
| 7：45 to 8：00 | 18 | 1 | 19 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8：00 to 8：15 | 18 | 4 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8：15 to 8：30 | 28 | 1 | 29 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 |
| 8：30 to $8: 45$ | 34 | 1 | 35 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8：45 to 9：00 | 20 | 1 | 21 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 1 | 0 | 0 | 0 |
| AM Totals | 157 | 28 | 185 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 2 | 4 | 3 | 0 | 3 | 0 | 0 | 0 |
| 16：00 to 16：15 | 62 | 3 | 65 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 |
| 16：15 to 16：30 | 71 | 1 | 72 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 16：30 to 16：45 | 76 | 1 | 77 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 16：45 to 17：00 | 63 | 0 | 63 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 17：00 to 17：15 | 84 | 0 | 84 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 |


| 17:15 to 17:30 | 89 | 0 | 89 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 17:30 to 17:45 | 83 | 1 | 84 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 |
| 17:45 to 18:00 | 76 | 1 | 77 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| PM Totals | 604 | 7 | 611 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 1 | 3 | 0 | 0 | 0 |


| Job No. | : N790 |
| :--- | :--- |
| Client | : Realty Realizations |
| Suburb | : Nowra |
| Location | : 2. Gulburra Rd / Mayfield Rd |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | : Fine |
| Description | : Classified Intersection Count |
|  | : Hourly Summary |






SKYHIGH - THE TRAFFIC SURVEY COMPANY





| Job No. | : N790 |
| :--- | :--- |
| Client | : Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $: 3$. Greenwell Point Rd / Pyree Ln |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | : Fine |
| Description | : Classified Intersection Count |
|  | : Hourly Summary |



SKYHIGH - THE TRAFFIC SURVEY COMPANY






Description ：Classified Intersection Count
： 15 mins Data

| Approach | Jindy Andy Ln |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 <br> （Through） |  |  | Direction 12 U （U Turn） |  |  |
| Time Period | $$ | ふ <br> $\substack{\text { en } \\ \text { ¢ }}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\circ}{1} \\ & \hline \end{aligned}$ | 菏 |  | $\begin{aligned} & \overline{\oplus 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline 1 \end{aligned}$ |  | $\begin{aligned} & \overline{\oplus 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  |  | 董 | 入 <br> $\substack{0 \\ \text { ¢ }}$ | \} |
| 7：00 to 7：15 | 3 | 0 | 3 | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 1 | 2 | 15 | 17 | 32 | 0 | 0 | 0 |
| 7：15 to 7：30 | 5 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 2 | 20 | 14 | 34 | 0 | 0 | 0 |
| 7：30 to 7：45 | 1 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 16 | 3 | 19 | 0 | 0 | 0 |
| 7：45 to 8：00 | 3 | 0 | 3 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 22 | 5 | 27 | 0 | 0 | 0 |
| 8：00 to 8：15 | 8 | 0 | 8 | 3 | 1 | 4 | 0 | 0 | 0 | 2 | 0 | 2 | 19 | 5 | 24 | 0 | 0 | 0 |
| $8: 15$ to $8: 30$ | 5 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 32 | 0 | 32 | 0 | 0 | 0 |
| 8：30 to 8：45 | 10 | 1 | 11 | 1 | 0 | 1 | 0 | 0 | 0 | 2 | 2 | 4 | 33 | 2 | 35 | 0 | 0 | 0 |
| 8：45 to 9：00 | 9 | 0 | 9 | 3 | 2 | 5 | 1 | 0 | 1 | 0 | 1 | 1 | 21 | 3 | 24 | 0 | 0 | 0 |
| AM Totals | 44 | 2 | 46 | 8 | 4 | 12 | 1 | 0 | 1 | 7 | 6 | 13 | 178 | 49 | 227 | 0 | 0 | 0 |


| 16:00 to 16:15 | 27 | 1 | 28 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 65 | 2 | 67 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16:15 to 16:30 | 24 | 1 | 25 | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 1 | 2 | 67 | 0 | 67 | 0 | 0 | 0 |
| 16:30 to 16:45 | 28 | 0 | 28 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 73 | 3 | 76 | 0 | 0 | 0 |
| 16:45 to 17:00 | 27 | 0 | 27 | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 59 | 0 | 59 | 0 | 0 | 0 |
| 17:00 to 17:15 | 32 | 0 | 32 | 3 | 0 | 3 | 0 | 0 | 0 | 1 | 0 | 1 | 77 | 0 | 77 | 0 | 0 | 0 |
| 17:15 to 17:30 | 37 | 0 | 37 | 2 | 0 | 2 | 0 | 0 | 0 | 1 | 0 | 1 | 79 | 0 | 79 | 0 | 0 | 0 |
| 17:30 to 17:45 | 26 | 0 | 26 | 1 | 0 | 1 | 0 | 0 | 0 | 3 | 0 | 3 | 77 | 0 | 77 | 0 | 0 | 0 |
| 17:45 to 18:00 | 23 | 1 | 24 | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 64 | 2 | 66 | 0 | 0 | 0 |
| PM Totals | 224 | 3 | 227 | 9 | 0 | 9 | 0 | 0 | 0 | 14 | 1 | 15 | 561 | 7 | 568 | 0 | 0 | 0 |









| Job No. | : N790 |
| :--- | :--- |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $: 5$. Greenwell Point Rd / Mayfield Rd |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |



SKYHIGH - THE TRAFFIC SURVEY COMPANY


| Approach |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction |  |  | Direction 11 (Through) |  |  | Direction 12 (Right Turn) |  |  | Direction 12U (U Turn) |  |  |
| Time Period |  |  |  | ¢ | $\begin{aligned} & \overline{5} \\ & \stackrel{\circ}{1} \end{aligned}$ | 苛 | त § ¢ ¢ | ¢ |  | $\xrightarrow{\substack{\text { ¢ } \\ \text { ¢ }}}$ | 픈 |
| 7:00 to 8:00 |  |  | 75 | 41 | 116 | 3 | 0 | 3 | 0 | 0 | 0 |
| 7:15 to $8: 15$ |  |  | 79 | 28 | 107 | 3 | 0 | 3 | 0 | 0 | 0 |
| 7:30 to 8:30 |  |  | 92 | 13 | 105 | 2 | 0 | 2 | 0 | 0 | 0 |
| 7:45 to 8:45 |  |  | 110 | 12 | 122 | 4 | 0 | 4 | 0 | 0 | 0 |
| 8:00 to 9:00 |  |  | 109 | 11 | 120 | 5 | 0 | 5 | 0 | 0 | 0 |
| AM Totals |  |  | 184 | 52 | 236 | 8 | 0 | 8 | 0 | 0 | 0 |
| 16:00 to 17:00 |  |  | 274 | 4 | 278 | 10 | 0 | 10 | 0 | 0 | 0 |
| 16:15 to 17:15 |  |  | 285 | 2 | 287 | 12 | 0 | 12 | 0 | 0 | 0 |
| 16:30 to 17:30 |  |  | 292 | 2 | 294 | 9 | 0 | 9 | 0 | 0 | 0 |
| 16:45 to 17:45 |  |  | 302 | 0 | 302 | 10 | 0 | 10 | 0 | 0 | 0 |
| 17:00 to 18:00 |  |  | 300 | 2 | 302 | 10 | 0 | 10 | 0 | 0 | 0 |
| PM Totals |  |  | 574 | 6 | 580 | 20 | 0 | 20 | 0 | 0 | 0 |



| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 岩 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $$ | $\begin{aligned} & 3 \\ & \begin{array}{l} 3 \\ \frac{0}{x} \end{array} \end{aligned}$ | $\begin{aligned} & \overline{\text { In }} \\ & \text { ○ } \end{aligned}$ |  |  | $\begin{aligned} & \overline{\mathrm{W}} \\ & \stackrel{-}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 芌 } \\ & \stackrel{O}{J} \end{aligned}$ | $\begin{aligned} & 3 \\ & \begin{array}{c} 3 \\ \text { I } \end{array} \end{aligned}$ | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{\rightharpoonup}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 들 } \\ & \\ & \hline \end{aligned}$ | ¢ | ¢ |
| 7：00 to 7：15 | 6 | 0 | 6 | 4 | 2 | 6 | 0 | 2 | 2 | 0 | 0 | 0 | 5 | 0 | 5 | 47 | 2 | 49 | 7 | 0 | 7 | 0 | 0 | 0 |
| 7：15 to $7: 30$ | 7 | 1 | 8 | 4 | 0 | 4 | 1 | 1 | 2 | 0 | 0 | 0 | 5 | 0 | 5 | 43 | 0 | 43 | 8 | 1 | 9 | 0 | 0 | 0 |
| 7：30 to 7：45 | 9 | 1 | 10 | 2 | 0 | 2 | 3 | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 3 | 81 | 6 | 87 | 7 | 0 | 7 | 0 | 0 | 0 |
| 7：45 to 8：00 | 21 | 1 | 22 | 9 | 0 | 9 | 2 | 0 | 2 | 0 | 0 | 0 | 4 | 3 | 7 | 73 | 4 | 77 | 3 | 1 | 4 | 0 | 0 | 0 |
| 8：00 to 8：15 | 18 | 2 | 20 | 19 | 3 | 22 | 4 | 0 | 4 | 0 | 0 | 0 | 3 | 0 | 3 | 100 | 1 | 101 | 4 | 2 | 6 | 0 | 0 | 0 |
| 8：15 to 8：30 | 15 | 1 | 16 | 30 | 0 | 30 | 8 | 0 | 8 | 0 | 0 | 0 | 1 | 0 | 1 | 59 | 2 | 61 | 5 | 0 | 5 | 0 | 0 | 0 |
| $8: 30$ to $8: 45$ | 8 | 0 | 8 | 35 | 0 | 35 | 4 | 0 | 4 | 0 | 0 | 0 | 2 | 0 | 2 | 70 | 3 | 73 | 13 | 0 | 13 | 0 | 0 | 0 |
| 8：45 to 9：00 | 15 | 0 | 15 | 13 | 0 | 13 | 2 | 0 | 2 | 0 | 0 | 0 | 4 | 0 | 4 | 65 | 3 | 68 | 3 | 0 | 3 | 0 | 0 | 0 |
| AM Totals | 99 | 6 | 105 | 116 | 5 | 121 | 24 | 3 | 27 | 0 | 0 | 0 | 27 | 3 | 30 | 538 | 21 | 559 | 50 | 4 | 54 | 0 | 0 | 0 |
| 16：00 to 16：15 | 10 | 0 | 10 | 4 | 0 | 4 | 2 | 0 | 2 | 0 | 0 | 0 | 5 | 0 | 5 | 38 | 3 | 41 | 2 | 0 | 2 | 0 | 0 | 0 |
| 16：15 to $16: 30$ | 9 | 0 | 9 | 12 | 0 | 12 | 6 | 0 | 6 | 0 | 0 | 0 | 3 | 0 | 3 | 21 | 2 | 23 | 3 | 0 | 3 | 0 | 0 | 0 |
| 16：30 to 16：45 | 7 | 0 | 7 | 7 | 0 | 7 | 7 | 1 | 8 | 0 | 0 | 0 | 4 | 0 | 4 | 22 | 1 | 23 | 2 | 0 | 2 | 0 | 0 | 0 |
| 16：45 to 17：00 | 13 | 0 | 13 | 9 | 0 | 9 | 3 | 0 | 3 | 0 | 0 | 0 | 1 | 1 | 2 | 23 | 0 | 23 | 2 | 0 | 2 | 0 | 0 | 0 |
| 17：00 to 17：15 | 9 | 1 | 10 | 7 | 0 | 7 | 10 | 0 | 10 | 0 | 0 | 0 | 8 | 0 | 8 | 26 | 0 | 26 | 4 | 0 | 4 | 0 | 0 | 0 |
| 17：15 to 17：30 | 5 | 0 | 5 | 4 | 0 | 4 | 6 | 0 | 6 | 0 | 0 | 0 | 3 | 0 | 3 | 32 | 0 | 32 | 3 | 0 | 3 | 0 | 0 | 0 |
| 17：30 to 17：45 | 9 | 0 | 9 | 7 | 0 | 7 | 6 | 0 | 6 | 0 | 0 | 0 | 3 | 0 | 3 | 32 | 0 | 32 | 1 | 1 | 2 | 0 | 0 | 0 |
| 17：45 to 18：00 | 11 | 0 | 11 | 6 | 0 | 6 | 5 | 0 | 5 | 0 | 0 | 0 | 2 | 0 | 2 | 20 | 1 | 21 | 6 | 0 | 6 | 0 | 0 | 0 |
| PM Totals | 73 | 1 | 74 | 56 | 0 | 56 | 45 | 1 | 46 | 0 | 0 | 0 | 29 | 1 | 30 | 214 | 7 | 221 | 23 | 1 | 24 | 0 | 0 | 0 |

： 15 mins Data
Millbank Rd

| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 <br> （Right Turn） |  |  | Direction 12 U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ |  | $\begin{aligned} & \bar{\Pi} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{aligned} & \overline{\# ँ} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ |  | $\begin{aligned} & \overline{50} \\ & \stackrel{0}{1} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \text { IO } \end{aligned}$ |  | $\begin{aligned} & \overline{50} \\ & \text { 으 } \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\oplus 5} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ | $\frac{\stackrel{\rightharpoonup}{5}}{\underline{3}}$ |  | $\begin{aligned} & \bar{\Pi} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{IN}} \\ & \stackrel{0}{1} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | 듳 |
| 7：00 to 7：15 | 2 | 1 | 3 | 1 | 0 | 1 | 1 | 1 | 2 | 0 | 0 | 0 | 3 | 0 | 3 | 20 | 8 | 28 | 2 | 0 | 2 | 0 | 0 | 0 |
| 7：15 to 7：30 | 0 | 0 | 0 | 0 | 1 | 1 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 16 | 6 | 22 | 1 | 1 | 2 | 0 | 0 | 0 |
| 7：30 to 7：45 | 3 | 0 | 3 | 0 | 0 | 0 | 4 | 0 | 4 | 0 | 0 | 0 | 2 | 0 | 2 | 16 | 4 | 20 | 6 | 1 | 7 | 0 | 0 | 0 |
| 7：45 to 8：00 | 3 | 0 | 3 | 2 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 20 | 4 | 24 | 5 | 0 | 5 | 0 | 0 | 0 |
| 8：00 to 8：15 | 4 | 0 | 4 | 3 | 1 | 4 | 2 | 0 | 2 | 0 | 0 | 0 | 11 | 1 | 12 | 23 | 2 | 25 | 3 | 0 | 3 | 0 | 0 | 0 |
| 8：15 to 8：30 | 1 | 0 | 1 | 8 | 1 | 9 | 2 | 0 | 2 | 0 | 0 | 0 | 14 | 0 | 14 | 32 | 4 | 36 | 5 | 0 | 5 | 0 | 0 | 0 |
| 8：30 to 8：45 | 3 | 0 | 3 | 6 | 1 | 7 | 3 | 0 | 3 | 0 | 0 | 0 | 9 | 0 | 9 | 23 | 1 | 24 | 5 | 1 | 6 | 0 | 0 | 0 |
| 8：45 to 9：00 | 2 | 2 | 4 | 2 | 0 | 2 | 3 | 0 | 3 | 0 | 0 | 0 | 3 | 2 | 5 | 32 | 4 | 36 | 36 | 2 | 38 | 0 | 0 | 0 |
| AM Totals | 18 | 3 | 21 | 22 | 4 | 26 | 16 | 1 | 17 | 0 | 0 | 0 | 44 | 3 | 47 | 182 | 33 | 215 | 63 | 5 | 68 | 0 | 0 | 0 |


| 16：00 to 16：15 | 2 | 0 | 2 | 9 | 1 | 10 | 4 | 0 | 4 | 0 | 0 | 0 | 4 | 0 | 4 | 72 | 0 | 72 | 21 | 0 | 21 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16：15 to 16：30 | 5 | 0 | 5 | 9 | 0 | 9 | 3 | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 3 | 69 | 1 | 70 | 17 | 1 | 18 | 0 | 0 | 0 |
| 16：30 to 16：45 | 2 | 0 | 2 | 8 | 0 | 8 | 3 | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 3 | 67 | 1 | 68 | 19 | 0 | 19 | 0 | 0 | 0 |
| 16：45 to 17：00 | 6 | 0 | 6 | 9 | 0 | 9 | 2 | 0 | 2 | 0 | 0 | 0 | 1 | 0 | 1 | 66 | 0 | 66 | 19 | 0 | 19 | 0 | 0 | 0 |
| 17：00 to 17：15 | 4 | 0 | 4 | 8 | 0 | 8 | 2 | 0 | 2 | 0 | 0 | 0 | 4 | 0 | 4 | 64 | 1 | 65 | 21 | 0 | 21 | 0 | 0 | 0 |
| 17：15 to 17：30 | 2 | 0 | 2 | 8 | 0 | 8 | 4 | 0 | 4 | 0 | 0 | 0 | 2 | 0 | 2 | 83 | 0 | 83 | 22 | 1 | 23 | 0 | 0 | 0 |
| 17：30 to 17：45 | 2 | 0 | 2 | 8 | 0 | 8 | 4 | 0 | 4 | 0 | 0 | 0 | 1 | 0 | 1 | 71 | 5 | 76 | 15 | 0 | 15 | 0 | 0 | 0 |
| 17：45 to 18：00 | 4 | 0 | 4 | 6 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 57 | 0 | 57 | 17 | 0 | 17 | 0 | 0 | 0 |
| PM Totals | 27 | 0 | 27 | 65 | 1 | 66 | 22 | 0 | 22 | 0 | 0 | 0 | 21 | 0 | 21 | 549 | 8 | 557 | 151 | 2 | 153 | 0 | 0 | 0 |


| Job No． | $:$ N790 |
| :--- | :--- |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $: 6$. Greenwell Point Rd／Millbank Rd |
|  |  |
| Day／Date | ：Fri，4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |



SKYHICH－THE TRAFFIC SURVEY COMPANY

| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U <br> （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{aligned} & \bar{\Xi} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\frac{\stackrel{7}{0}}{\underline{I}}$ |  | $\begin{aligned} & \overline{๊ ๊} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{O}} \\ & \stackrel{y}{\mathrm{O}} \end{aligned}$ | $\begin{aligned} & \text { ふ } \\ & \text { オ } \\ & \text { d } \end{aligned}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \underline{3} \end{aligned}$ |  | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\text { Z }}{\substack{x}} \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline 1 \end{aligned}$ | $\begin{aligned} & \underset{\text { Z }}{\substack{\text { I }}} \end{aligned}$ | $\begin{gathered} \overline{\mathrm{I}} \\ \stackrel{-}{\circ} \end{gathered}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\text { Z }}{\substack{\text { I } \\ \hline}} \end{aligned}$ | ¢ |
| 7：00 to 8：00 | 43 | 3 | 46 | 19 | 2 | 21 | 6 | 3 | 9 | 0 | 0 | 0 | 17 | 3 | 20 | 244 | 12 | 256 | 25 | 2 | 27 | 0 | 0 | 0 |
| 7：15 to 8：15 | 55 | 5 | 60 | 34 | 3 | 37 | 10 | 1 | 11 | 0 | 0 | 0 | 15 | 3 | 18 | 297 | 11 | 308 | 22 | 4 | 26 | 0 | 0 | 0 |
| 7：30 to 8：30 | 63 | 5 | 68 | 60 | 3 | 63 | 17 | 0 | 17 | 0 | 0 | 0 | 11 | 3 | 14 | 313 | 13 | 326 | 19 | 3 | 22 | 0 | 0 | 0 |
| 7：45 to 8：45 | 62 | 4 | 66 | 93 | 3 | 96 | 18 | 0 | 18 | 0 | 0 | 0 | 10 | 3 | 13 | 302 | 10 | 312 | 25 | 3 | 28 | 0 | 0 | 0 |
| 8：00 to 9：00 | 56 | 3 | 59 | 97 | 3 | 100 | 18 | 0 | 18 | 0 | 0 | 0 | 10 | 0 | 10 | 294 | 9 | 303 | 25 | 2 | 27 | 0 | 0 | 0 |
| AM Totals | 99 | 6 | 105 | 116 | 5 | 121 | 24 | 3 | 27 | 0 | 0 | 0 | 27 | 3 | 30 | 538 | 21 | 559 | 50 | 4 | 54 | 0 | 0 | 0 |
| 16：00 to 17：00 | 39 | 0 | 39 | 32 | 0 | 32 | 18 | 1 | 19 | 0 | 0 | 0 | 13 | 1 | 14 | 104 | 6 | 110 | 9 | 0 | 9 | 0 | 0 | 0 |
| 16：15 to 17：15 | 38 | 1 | 39 | 35 | 0 | 35 | 26 | 1 | 27 | 0 | 0 | 0 | 16 | 1 | 17 | 92 | 3 | 95 | 11 | 0 | 11 | 0 | 0 | 0 |
| 16：30 to 17：30 | 34 | 1 | 35 | 27 | 0 | 27 | 26 | 1 | 27 | 0 | 0 | 0 | 16 | 1 | 17 | 103 | 1 | 104 | 11 | 0 | 11 | 0 | 0 | 0 |
| 16：45 to 17：45 | 36 | 1 | 37 | 27 | 0 | 27 | 25 | 0 | 25 | 0 | 0 | 0 | 15 | 1 | 16 | 113 | 0 | 113 | 10 | 1 | 11 | 0 | 0 | 0 |
| 17：00 to 18：00 | 34 | 1 | 35 | 24 | 0 | 24 | 27 | 0 | 27 | 0 | 0 | 0 | 16 | 0 | 16 | 110 | 1 | 111 | 14 | 1 | 15 | 0 | 0 | 0 |
| PM Totals | 73 | 1 | 74 | 56 | 0 | 56 | 45 | 1 | 46 | 0 | 0 | 0 | 29 | 1 | 30 | 214 | 7 | 221 | 23 | 1 | 24 | 0 | 0 | 0 |


| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline 1 \end{aligned}$ | $\begin{aligned} & 3 \\ & \begin{array}{l} 7 \\ \text { I } \end{array} \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\rightharpoonup}{1} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \underline{J} \end{aligned}$ | $\begin{aligned} & \sum_{\pi}^{入} \\ & \text { © } \end{aligned}$ | $\begin{aligned} & \overline{\boxed{\circ}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\frac{\stackrel{\rightharpoonup}{0}}{\underline{O}}$ |  | $\begin{gathered} \overline{\boxed{\circ}} \\ \stackrel{\circ}{\circ} \end{gathered}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \mathrm{I} \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \bar{Ð} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{V}} \\ & \stackrel{\rightharpoonup}{J} \end{aligned}$ | 入 <br> $\substack{\text { ¢ } \\ \text { ¢ }}$ <br>  | \} |
| 7：00 to 8：00 | 8 | 1 | 9 | 3 | 1 | 4 | 6 | 1 | 7 | 0 | 0 | 0 | 7 | 0 | 7 | 72 | 22 | 94 | 14 | 2 | 16 | 0 | 0 | 0 |
| 7：15 to 8：15 | 10 | 0 | 10 | 5 | 2 | 7 | 7 | 0 | 7 | 0 | 0 | 0 | 15 | 1 | 16 | 75 | 16 | 91 | 15 | 2 | 17 | 0 | 0 | 0 |
| 7：30 to 8：30 | 11 | 0 | 11 | 13 | 2 | 15 | 8 | 0 | 8 | 0 | 0 | 0 | 29 | 1 | 30 | 91 | 14 | 105 | 19 | 1 | 20 | 0 | 0 | 0 |
| 7：45 to 8：45 | 11 | 0 | 11 | 19 | 3 | 22 | 7 | 0 | 7 | 0 | 0 | 0 | 36 | 1 | 37 | 98 | 11 | 109 | 18 | 1 | 19 | 0 | 0 | 0 |
| 8：00 to 9：00 | 10 | 2 | 12 | 19 | 3 | 22 | 10 | 0 | 10 | 0 | 0 | 0 | 37 | 3 | 40 | 110 | 11 | 121 | 49 | 3 | 52 | 0 | 0 | 0 |
| AM Totals | 18 | 3 | 21 | 22 | 4 | 26 | 16 | 1 | 17 | 0 | 0 | 0 | 44 | 3 | 47 | 182 | 33 | 215 | 63 | 5 | 68 | 0 | 0 | 0 |
| 16：00 to 17：00 | 15 | 0 | 15 | 35 | 1 | 36 | 12 | 0 | 12 | 0 | 0 | 0 | 11 | 0 | 11 | 274 | 2 | 276 | 76 | 1 | 77 | 0 | 0 | 0 |
| 16：15 to 17：15 | 17 | 0 | 17 | 34 | 0 | 34 | 10 | 0 | 10 | 0 | 0 | 0 | 11 | 0 | 11 | 266 | 3 | 269 | 76 | 1 | 77 | 0 | 0 | 0 |
| 16：30 to 17：30 | 14 | 0 | 14 | 33 | 0 | 33 | 11 | 0 | 11 | 0 | 0 | 0 | 10 | 0 | 10 | 280 | 2 | 282 | 81 | 1 | 82 | 0 | 0 | 0 |
| 16：45 to 17：45 | 14 | 0 | 14 | 33 | 0 | 33 | 12 | 0 | 12 | 0 | 0 | 0 | 8 | 0 | 8 | 284 | 6 | 290 | 77 | 1 | 78 | 0 | 0 | 0 |
| 17：00 to 18：00 | 12 | 0 | 12 | 30 | 0 | 30 | 10 | 0 | 10 | 0 | 0 | 0 | 10 | 0 | 10 | 275 | 6 | 281 | 75 | 1 | 76 | 0 | 0 | 0 |
| PM Totals | 27 | 0 | 27 | 65 | 1 | 66 | 22 | 0 | 22 | 0 | 0 | 0 | 21 | 0 | 21 | 549 | 8 | 557 | 151 | 2 | 153 | 0 | 0 | 0 |



| Approach <br> Direction | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U <br> （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 <br> （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{gathered} \text { 历̈ } \\ \stackrel{\circ}{\circ} \end{gathered}$ | $$ |  | $\begin{aligned} & \bar{\leftrightarrows} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $$ | $\begin{aligned} & \underset{0}{\lambda} \\ & \text { ָin } \end{aligned}$ | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\boxed{\circ}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $$ |  | $\begin{aligned} & \text { 历̈ } \\ & \stackrel{\rightharpoonup}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { ㄷㅡㅡㄹ } \\ & \hline 0 \end{aligned}$ |  | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $$ | $\begin{aligned} & \underset{\substack{3 \\ \mathbf{x} \\ \text { In }}}{ } \end{aligned}$ | $\begin{aligned} & \stackrel{\pi}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & 3 \\ & \begin{array}{c} 3 \\ \text { I } \end{array} \end{aligned}$ | 픙 |
| 7：00 to 7：15 | 0 | 0 | 0 | 67 | 23 | 90 | 5 | 2 | 7 | 0 | 0 | 0 | 9 | 1 | 10 | 35 | 0 | 35 | 47 | 2 | 49 | 0 | 0 | 0 |
| 7：15 to 7：30 | 0 | 0 | 0 | 120 | 4 | 124 | 9 | 0 | 9 | 0 | 0 | 0 | 6 | 1 | 7 | 36 | 2 | 38 | 78 | 0 | 78 | 0 | 0 | 0 |
| 7：30 to 7：45 | 1 | 0 | 1 | 136 | 20 | 156 | 6 | 0 | 6 | 0 | 0 | 0 | 8 | 0 | 8 | 29 | 0 | 29 | 83 | 3 | 86 | 0 | 0 | 0 |
| 7：45 to 8：00 | 0 | 0 | 0 | 164 | 7 | 171 | 6 | 1 | 7 | 0 | 0 | 0 | 13 | 1 | 14 | 62 | 0 | 62 | 114 | 0 | 114 | 0 | 0 | 0 |
| 8：00 to 8：15 | 1 | 0 | 1 | 184 | 15 | 199 | 9 | 0 | 9 | 0 | 0 | 0 | 5 | 0 | 5 | 47 | 1 | 48 | 130 | 5 | 135 | 0 | 0 | 0 |
| 8：15 to 8：30 | 1 | 0 | 1 | 185 | 8 | 193 | 4 | 0 | 4 | 0 | 0 | 0 | 7 | 0 | 7 | 75 | 2 | 77 | 172 | 6 | 178 | 0 | 0 | 0 |
| 8：30 to 8：45 | 1 | 0 | 1 | 200 | 10 | 210 | 11 | 0 | 11 | 0 | 0 | 0 | 8 | 2 | 10 | 64 | 0 | 64 | 182 | 5 | 187 | 0 | 0 | 0 |
| 8：45 to 9：00 | 0 | 0 | 0 | 195 | 12 | 207 | 8 | 0 | 8 | 0 | 0 | 0 | 9 | 1 | 10 | 66 | 5 | 71 | 160 | 3 | 163 | 0 | 0 | 0 |
| AM Totals | 4 | 0 | 4 | 1251 | 99 | 1350 | 58 | 3 | 61 | 0 | 0 | 0 | 65 | 6 | 71 | 414 | 10 | 424 | 966 | 24 | 990 | 0 | 0 | 0 |
| 16：00 to 16：15 | 5 | 0 | 5 | 163 | 6 | 169 | 18 | 0 | 18 | 0 | 0 | 0 | 18 | 1 | 19 | 44 | 0 | 44 | 101 | 3 | 104 | 0 | 0 | 0 |
| 16：15 to $16: 30$ | 0 | 0 | 0 | 184 | 4 | 188 | 22 | 1 | 23 | 0 | 0 | 0 | 15 | 0 | 15 | 42 | 1 | 43 | 88 | 1 | 89 | 0 | 0 | 0 |
| 16：30 to $16: 45$ | 1 | 0 | 1 | 161 | 2 | 163 | 22 | 0 | 22 | 0 | 0 | 0 | 18 | 0 | 18 | 35 | 2 | 37 | 98 | 3 | 101 | 0 | 0 | 0 |
| 16：45 to 17：00 | 2 | 0 | 2 | 171 | 6 | 177 | 11 | 0 | 11 | 0 | 0 | 0 | 15 | 0 | 15 | 40 | 1 | 41 | 88 | 0 | 88 | 0 | 0 | 0 |
| 17：00 to 17：15 | 1 | 0 | 1 | 156 | 4 | 160 | 16 | 2 | 18 | 0 | 0 | 0 | 20 | 1 | 21 | 44 | 1 | 45 | 99 | 0 | 99 | 0 | 0 | 0 |
| 17：15 to 17：30 | 0 | 0 | 0 | 130 | 2 | 132 | 14 | 0 | 14 | 0 | 0 | 0 | 19 | 0 | 19 | 47 | 1 | 48 | 110 | 1 | 111 | 0 | 0 | 0 |
| 17：30 to 17：45 | 1 | 0 | 1 | 162 | 5 | 167 | 15 | 0 | 15 | 0 | 0 | 0 | 13 | 0 | 13 | 38 | 0 | 38 | 91 | 1 | 92 | 0 | 0 | 0 |
| 17：45 to 18：00 | 3 | 0 | 3 | 126 | 4 | 130 | 10 | 0 | 10 | 0 | 0 | 0 | 15 | 0 | 15 | 51 | 0 | 51 | 100 | 0 | 100 | 0 | 0 | 0 |
| PM Totals | 13 | 0 | 13 | 1253 | 33 | 1286 | 128 | 3 | 131 | 0 | 0 | 0 | 133 | 2 | 135 | 341 | 6 | 347 | 775 | 9 | 784 | 0 | 0 | 0 |



| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 <br> （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 <br> （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 鹍 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \sum_{J}^{\gtrless} \\ & \text { din } \end{aligned}$ | $\begin{aligned} & \text { 픈 } \\ & \hline \end{aligned}$ |  |  | $\begin{aligned} & \bar{\cong} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline .0 \end{aligned}$ | $\begin{aligned} & \text { 入 } \\ & \text { 조 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{50} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ | $\begin{aligned} & \text { 入 } \\ & \text { 조 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{5} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ | $\begin{aligned} & \sum_{J}^{7} \\ & \frac{0}{1} \end{aligned}$ | $\begin{aligned} & \overline{\ddagger 5} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ | $\begin{aligned} & \text { 入 } \\ & \text { 조 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \bar{\cong} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline .0 \end{aligned}$ | ス <br> $\substack{\text { ¢ } \\ \text { ¢ }}$ | $\begin{aligned} & \overline{5} \\ & \stackrel{\circ}{1} \\ & \hline \end{aligned}$ | $\frac{\text { 苛 }}{3}$ | 橘 | ¢ |
| 7：00 to 7：15 | 22 | 5 | 27 | 101 | 15 | 116 | 18 | 2 | 20 | 0 | 0 | 0 | 2 | 2 | 4 | 11 | 5 | 16 | 19 | 0 | 19 | 0 | 0 | 0 |
| 7：15 to 7：30 | 29 | 4 | 33 | 118 | 17 | 135 | 9 | 1 | 10 | 0 | 0 | 0 | 3 | 2 | 5 | 12 | 1 | 13 | 19 | 0 | 19 | 0 | 0 | 0 |
| 7：30 to 7：45 | 21 | 2 | 23 | 151 | 15 | 166 | 18 | 3 | 21 | 0 | 0 | 0 | 11 | 1 | 12 | 11 | 3 | 14 | 14 | 1 | 15 | 0 | 0 | 0 |
| 7：45 to 8：00 | 41 | 2 | 43 | 178 | 12 | 190 | 21 | 0 | 21 | 0 | 0 | 0 | 6 | 2 | 8 | 14 | 1 | 15 | 28 | 0 | 28 | 0 | 0 | 0 |
| 8：00 to 8：15 | 49 | 2 | 51 | 195 | 10 | 205 | 27 | 4 | 31 | 0 | 0 | 0 | 8 | 2 | 10 | 20 | 1 | 21 | 34 | 4 | 38 | 0 | 0 | 0 |
| 8：15 to 8：30 | 40 | 2 | 42 | 185 | 12 | 197 | 35 | 5 | 40 | 0 | 0 | 0 | 10 | 2 | 12 | 26 | 0 | 26 | 33 | 1 | 34 | 0 | 0 | 0 |
| 8：30 to 8：45 | 46 | 5 | 51 | 150 | 16 | 166 | 33 | 2 | 35 | 0 | 0 | 0 | 12 | 1 | 13 | 27 | 0 | 27 | 30 | 0 | 30 | 0 | 0 | 0 |
| 8：45 to 9：00 | 69 | 7 | 76 | 170 | 17 | 187 | 23 | 4 | 27 | 0 | 0 | 0 | 8 | 1 | 9 | 52 | 1 | 53 | 28 | 2 | 30 | 0 | 0 | 0 |
| AM Totals | 317 | 29 | 346 | 1248 | 114 | 1362 | 184 | 21 | 205 | 0 | 0 | 0 | 60 | 13 | 73 | 173 | 12 | 185 | 205 | 8 | 213 | 0 | 0 | 0 |


| 16：00 to 16：15 | 150 | 6 | 156 | 289 | 8 | 297 | 29 | 2 | 31 | 0 | 0 | 0 | 15 | 3 | 18 | 55 | 0 | 55 | 61 | 1 | 62 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16：15 to 16：30 | 128 | 0 | 128 | 259 | 8 | 267 | 20 | 1 | 21 | 0 | 0 | 0 | 24 | 1 | 25 | 49 | 0 | 49 | 53 | 0 | 53 | 0 | 0 | 0 |
| 16：30 to 16：45 | 128 | 0 | 128 | 268 | 7 | 275 | 22 | 2 | 24 | 0 | 0 | 0 | 15 | 3 | 18 | 50 | 0 | 50 | 50 | 1 | 51 | 0 | 0 | 0 |
| 16：45 to 17：00 | 148 | 0 | 148 | 304 | 13 | 317 | 17 | 0 | 17 | 0 | 0 | 0 | 13 | 0 | 13 | 52 | 0 | 52 | 38 | 0 | 38 | 0 | 0 | 0 |
| 17：00 to 17：15 | 149 | 1 | 150 | 284 | 4 | 288 | 15 | 1 | 16 | 0 | 0 | 0 | 16 | 0 | 16 | 67 | 0 | 67 | 45 | 0 | 45 | 0 | 0 | 0 |
| 17：15 to 17：30 | 156 | 2 | 158 | 282 | 6 | 288 | 12 | 0 | 12 | 0 | 0 | 0 | 8 | 0 | 8 | 47 | 1 | 48 | 44 | 0 | 44 | 0 | 0 | 0 |
| 17：30 to 17：45 | 157 | 4 | 161 | 280 | 7 | 287 | 22 | 1 | 23 | 0 | 0 | 0 | 8 | 0 | 8 | 51 | 2 | 53 | 38 | 0 | 38 | 0 | 0 | 0 |
| 17：45 to 18：00 | 127 | 3 | 130 | 258 | 7 | 265 | 10 | 1 | 11 | 0 | 0 | 0 | 10 | 2 | 12 | 52 | 0 | 52 | 41 | 0 | 41 | 0 | 0 | 0 |
| PM Totals | 1143 | 16 | 1159 | 2224 | 60 | 2284 | 147 | 8 | 155 | 0 | 0 | 0 | 109 | 9 | 118 | 423 | 3 | 426 | 370 | 2 | 372 | 0 | 0 | 0 |


| Job No． | ：N790 |
| :--- | :--- |
| Client | ：Realty Realizations |
| Suburb | ：Nowra |
| Location | $:$ 7．Kalandar St／Princes Hwy |
|  |  |
| Day／Date | ：Fri，4th May 2012 |
| Weather | ：Fine |
| Description | ：Classified Intersection Count |
|  | $:$ Hourly Summary |



Princes Hwy

| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ |  |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\frac{\stackrel{7}{0}}{\underline{I}}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{1} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ |  | $\begin{aligned} & \text { ふ } \\ & \text { ふ } \\ & \text { ¹ } \end{aligned}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ |  |  | $\begin{aligned} & \overline{\mathrm{O}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { ふ } \\ & \text { 㐅} \\ & \text { © } \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{O}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { ふ } \\ & \text { § } \\ & \text { ² } \end{aligned}$ | ¢ |
| 7：00 to 8：00 | 1 | 0 | 1 | 487 | 54 | 541 | 26 | 3 | 29 | 0 | 0 | 0 | 36 | 3 | 39 | 162 | 2 | 164 | 322 | 5 | 327 | 0 | 0 | 0 |
| 7：15 to 8：15 | 2 | 0 | 2 | 604 | 46 | 650 | 30 | 1 | 31 | 0 | 0 | 0 | 32 | 2 | 34 | 174 | 3 | 177 | 405 | 8 | 413 | 0 | 0 | 0 |
| 7：30 to 8：30 | 3 | 0 | 3 | 669 | 50 | 719 | 25 | 1 | 26 | 0 | 0 | 0 | 33 | 1 | 34 | 213 | 3 | 216 | 499 | 14 | 513 | 0 | 0 | 0 |
| 7：45 to 8：45 | 3 | 0 | 3 | 733 | 40 | 773 | 30 | 1 | 31 | 0 | 0 | 0 | 33 | 3 | 36 | 248 | 3 | 251 | 598 | 16 | 614 | 0 | 0 | 0 |
| 8：00 to 9：00 | 3 | 0 | 3 | 764 | 45 | 809 | 32 | 0 | 32 | 0 | 0 | 0 | 29 | 3 | 32 | 252 | 8 | 260 | 644 | 19 | 663 | 0 | 0 | 0 |
| AM Totals | 4 | 0 | 4 | 1251 | 99 | 1350 | 58 | 3 | 61 | 0 | 0 | 0 | 65 | 6 | 71 | 414 | 10 | 424 | 966 | 24 | 990 | 0 | 0 | 0 |
| 16：00 to 17：00 | 8 | 0 | 8 | 679 | 18 | 697 | 73 | 1 | 74 | 0 | 0 | 0 | 66 | 1 | 67 | 161 | 4 | 165 | 375 | 7 | 382 | 0 | 0 | 0 |
| 16：15 to 17：15 | 4 | 0 | 4 | 672 | 16 | 688 | 71 | 3 | 74 | 0 | 0 | 0 | 68 | 1 | 69 | 161 | 5 | 166 | 373 | 4 | 377 | 0 | 0 | 0 |
| 16：30 to $17: 30$ | 4 | 0 | 4 | 618 | 14 | 632 | 63 | 2 | 65 | 0 | 0 | 0 | 72 | 1 | 73 | 166 | 5 | 171 | 395 | 4 | 399 | 0 | 0 | 0 |
| 16：45 to 17：45 | 4 | 0 | 4 | 619 | 17 | 636 | 56 | 2 | 58 | 0 | 0 | 0 | 67 | 1 | 68 | 169 | 3 | 172 | 388 | 2 | 390 | 0 | 0 | 0 |
| 17：00 to 18：00 | 5 | 0 | 5 | 574 | 15 | 589 | 55 | 2 | 57 | 0 | 0 | 0 | 67 | 1 | 68 | 180 | 2 | 182 | 400 | 2 | 402 | 0 | 0 | 0 |
| PM Totals | 13 | 0 | 13 | 1253 | 33 | 1286 | 128 | 3 | 131 | 0 | 0 | 0 | 133 | 2 | 135 | 341 | 6 | 347 | 775 | 9 | 784 | 0 | 0 | 0 |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{O}} \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \mathrm{I} \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{\circ}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{I}} \\ & \hline \mathrm{~J} \end{aligned}$ | $\begin{aligned} & \text { ス } \\ & \text { त⿹丁口 } \\ & \text { in } \end{aligned}$ | $\begin{gathered} \overline{\mathrm{O}} \\ \stackrel{-}{\circ} \end{gathered}$ | $\frac{\stackrel{\rightharpoonup}{\mathrm{V}}}{\underline{\mathrm{~J}}}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{V}} \\ & \stackrel{\rightharpoonup}{\mathrm{O}} \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{I}} \\ & \hline \mathrm{~J} \end{aligned}$ | $\begin{aligned} & \text { } \\ & \text { त } \\ & \frac{0}{x} \end{aligned}$ | ¢ |
| 7：00 to 8：00 | 113 | 13 | 126 | 548 | 59 | 607 | 66 | 6 | 72 | 0 | 0 | 0 | 22 | 7 | 29 | 48 | 10 | 58 | 80 | 1 | 81 | 0 | 0 | 0 |
| 7：15 to 8：15 | 140 | 10 | 150 | 642 | 54 | 696 | 75 | 8 | 83 | 0 | 0 | 0 | 28 | 7 | 35 | 57 | 6 | 63 | 95 | 5 | 100 | 0 | 0 | 0 |
| 7：30 to 8：30 | 151 | 8 | 159 | 709 | 49 | 758 | 101 | 12 | 113 | 0 | 0 | 0 | 35 | 7 | 42 | 71 | 5 | 76 | 109 | 6 | 115 | 0 | 0 | 0 |
| 7：45 to 8：45 | 176 | 11 | 187 | 708 | 50 | 758 | 116 | 11 | 127 | 0 | 0 | 0 | 36 | 7 | 43 | 87 | 2 | 89 | 125 | 5 | 130 | 0 | 0 | 0 |
| 8：00 to 9：00 | 204 | 16 | 220 | 700 | 55 | 755 | 118 | 15 | 133 | 0 | 0 | 0 | 38 | 6 | 44 | 125 | 2 | 127 | 125 | 7 | 132 | 0 | 0 | 0 |
| AM Totals | 317 | 29 | 346 | 1248 | 114 | 1362 | 184 | 21 | 205 | 0 | 0 | 0 | 60 | 13 | 73 | 173 | 12 | 185 | 205 | 8 | 213 | 0 | 0 | 0 |
| 16：00 to 17：00 | 554 | 6 | 560 | 1120 | 36 | 1156 | 88 | 5 | 93 | 0 | 0 | 0 | 67 | 7 | 74 | 206 | 0 | 206 | 202 | 2 | 204 | 0 | 0 | 0 |
| 16：15 to 17：15 | 553 | 1 | 554 | 1115 | 32 | 1147 | 74 | 4 | 78 | 0 | 0 | 0 | 68 | 4 | 72 | 218 | 0 | 218 | 186 | 1 | 187 | 0 | 0 | 0 |
| 16：30 to $17: 30$ | 581 | 3 | 584 | 1138 | 30 | 1168 | 66 | 3 | 69 | 0 | 0 | 0 | 52 | 3 | 55 | 216 | 1 | 217 | 177 | 1 | 178 | 0 | 0 | 0 |
| 16：45 to 17：45 | 610 | 7 | 617 | 1150 | 30 | 1180 | 66 | 2 | 68 | 0 | 0 | 0 | 45 | 0 | 45 | 217 | 3 | 220 | 165 | 0 | 165 | 0 | 0 | 0 |
| 17：00 to 18：00 | 589 | 10 | 599 | 1104 | 24 | 1128 | 59 | 3 | 62 | 0 | 0 | 0 | 42 | 2 | 44 | 217 | 3 | 220 | 168 | 0 | 168 | 0 | 0 | 0 |
| PM Totals | 1143 | 16 | 1159 | 2224 | 60 | 2284 | 147 | 8 | 155 | 0 | 0 | 0 | 109 | 9 | 118 | 423 | 3 | 426 | 370 | 2 | 372 | 0 | 0 | 0 |




|  |  |
| :--- | :--- |
| Job No. | $:$ N790 |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $:$ 8. Forest Rd / Coonamia Rd |
| Day/Date | $:$ Fri, 4th May 2012 |
| Weather | : Fine |
| Description | $:$ Classified Intersection Count |
|  | $: 15$ mins Data |



| 16:00 to 16:15 | 9 | 0 | 9 | 32 | 2 | 34 | 0 | 0 | 0 | 32 | 2 | 34 | 9 | 0 | 9 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16:15 to 16:30 | 9 | 0 | 9 | 32 | 2 | 34 | 0 | 0 | 0 | 26 | 1 | 27 | 7 | 1 | 8 | 0 | 0 | 0 |
| 16:30 to 16:45 | 4 | 0 | 4 | 28 | 0 | 28 | 0 | 0 | 0 | 15 | 1 | 16 | 3 | 0 | 3 | 0 | 0 | 0 |
| 16:45 to 17:00 | 3 | 0 | 3 | 28 | 1 | 29 | 0 | 0 | 0 | 14 | 1 | 15 | 3 | 0 | 3 | 0 | 0 | 0 |
| 17:00 to 17:15 | 6 | 0 | 6 | 35 | 0 | 35 | 0 | 0 | 0 | 18 | 0 | 18 | 2 | 0 | 2 | 0 | 0 | 0 |
| 17:15 to 17:30 | 4 | 0 | 4 | 42 | 0 | 42 | 0 | 0 | 0 | 19 | 0 | 19 | 4 | 0 | 4 | 0 | 0 | 0 |
| 17:30 to 17:45 | 9 | 0 | 9 | 25 | 0 | 25 | 0 | 0 | 0 | 16 | 1 | 17 | 4 | 0 | 4 | 0 | 0 | 0 |
| 17:45 to 18:00 | 5 | 0 | 5 | 33 | 0 | 33 | 0 | 0 | 0 | 15 | 0 | 15 | 5 | 0 | 5 | 0 | 0 | 0 |
| PM Totals | 49 | 0 | 49 | 255 | 5 | 260 | 0 | 0 | 0 | 155 | 6 | 161 | 37 | 1 | 38 | 0 | 0 | 0 |


| Job No. | : N790 |
| :--- | :--- |
| Client | : Realty Realizations |
| Suburb | : Nowra |
| Location | $:$ 8. Forest Rd / Coonamia Rd |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | : Fine |
| Description | : Classified Intersection Count |
|  | $:$ Hourly Summary |


: Hourly Summary




| Approach <br> Direction | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U <br> （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 <br> （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{gathered} \text { 历̈ } \\ \stackrel{\circ}{\circ} \end{gathered}$ | $\begin{aligned} & \frac{7}{0} \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\pi}{3} \\ & \text { © } \\ & \text { In } \end{aligned}$ | $\begin{aligned} & \overline{\amalg ٓ} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{5} \\ & \stackrel{O}{J} \end{aligned}$ | $\begin{aligned} & \underset{0}{\lambda} \\ & \text { ָin } \end{aligned}$ | $\begin{aligned} & \bar{\leftrightarrows} \\ & \stackrel{\rightharpoonup}{\circ} \\ & \hline \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\boxed{\circ}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\pi}{\square} \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\text { In }} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { ㄷㅡㅡㄹ } \\ & \hline 0 \end{aligned}$ |  | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $$ | $\begin{aligned} & \underset{\substack{3 \\ \mathbf{x} \\ \text { In }}}{ } \end{aligned}$ | $\begin{aligned} & \stackrel{\pi}{\circ} \\ & \stackrel{1}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & 3 \\ & \begin{array}{c} 3 \\ \text { I } \end{array} \end{aligned}$ | 픙 |
| 7：00 to 7：15 | 3 | 2 | 5 | 21 | 1 | 22 | 6 | 1 | 7 | 0 | 0 | 0 | 1 | 0 | 1 | 47 | 2 | 49 | 4 | 0 | 4 | 0 | 0 | 0 |
| 7：15 to 7：30 | 5 | 3 | 8 | 37 | 0 | 37 | 8 | 0 | 8 | 0 | 0 | 0 | 2 | 0 | 2 | 34 | 2 | 36 | 11 | 1 | 12 | 0 | 0 | 0 |
| 7：30 to 7：45 | 6 | 0 | 6 | 55 | 1 | 56 | 4 | 1 | 5 | 0 | 0 | 0 | 8 | 0 | 8 | 28 | 3 | 31 | 11 | 0 | 11 | 0 | 0 | 0 |
| 7：45 to 8：00 | 6 | 0 | 6 | 71 | 2 | 73 | 5 | 1 | 6 | 0 | 0 | 0 | 6 | 0 | 6 | 49 | 0 | 49 | 27 | 0 | 27 | 0 | 0 | 0 |
| 8：00 to 8：15 | 3 | 1 | 4 | 87 | 3 | 90 | 11 | 0 | 11 | 0 | 0 | 0 | 5 | 0 | 5 | 49 | 4 | 53 | 21 | 1 | 22 | 0 | 0 | 0 |
| 8：15 to 8：30 | 7 | 0 | 7 | 106 | 4 | 110 | 10 | 0 | 10 | 0 | 0 | 0 | 5 | 0 | 5 | 70 | 5 | 75 | 41 | 1 | 42 | 0 | 0 | 0 |
| 8：30 to 8：45 | 7 | 1 | 8 | 109 | 2 | 111 | 14 | 0 | 14 | 0 | 0 | 0 | 3 | 0 | 3 | 54 | 2 | 56 | 41 | 0 | 41 | 0 | 0 | 0 |
| 8：45 to 9：00 | 7 | 0 | 7 | 94 | 5 | 99 | 15 | 0 | 15 | 0 | 0 | 0 | 12 | 0 | 12 | 49 | 9 | 58 | 26 | 1 | 27 | 0 | 0 | 0 |
| AM Totals | 44 | 7 | 51 | 580 | 18 | 598 | 73 | 3 | 76 | 0 | 0 | 0 | 42 | 0 | 42 | 380 | 27 | 407 | 182 | 4 | 186 | 0 | 0 | 0 |
| 16：00 to 16：15 | 3 | 0 | 3 | 69 | 0 | 69 | 15 | 0 | 15 | 0 | 0 | 0 | 9 | 0 | 9 | 43 | 4 | 47 | 24 | 0 | 24 | 0 | 0 | 0 |
| 16：15 to $16: 30$ | 7 | 1 | 8 | 73 | 1 | 74 | 21 | 0 | 21 | 0 | 0 | 0 | 8 | 0 | 8 | 35 | 2 | 37 | 20 | 0 | 20 | 0 | 0 | 0 |
| 16：30 to $16: 45$ | 4 | 0 | 4 | 44 | 0 | 44 | 9 | 0 | 9 | 1 | 0 | 1 | 5 | 1 | 6 | 39 | 2 | 41 | 13 | 0 | 13 | 0 | 0 | 0 |
| 16：45 to 17：00 | 8 | 0 | 8 | 33 | 0 | 33 | 7 | 0 | 7 | 0 | 0 | 0 | 4 | 0 | 4 | 35 | 0 | 35 | 19 | 1 | 20 | 1 | 0 | 1 |
| 17：00 to 17：15 | 3 | 1 | 4 | 40 | 1 | 41 | 10 | 0 | 10 | 0 | 0 | 0 | 5 | 0 | 5 | 27 | 2 | 29 | 23 | 0 | 23 | 0 | 0 | 0 |
| 17：15 to 17：30 | 8 | 0 | 8 | 43 | 0 | 43 | 10 | 0 | 10 | 0 | 0 | 0 | 7 | 0 | 7 | 32 | 2 | 34 | 15 | 0 | 15 | 0 | 0 | 0 |
| 17：30 to 17：45 | 3 | 0 | 3 | 32 | 0 | 32 | 13 | 1 | 14 | 0 | 0 | 0 | 14 | 0 | 14 | 36 | 1 | 37 | 18 | 0 | 18 | 1 | 0 | 1 |
| 17：45 to 18：00 | 1 | 0 | 1 | 32 | 0 | 32 | 15 | 0 | 15 | 0 | 0 | 0 | 6 | 0 | 6 | 30 | 1 | 31 | 23 | 0 | 23 | 1 | 0 | 1 |
| PM Totals | 37 | 2 | 39 | 366 | 2 | 368 | 100 | 1 | 101 | 1 | 0 | 1 | 58 | 1 | 59 | 277 | 14 | 291 | 155 | 1 | 156 | 3 | 0 | 3 |


| Job No． | $:$ N790 |
| :--- | :--- |
| Client | ：Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $: 9$. Kalandar St／Kinghorne St |
|  |  |
| Day／Date | ：Fri，4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |

： 15 mins Data
 15 лериегея


SKYHIGH－THE TRAFFIC SURVEY COMPANY

| Approach | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 <br> （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 哥 } \\ & \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\circ}{1} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & .0 \end{aligned}$ |  | $\begin{gathered} \overline{Ð 5} \\ \stackrel{\circ}{\circ} \\ \hline \end{gathered}$ | $\frac{\text { 苛 }}{3}$ | $\begin{aligned} & \underset{\substack{\lambda \\ \\ ~}}{ } \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \sum_{\nwarrow}^{\lambda} \\ & \text { © } \end{aligned}$ | $\begin{aligned} & \overline{\ddagger 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\mathrm{IN}} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ | $\begin{aligned} & \gtrless_{0}^{7} \\ & \text { 오 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $$ |  | П |
| 7：00 to 7：15 | 9 | 1 | 10 | 2 | 0 | 2 | 110 | 1 | 111 | 0 | 0 | 0 | 19 | 0 | 19 | 15 | 5 | 20 | 0 | 1 | 1 | 0 | 0 | 0 |
| 7：15 to 7：30 | 15 | 1 | 16 | 2 | 0 | 2 | 101 | 1 | 102 | 0 | 0 | 0 | 27 | 1 | 28 | 10 | 2 | 12 | 2 | 0 | 2 | 0 | 0 | 0 |
| 7：30 to 7：45 | 12 | 1 | 13 | 4 | 0 | 4 | 42 | 1 | 43 | 0 | 0 | 0 | 27 | 2 | 29 | 21 | 3 | 24 | 1 | 0 | 1 | 0 | 0 | 0 |
| 7：45 to 8：00 | 18 | 0 | 18 | 6 | 0 | 6 | 52 | 2 | 54 | 0 | 0 | 0 | 33 | 0 | 33 | 21 | 3 | 24 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8：00 to 8：15 | 19 | 1 | 20 | 7 | 0 | 7 | 35 | 1 | 36 | 0 | 0 | 0 | 43 | 1 | 44 | 45 | 7 | 52 | 1 | 1 | 2 | 0 | 0 | 0 |
| 8：15 to 8：30 | 16 | 0 | 16 | 3 | 0 | 3 | 39 | 2 | 41 | 2 | 0 | 2 | 54 | 3 | 57 | 35 | 3 | 38 | 1 | 0 | 1 | 0 | 0 | 0 |
| 8：30 to 8：45 | 20 | 0 | 20 | 7 | 0 | 7 | 38 | 1 | 39 | 1 | 0 | 1 | 63 | 3 | 66 | 38 | 1 | 39 | 3 | 0 | 3 | 0 | 0 | 0 |
| 8：45 to 9：00 | 49 | 3 | 52 | 9 | 0 | 9 | 32 | 1 | 33 | 2 | 0 | 2 | 51 | 3 | 54 | 44 | 2 | 46 | 2 | 1 | 3 | 0 | 0 | 0 |
| AM Totals | 158 | 7 | 165 | 40 | 0 | 40 | 449 | 10 | 459 | 5 | 0 | 5 | 317 | 13 | 330 | 229 | 26 | 255 | 10 | 3 | 13 | 0 | 0 | 0 |


| 16：00 to 16：15 | 60 | 1 | 61 | 14 | 1 | 15 | 52 | 2 | 54 | 1 | 0 | 1 | 65 | 0 | 65 | 53 | 2 | 55 | 2 | 0 | 2 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16：15 to 16：30 | 48 | 0 | 48 | 15 | 0 | 15 | 49 | 3 | 52 | 1 | 0 | 1 | 58 | 1 | 59 | 61 | 1 | 62 | 3 | 0 | 3 | 0 | 0 | 0 |
| 16：30 to 16：45 | 47 | 1 | 48 | 16 | 1 | 17 | 45 | 2 | 47 | 0 | 0 | 0 | 42 | 1 | 43 | 61 | 1 | 62 | 4 | 0 | 4 | 0 | 0 | 0 |
| 16：45 to 17：00 | 55 | 0 | 55 | 7 | 0 | 7 | 52 | 0 | 52 | 0 | 0 | 0 | 35 | 0 | 35 | 41 | 0 | 41 | 1 | 0 | 1 | 0 | 0 | 0 |
| 17：00 to 17：15 | 75 | 0 | 75 | 16 | 0 | 16 | 42 | 2 | 44 | 0 | 0 | 0 | 50 | 2 | 52 | 56 | 1 | 57 | 3 | 0 | 3 | 0 | 0 | 0 |
| 17：15 to 17：30 | 46 | 0 | 46 | 12 | 0 | 12 | 53 | 2 | 55 | 2 | 0 | 2 | 40 | 0 | 40 | 42 | 0 | 42 | 4 | 0 | 4 | 0 | 0 | 0 |
| 17：30 to 17：45 | 49 | 0 | 49 | 9 | 0 | 9 | 53 | 1 | 54 | 0 | 0 | 0 | 31 | 1 | 32 | 50 | 1 | 51 | 0 | 0 | 0 | 0 | 0 | 0 |
| 17：45 to 18：00 | 39 | 0 | 39 | 10 | 0 | 10 | 35 | 1 | 36 | 0 | 0 | 0 | 46 | 1 | 47 | 37 | 1 | 38 | 1 | 0 | 1 | 0 | 0 | 0 |
| PM Totals | 419 | 2 | 421 | 99 | 2 | 101 | 381 | 13 | 394 | 4 | 0 | 4 | 367 | 6 | 373 | 401 | 7 | 408 | 18 | 0 | 18 | 0 | 0 | 0 |


| Job No． | $:$ N790 |
| :--- | :--- |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $: 9$. Kalandar St／Kinghorne St |
|  |  |
| Day／Date | ：Fri，4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |



SKYHIGH－THE TRAFFIC SURVEY COMPANY

| Approach | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\text { 号 }}{3} \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{0} \\ & \hline \end{aligned}$ |  | 픈 | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{O}} \\ & \stackrel{y}{\mathrm{O}} \end{aligned}$ | $\begin{aligned} & \text { ふ } \\ & \text { オ } \\ & \text { d } \end{aligned}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \underline{3} \end{aligned}$ |  | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\text { Z }}{\substack{x}} \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ |  |  | 픈 | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\text { Z }}{\substack{\text { I } \\ \hline}} \end{aligned}$ | ¢ |
| 7：00 to 8：00 | 20 | 5 | 25 | 184 | 4 | 188 | 23 | 3 | 26 | 0 | 0 | 0 | 17 | 0 | 17 | 158 | 7 | 165 | 53 | 1 | 54 | 0 | 0 | 0 |
| 7：15 to 8：15 | 20 | 4 | 24 | 250 | 6 | 256 | 28 | 2 | 30 | 0 | 0 | 0 | 21 | 0 | 21 | 160 | 9 | 169 | 70 | 2 | 72 | 0 | 0 | 0 |
| 7：30 to 8：30 | 22 | 1 | 23 | 319 | 10 | 329 | 30 | 2 | 32 | 0 | 0 | 0 | 24 | 0 | 24 | 196 | 12 | 208 | 100 | 2 | 102 | 0 | 0 | 0 |
| 7：45 to 8：45 | 23 | 2 | 25 | 373 | 11 | 384 | 40 | 1 | 41 | 0 | 0 | 0 | 19 | 0 | 19 | 222 | 11 | 233 | 130 | 2 | 132 | 0 | 0 | 0 |
| 8：00 to 9：00 | 24 | 2 | 26 | 396 | 14 | 410 | 50 | 0 | 50 | 0 | 0 | 0 | 25 | 0 | 25 | 222 | 20 | 242 | 129 | 3 | 132 | 0 | 0 | 0 |
| AM Totals | 44 | 7 | 51 | 580 | 18 | 598 | 73 | 3 | 76 | 0 | 0 | 0 | 42 | 0 | 42 | 380 | 27 | 407 | 182 | 4 | 186 | 0 | 0 | 0 |
| 16：00 to 17：00 | 22 | 1 | 23 | 219 | 1 | 220 | 52 | 0 | 52 | 1 | 0 | 1 | 26 | 1 | 27 | 152 | 8 | 160 | 76 | 1 | 77 | 1 | 0 | 1 |
| 16：15 to 17：15 | 22 | 2 | 24 | 190 | 2 | 192 | 47 | 0 | 47 | 1 | 0 | 1 | 22 | 1 | 23 | 136 | 6 | 142 | 75 | 1 | 76 | 1 | 0 | 1 |
| 16：30 to 17：30 | 23 | 1 | 24 | 160 | 1 | 161 | 36 | 0 | 36 | 1 | 0 | 1 | 21 | 1 | 22 | 133 | 6 | 139 | 70 | 1 | 71 | 1 | 0 | 1 |
| 16：45 to 17：45 | 22 | 1 | 23 | 148 | 1 | 149 | 40 | 1 | 41 | 0 | 0 | 0 | 30 | 0 | 30 | 130 | 5 | 135 | 75 | 1 | 76 | 2 | 0 | 2 |
| 17：00 to 18：00 | 15 | 1 | 16 | 147 | 1 | 148 | 48 | 1 | 49 | 0 | 0 | 0 | 32 | 0 | 32 | 125 | 6 | 131 | 79 | 0 | 79 | 2 | 0 | 2 |
| PM Totals | 37 | 2 | 39 | 366 | 2 | 368 | 100 | 1 | 101 | 1 | 0 | 1 | 58 | 1 | 59 | 277 | 14 | 291 | 155 | 1 | 156 | 3 | 0 | 3 |


| Approach | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ | $\frac{\stackrel{7}{0}}{\underline{I}}$ | $\begin{aligned} & 3 \\ & \begin{array}{l} \lambda \\ \\ \hline \end{array} \end{aligned}$ | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{V}} \\ & \hline \mathrm{~J} \end{aligned}$ |  | $\begin{gathered} \overline{\boxed{\circ}} \\ \stackrel{-}{\circ} \end{gathered}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{I}} \\ & \hline \mathrm{~J} \end{aligned}$ | $\begin{aligned} & \lambda_{\pi}^{\lambda} \\ & \text { N్土 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & 7 \\ & \substack{0 \\ \text { N } \\ \hline} \end{aligned}$ | $\begin{gathered} \overline{\boxed{0}} \\ \stackrel{-}{\circ} \end{gathered}$ | $\frac{\stackrel{\rightharpoonup}{\mathrm{I}}}{\underline{\mathrm{I}}}$ |  | $\begin{gathered} \overline{\boxed{O}} \\ \stackrel{-}{\circ} \end{gathered}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{I}} \\ & \hline \mathrm{~J} \end{aligned}$ |  | 든 |
| 7：00 to 8：00 | 54 | 3 | 57 | 14 | 0 | 14 | 305 | 5 | 310 | 0 | 0 | 0 | 106 | 3 | 109 | 67 | 13 | 80 | 3 | 1 | 4 | 0 | 0 | 0 |
| 7：15 to 8：15 | 64 | 3 | 67 | 19 | 0 | 19 | 230 | 5 | 235 | 0 | 0 | 0 | 130 | 4 | 134 | 97 | 15 | 112 | 4 | 1 | 5 | 0 | 0 | 0 |
| 7：30 to 8：30 | 65 | 2 | 67 | 20 | 0 | 20 | 168 | 6 | 174 | 2 | 0 | 2 | 157 | 6 | 163 | 122 | 16 | 138 | 3 | 1 | 4 | 0 | 0 | 0 |
| 7：45 to 8：45 | 73 | 1 | 74 | 23 | 0 | 23 | 164 | 6 | 170 | 3 | 0 | 3 | 193 | 7 | 200 | 139 | 14 | 153 | 5 | 1 | 6 | 0 | 0 | 0 |
| 8：00 to 9：00 | 104 | 4 | 108 | 26 | 0 | 26 | 144 | 5 | 149 | 5 | 0 | 5 | 211 | 10 | 221 | 162 | 13 | 175 | 7 | 2 | 9 | 0 | 0 | 0 |
| AM Totals | 158 | 7 | 165 | 40 | 0 | 40 | 449 | 10 | 459 | 5 | 0 | 5 | 317 | 13 | 330 | 229 | 26 | 255 | 10 | 3 | 13 | 0 | 0 | 0 |
| 16：00 to 17：00 | 210 | 2 | 212 | 52 | 2 | 54 | 198 | 7 | 205 | 2 | 0 | 2 | 200 | 2 | 202 | 216 | 4 | 220 | 10 | 0 | 10 | 0 | 0 | 0 |
| 16：15 to 17：15 | 225 | 1 | 226 | 54 | 1 | 55 | 188 | 7 | 195 | 1 | 0 | 1 | 185 | 4 | 189 | 219 | 3 | 222 | 11 | 0 | 11 | 0 | 0 | 0 |
| 16：30 to 17：30 | 223 | 1 | 224 | 51 | 1 | 52 | 192 | 6 | 198 | 2 | 0 | 2 | 167 | 3 | 170 | 200 | 2 | 202 | 12 | 0 | 12 | 0 | 0 | 0 |
| 16：45 to 17：45 | 225 | 0 | 225 | 44 | 0 | 44 | 200 | 5 | 205 | 2 | 0 | 2 | 156 | 3 | 159 | 189 | 2 | 191 | 8 | 0 | 8 | 0 | 0 | 0 |
| 17：00 to 18：00 | 209 | 0 | 209 | 47 | 0 | 47 | 183 | 6 | 189 | 2 | 0 | 2 | 167 | 4 | 171 | 185 | 3 | 188 | 8 | 0 | 8 | 0 | 0 | 0 |
| PM Totals | 419 | 2 | 421 | 99 | 2 | 101 | 381 | 13 | 394 | 4 | 0 | 4 | 367 | 6 | 373 | 401 | 7 | 408 | 18 | 0 | 18 | 0 | 0 | 0 |


|  |  |
| :--- | :--- |
| Job No. | : N790 |
| Client | : Realty Realizations |
| Suburb | : Nowra |
| Location | $:$ 10. Forest Rd / Princes Hwy |
|  | $:$ Fri, 4th May 2012 |
| Day/Date | : Fine |
| Weather |  |
| Description | $:$ Classified Intersection Count |
|  | $: 15$ mins Data |






| 16:00 to 16:15 | 20 | 0 | 20 | 359 | 10 | 369 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16:15 to 16:30 | 28 | 2 | 30 | 306 | 10 | 316 | 0 | 0 | 0 |
| 16:30 to 16:45 | 29 | 0 | 29 | 279 | 5 | 284 | 1 | 0 | 1 |
| 16:45 to 17:00 | 21 | 0 | 21 | 339 | 1 | 340 | 0 | 0 | 0 |
| 17:00 to 17:15 | 32 | 1 | 33 | 340 | 8 | 348 | 0 | 0 | 0 |
| 17:15 to 17:30 | 29 | 0 | 29 | 324 | 7 | 331 | 0 | 0 | 0 |
| 17:30 to 17:45 | 30 | 0 | 30 | 294 | 5 | 299 | 0 | 0 | 0 |
| 17:45 to 18:00 | 16 | 1 | 17 | 263 | 4 | 267 | 0 | 0 | 0 |
| PM Totals | 205 | 4 | 209 | 2504 | 50 | 2554 | 1 | 0 | 1 |


| Job No. | $:$ N790 |
| :--- | :--- |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $:$ 10. Forest Rd / Princes Hwy |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |






| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 <br> （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\text { IN }} \\ & \stackrel{0}{1} \\ & \hline \end{aligned}$ |  | $\begin{array}{r} 3 \\ \substack{3 \\ \text { I } \\ \hline \\ \hline} \end{array}$ | $\begin{aligned} & \bar{\Pi} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\text { IN }} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $$ | $\begin{aligned} & \underset{\text { J }}{\substack{\text { In }}} \end{aligned}$ |  |  | $\begin{array}{r} \underset{\pi}{3} \\ \stackrel{\pi}{I} \\ \hline \end{array}$ | $\begin{aligned} & \overline{\mathrm{O}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{array}{r} \substack{3 \\ \vdots \\ \text { 0 } \\ \hline \\ \hline} \end{array}$ | $\begin{aligned} & \text { Г̄ } \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $$ | $\begin{array}{r} 3 \\ \begin{array}{c} 3 \\ \text { 0 } \\ \hline \end{array} \\ \hline \end{array}$ | 픙 |
| 7：00 to 7：15 | 0 | 0 | 0 | 95 | 26 | 121 | 3 | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 3 | 13 | 0 | 13 | 6 | 0 | 6 | 0 | 0 | 0 |
| 7：15 to 7：30 | 2 | 0 | 2 | 138 | 11 | 149 | 5 | 1 | 6 | 0 | 0 | 0 | 2 | 0 | 2 | 5 | 0 | 5 | 19 | 5 | 24 | 0 | 0 | 0 |
| 7：30 to 7：45 | 0 | 0 | 0 | 145 | 22 | 167 | 4 | 1 | 5 | 0 | 0 | 0 | 4 | 0 | 4 | 18 | 0 | 18 | 36 | 0 | 36 | 0 | 0 | 0 |
| 7：45 to 8：00 | 0 | 0 | 0 | 153 | 12 | 165 | 9 | 0 | 9 | 0 | 0 | 0 | 1 | 0 | 1 | 33 | 0 | 33 | 23 | 0 | 23 | 0 | 0 | 0 |
| 8：00 to 8：15 | 0 | 0 | 0 | 143 | 16 | 159 | 12 | 2 | 14 | 0 | 0 | 0 | 5 | 0 | 5 | 37 | 3 | 40 | 42 | 1 | 43 | 0 | 0 | 0 |
| $8: 15$ to $8: 30$ | 0 | 0 | 0 | 188 | 15 | 203 | 22 | 3 | 25 | 0 | 0 | 0 | 4 | 1 | 5 | 41 | 2 | 43 | 42 | 4 | 46 | 0 | 0 | 0 |
| $8: 30$ to $8: 45$ | 3 | 0 | 3 | 202 | 13 | 215 | 47 | 0 | 47 | 0 | 0 | 0 | 8 | 0 | 8 | 48 | 3 | 51 | 66 | 5 | 71 | 0 | 0 | 0 |
| 8：45 to 9：00 | 0 | 0 | 0 | 169 | 14 | 183 | 48 | 0 | 48 | 0 | 0 | 0 | 11 | 0 | 11 | 59 | 0 | 59 | 58 | 0 | 58 | 0 | 0 | 0 |
| AM Totals | 5 | 0 | 5 | 1233 | 129 | 1362 | 150 | 7 | 157 | 0 | 0 | 0 | 38 | 1 | 39 | 254 | 8 | 262 | 292 | 15 | 307 | 0 | 0 | 0 |
| 16：00 to 16：15 | 2 | 0 | 2 | 250 | 8 | 258 | 21 | 0 | 21 | 1 | 0 | 1 | 5 | 0 | 5 | 28 | 0 | 28 | 51 | 2 | 53 | 0 | 0 | 0 |
| 16：15 to 16：30 | 1 | 0 | 1 | 216 | 8 | 224 | 22 | 0 | 22 | 0 | 0 | 0 | 6 | 1 | 7 | 31 | 0 | 31 | 37 | 0 | 37 | 0 | 0 | 0 |
| 16：30 to 16：45 | 0 | 0 | 0 | 273 | 11 | 284 | 21 | 0 | 21 | 0 | 0 | 0 | 5 | 0 | 5 | 23 | 0 | 23 | 34 | 1 | 35 | 0 | 0 | 0 |
| 16：45 to 17：00 | 4 | 0 | 4 | 222 | 4 | 226 | 13 | 0 | 13 | 0 | 0 | 0 | 6 | 0 | 6 | 19 | 0 | 19 | 31 | 0 | 31 | 0 | 0 | 0 |
| 17：00 to 17：15 | 0 | 0 | 0 | 272 | 4 | 276 | 9 | 0 | 9 | 0 | 0 | 0 | 2 | 0 | 2 | 18 | 0 | 18 | 28 | 1 | 29 | 0 | 0 | 0 |
| 17：15 to 17：30 | 1 | 0 | 1 | 239 | 3 | 242 | 21 | 0 | 21 | 0 | 0 | 0 | 1 | 0 | 1 | 11 | 0 | 11 | 27 | 0 | 27 | 0 | 0 | 0 |
| 17：30 to 17：45 | 1 | 0 | 1 | 217 | 6 | 223 | 11 | 0 | 11 | 1 | 0 | 1 | 2 | 0 | 2 | 25 | 0 | 25 | 32 | 0 | 32 | 0 | 0 | 0 |
| 17：45 to 18：00 | 1 | 0 | 1 | 194 | 6 | 200 | 7 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 0 | 15 | 25 | 0 | 25 | 0 | 0 | 0 |
| PM Totals | 10 | 0 | 10 | 1883 | 50 | 1933 | 125 | 0 | 125 | 2 | 0 | 2 | 27 | 1 | 28 | 170 | 0 | 170 | 265 | 4 | 269 | 0 | 0 | 0 |


： 15 mins Data
Princes Hwy

| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 <br> （Left Turn） |  |  | Direction 8 <br> （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 <br> （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period |  |  | $\begin{aligned} & \overline{\# 5} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{5} \\ & \stackrel{\circ}{1} \\ & \hline \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{5} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline . . \end{aligned}$ |  | $\begin{aligned} & \overline{5} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\mathrm{N}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $$ | $\begin{aligned} & \sum_{J}^{\gtrless} \\ & \text { © } \end{aligned}$ | $\begin{aligned} & \overline{5} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | \} | $\begin{aligned} & \overline{\mathrm{IN}} \\ & \stackrel{0}{1} \end{aligned}$ |  | \} $\\ {\substack{\text { ¢ } \\ \text { ¢ }}} \\ {\hline}$ | П |
| 7：00 to 7：15 | 9 | 1 | 10 | 214 | 22 | 236 | 34 | 2 | 36 | 0 | 0 | 0 | 10 | 0 | 10 | 0 | 0 | 0 | 2 | 1 | 3 | 0 | 0 | 0 |
| 7：15 to 7：30 | 6 | 1 | 7 | 220 | 16 | 236 | 27 | 2 | 29 | 0 | 0 | 0 | 13 | 1 | 14 | 7 | 1 | 8 | 5 | 1 | 6 | 0 | 0 | 0 |
| 7：30 to 7：45 | 14 | 1 | 15 | 247 | 15 | 262 | 57 | 3 | 60 | 0 | 0 | 0 | 16 | 2 | 18 | 7 | 0 | 7 | 5 | 2 | 7 | 0 | 0 | 0 |
| 7：45 to 8：00 | 23 | 1 | 24 | 282 | 19 | 301 | 82 | 4 | 86 | 0 | 0 | 0 | 16 | 0 | 16 | 10 | 1 | 11 | 7 | 0 | 7 | 0 | 0 | 0 |
| 8：00 to 8：15 | 27 | 0 | 27 | 271 | 17 | 288 | 72 | 0 | 72 | 0 | 0 | 0 | 21 | 4 | 25 | 20 | 0 | 20 | 6 | 2 | 8 | 0 | 0 | 0 |
| 8：15 to 8：30 | 43 | 2 | 45 | 289 | 23 | 312 | 74 | 3 | 77 | 0 | 0 | 0 | 30 | 3 | 33 | 25 | 2 | 27 | 9 | 1 | 10 | 0 | 0 | 0 |
| 8：30 to 8：45 | 39 | 4 | 43 | 261 | 15 | 276 | 73 | 5 | 78 | 0 | 0 | 0 | 20 | 2 | 22 | 34 | 2 | 36 | 14 | 2 | 16 | 0 | 0 | 0 |
| 8：45 to 9：00 | 42 | 2 | 44 | 296 | 27 | 323 | 91 | 4 | 95 | 0 | 0 | 0 | 25 | 2 | 27 | 28 | 0 | 28 | 8 | 0 | 8 | 0 | 0 | 0 |
| AM Totals | 203 | 12 | 215 | 2080 | 154 | 2234 | 510 | 23 | 533 | 0 | 0 | 0 | 151 | 14 | 165 | 131 | 6 | 137 | 56 | 9 | 65 | 0 | 0 | 0 |


| 16：00 to 16：15 | 28 | 1 | 29 | 267 | 15 | 282 | 81 | 1 | 82 | 0 | 0 | 0 | 85 | 0 | 85 | 40 | 0 | 40 | 20 | 0 | 20 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16：15 to 16：30 | 36 | 2 | 38 | 273 | 6 | 279 | 69 | 1 | 70 | 0 | 0 | 0 | 91 | 2 | 93 | 42 | 0 | 42 | 28 | 1 | 29 | 0 | 0 | 0 |
| 16：30 to 16：45 | 31 | 1 | 32 | 343 | 13 | 356 | 62 | 0 | 62 | 0 | 0 | 0 | 65 | 2 | 67 | 27 | 0 | 27 | 14 | 0 | 14 | 0 | 0 | 0 |
| 16：45 to 17：00 | 34 | 1 | 35 | 247 | 7 | 254 | 61 | 0 | 61 | 0 | 0 | 0 | 96 | 0 | 96 | 56 | 0 | 56 | 28 | 0 | 28 | 1 | 0 | 1 |
| 17：00 to 17：15 | 35 | 1 | 36 | 308 | 7 | 315 | 74 | 0 | 74 | 0 | 0 | 0 | 83 | 0 | 83 | 51 | 0 | 51 | 35 | 1 | 36 | 0 | 0 | 0 |
| 17：15 to 17：30 | 25 | 0 | 25 | 273 | 10 | 283 | 48 | 1 | 49 | 0 | 0 | 0 | 70 | 1 | 71 | 32 | 0 | 32 | 17 | 0 | 17 | 0 | 0 | 0 |
| 17：30 to 17：45 | 33 | 0 | 33 | 280 | 7 | 287 | 58 | 1 | 59 | 0 | 0 | 0 | 78 | 0 | 78 | 32 | 0 | 32 | 25 | 0 | 25 | 0 | 0 | 0 |
| 17：45 to 18：00 | 19 | 0 | 19 | 268 | 10 | 278 | 47 | 0 | 47 | 0 | 0 | 0 | 60 | 0 | 60 | 26 | 0 | 26 | 15 | 0 | 15 | 0 | 0 | 0 |
| PM Totals | 241 | 6 | 247 | 2259 | 75 | 2334 | 500 | 4 | 504 | 0 | 0 | 0 | 628 | 5 | 633 | 306 | 0 | 306 | 182 | 2 | 184 | 1 | 0 | 1 |


| Job No． | $:$ N790 |
| :--- | :--- |
| Client | ：Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $:$ 11．Moss St／Princes Hwy |
|  |  |
| Day／Date | ：Fri，4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |



SKYHIGH－THE TRAFFIC SURVEY COMPANY

| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 <br> （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period |  |  | $\begin{aligned} & \overline{\amalg ँ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \mathrm{J} \end{aligned}$ |  | $\begin{aligned} & \overline{\amalg ँ} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{I}} \\ & \hline \mathrm{~J} \end{aligned}$ |  | $\begin{aligned} & \overline{\text { In }} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\frac{\text { 苛 }}{3}$ | $\begin{aligned} & \underset{\nwarrow}{\lambda} \\ & \stackrel{\text { x }}{\mathbf{I}} \end{aligned}$ | $\begin{aligned} & \overline{\text { In }} \\ & \stackrel{1}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\pi}{\grave{~}} \\ & \stackrel{y}{x} \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{-}{\circ} \end{aligned}$ | 苛 | $\begin{aligned} & {\underset{刃}{\mathbf{J}}}_{\substack{\text { I }}} \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{O}} \\ & \stackrel{-}{\circ} \end{aligned}$ |  | $\begin{aligned} & \underset{\pi}{\grave{~ N}} \\ & \stackrel{y}{I} \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $$ | 入 <br> ¢ <br> ¢ | \} |
| 7：00 to 8：00 | 2 | 0 | 2 | 531 | 71 | 602 | 21 | 2 | 23 | 0 | 0 | 0 | 10 | 0 | 10 | 69 | 0 | 69 | 84 | 5 | 89 | 0 | 0 | 0 |
| 7：15 to 8：15 | 2 | 0 | 2 | 579 | 61 | 640 | 30 | 4 | 34 | 0 | 0 | 0 | 12 | 0 | 12 | 93 | 3 | 96 | 120 | 6 | 126 | 0 | 0 | 0 |
| 7：30 to 8：30 | 0 | 0 | 0 | 629 | 65 | 694 | 47 | 6 | 53 | 0 | 0 | 0 | 14 | 1 | 15 | 129 | 5 | 134 | 143 | 5 | 148 | 0 | 0 | 0 |
| 7：45 to 8：45 | 3 | 0 | 3 | 686 | 56 | 742 | 90 | 5 | 95 | 0 | 0 | 0 | 18 | 1 | 19 | 159 | 8 | 167 | 173 | 10 | 183 | 0 | 0 | 0 |
| 8：00 to 9：00 | 3 | 0 | 3 | 702 | 58 | 760 | 129 | 5 | 134 | 0 | 0 | 0 | 28 | 1 | 29 | 185 | 8 | 193 | 208 | 10 | 218 | 0 | 0 | 0 |
| AM Totals | 5 | 0 | 5 | 1233 | 129 | 1362 | 150 | 7 | 157 | 0 | 0 | 0 | 38 | 1 | 39 | 254 | 8 | 262 | 292 | 15 | 307 | 0 | 0 | 0 |
| 16：00 to 17：00 | 7 | 0 | 7 | 961 | 31 | 992 | 77 | 0 | 77 | 1 | 0 | 1 | 22 | 1 | 23 | 101 | 0 | 101 | 153 | 3 | 156 | 0 | 0 | 0 |
| 16：15 to 17：15 | 5 | 0 | 5 | 983 | 27 | 1010 | 65 | 0 | 65 | 0 | 0 | 0 | 19 | 1 | 20 | 91 | 0 | 91 | 130 | 2 | 132 | 0 | 0 | 0 |
| 16：30 to 17：30 | 5 | 0 | 5 | 1006 | 22 | 1028 | 64 | 0 | 64 | 0 | 0 | 0 | 14 | 0 | 14 | 71 | 0 | 71 | 120 | 2 | 122 | 0 | 0 | 0 |
| 16：45 to 17：45 | 6 | 0 | 6 | 950 | 17 | 967 | 54 | 0 | 54 | 1 | 0 | 1 | 11 | 0 | 11 | 73 | 0 | 73 | 118 | 1 | 119 | 0 | 0 | 0 |
| 17：00 to 18：00 | 3 | 0 | 3 | 922 | 19 | 941 | 48 | 0 | 48 | 1 | 0 | 1 | 5 | 0 | 5 | 69 | 0 | 69 | 112 | 1 | 113 | 0 | 0 | 0 |
| PM Totals | 10 | 0 | 10 | 1883 | 50 | 1933 | 125 | 0 | 125 | 2 | 0 | 2 | 27 | 1 | 28 | 170 | 0 | 170 | 265 | 4 | 269 | 0 | 0 | 0 |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ |  |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ |  |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline 1 \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\frac{\text { 岢 }}{3}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ | $\frac{\stackrel{7}{0}}{\underline{I}}$ | $\begin{aligned} & 3 \\ & \begin{array}{l} \lambda \\ \frac{0}{1} \end{array} \end{aligned}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\frac{\stackrel{7}{0}}{\underline{I}}$ | $\begin{aligned} & \underset{\sim}{\lambda} \\ & \stackrel{\pi}{\top} \end{aligned}$ | $\begin{aligned} & \bar{\Pi} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | ふ ¢ ¢ ¢ | 工 |
| 7：00 to 8：00 | 52 | 4 | 56 | 963 | 72 | 1035 | 200 | 11 | 211 | 0 | 0 | 0 | 55 | 3 | 58 | 24 | 2 | 26 | 19 | 4 | 23 | 0 | 0 | 0 |
| 7：15 to 8：15 | 70 | 3 | 73 | 1020 | 67 | 1087 | 238 | 9 | 247 | 0 | 0 | 0 | 66 | 7 | 73 | 44 | 2 | 46 | 23 | 5 | 28 | 0 | 0 | 0 |
| 7：30 to 8：30 | 107 | 4 | 111 | 1089 | 74 | 1163 | 285 | 10 | 295 | 0 | 0 | 0 | 83 | 9 | 92 | 62 | 3 | 65 | 27 | 5 | 32 | 0 | 0 | 0 |
| 7：45 to 8：45 | 132 | 7 | 139 | 1103 | 74 | 1177 | 301 | 12 | 313 | 0 | 0 | 0 | 87 | 9 | 96 | 89 | 5 | 94 | 36 | 5 | 41 | 0 | 0 | 0 |
| 8：00 to 9：00 | 151 | 8 | 159 | 1117 | 82 | 1199 | 310 | 12 | 322 | 0 | 0 | 0 | 96 | 11 | 107 | 107 | 4 | 111 | 37 | 5 | 42 | 0 | 0 | 0 |
| AM Totals | 203 | 12 | 215 | 2080 | 154 | 2234 | 510 | 23 | 533 | 0 | 0 | 0 | 151 | 14 | 165 | 131 | 6 | 137 | 56 | 9 | 65 | 0 | 0 | 0 |
| 16：00 to 17：00 | 129 | 5 | 134 | 1130 | 41 | 1171 | 273 | 2 | 275 | 0 | 0 | 0 | 337 | 4 | 341 | 165 | 0 | 165 | 90 | 1 | 91 | 1 | 0 | 1 |
| 16：15 to 17：15 | 136 | 5 | 141 | 1171 | 33 | 1204 | 266 | 1 | 267 | 0 | 0 | 0 | 335 | 4 | 339 | 176 | 0 | 176 | 105 | 2 | 107 | 1 | 0 | 1 |
| 16：30 to 17：30 | 125 | 3 | 128 | 1171 | 37 | 1208 | 245 | 1 | 246 | 0 | 0 | 0 | 314 | 3 | 317 | 166 | 0 | 166 | 94 | 1 | 95 | 1 | 0 | 1 |
| 16：45 to 17：45 | 127 | 2 | 129 | 1108 | 31 | 1139 | 241 | 2 | 243 | 0 | 0 | 0 | 327 | 1 | 328 | 171 | 0 | 171 | 105 | 1 | 106 | 1 | 0 | 1 |
| 17：00 to 18：00 | 112 | 1 | 113 | 1129 | 34 | 1163 | 227 | 2 | 229 | 0 | 0 | 0 | 291 | 1 | 292 | 141 | 0 | 141 | 92 | 1 | 93 | 0 | 0 | 0 |
| PM Totals | 241 | 6 | 247 | 2259 | 75 | 2334 | 500 | 4 | 504 | 0 | 0 | 0 | 628 | 5 | 633 | 306 | 0 | 306 | 182 | 2 | 184 | 1 | 0 | 1 |

## B.CALCULATION OF TRAFFIC GROWTH FACTORS \& TRIP GENERATION RATES (SHOALHAVEN CITY COUNCIL)



## From:

Sent:
To:
Subject:
Attachments:

Ken Hollyoak
Wednesday, 20 February 2013 9:21 AM
Wayne Johnson; Justin Murphy
FW: UPDATED Trip Generation Analysis - Culburra 3A Development - assumptions for traffic study
Hourly Data 200407053 Falls Creek Correction.xls; Greenwell Point Rd Annual Analysis based on 2008.xlsx; Forest Rd Annual Analysis based on 2008.xlsx; Culburra Traffic Gen Analysis.xlsx

From: Wells, Scott [mailto:WELLSS@shoalhaven.nsw.gov.au]
Sent: Tuesday, 19 February 2013 6:37 PM
To: Ken Hollyoak
Cc: Britton, John; Williams, Brett; 'MILLET Chris P'
Subject: UPDATED Trip Generation Analysis - Culburra 3A Development - assumptions for traffic study

## Hi Ken

The following is summary of our analysis of external traffic generation rates to be applied, and what adjustments are required in our view to the May 2012 base survey data you are intending to use as basis for your analysis. The spreadsheet used to base this analysis summary is attached (Culburra Traffic Gen analysis) and the annual traffic data used to derive AADT and $120^{\text {th }} \mathrm{HH}$ factors are also attached (Greenwell Pt Rd and Forest Rd for local adjustment factors, Princes Highway Falls Creek for Highway adjustment factors).

The analysis is detailed, undertaken by our Transport Engineer. I have reviewed and support the findings. The impact of the development is likely to be considerable and we have undertaken the analysis to ensure the analysis of the developments impacts is sufficiently detailed, robust and realistic in order for Council and RMS to better understand the developments likely impacts.

As previously stated this area is objected to significant seasonal fluctuations in traffic levels, thus the request to consider an AADT scenario as well as $120^{\text {th }} \mathrm{HH}$ scenario in accordance with RMS guidelines and AUSTROADS.

## Peak Hour Development Traffic Generation

To be applied to proposed residential development - these rates are based on detached dwellings, reduction may need to be considered for any proposed non-detached dwellings. Note this is for external regional traffic distribution only. Directional split data obtained from examining local road annual data in equivalent AADT \& $120^{\text {th }}$ Highest Annual Hour periods.

| Peak Hour <br> Scenario | Factor (vehicles per <br> hour per occupied <br> dwelling) | Directional Split - AADT <br> (eastbound / westbound) | Directional Split - 120 <br> (eastbound $/$ HH <br> (estbound) |
| :---: | :---: | :---: | :---: |
| Friday AM | 0.22 | $22 \% / 78 \%$ | $24 \% / 76 \%$ |
| Friday PM | 0.21 | $65 \% / 35 \%$ | $75 \% / 25 \%$ |
| Saturday MD | 0.23 | $53 \% / 47 \%$ | $50 \% / 50 \%$ |

Note: The balance of peak hour trips per dwelling (in accordance with RTA's Guide to Traffic Generating Developments) must be assigned to/from Culburra to complete the external distribution analysis.

## Survey Data Conversion Rates for Peak Hour Traffic Volumes - NON-HIGHWAY DATA - Based on Local Peaks

These factors convert 1-hour data from the applicants May 2012 surveys to theoretical AADT \& Seasonal Peak flows for the intersection analysis. These factors apply to all surveyed local road peak hour flows, including all movements to/from the Highway, but not north-south through movements on the Highway (refer factors below for highway analysis). The additional factors for Friday ( $3-4 \mathrm{pm}$ ) \& Saturday ( $8-9 \mathrm{am}$ ) are provided for separate analysis as the local and highway peaks do not coincide at these times. Note the Friday AM peak ( $8-9 \mathrm{am}$ ) for local \& Highway traffic coincided, therefore separate analysis is not required. Because of the conflicting peak times our recommendation is for all of the following scenarios to be assessed for worst case in each of the AADT and $120^{\text {th }} \mathrm{HH}$ scenarios.

| Peak Hour Scenario <br> AADT Analysis | Factor <br> (converts 2012 survey data to theoretical AADT <br> values - LOS C target for intersection analysis) |
| :---: | :---: |
| Friday AM (8-9am) | 0.92 |
| Friday PM (2-3pm) | $1.10^{*}$ |
| Saturday MD (12-1pm) | 1.11 |
| Friday PM (3-4pm) | 1.10 |
| Saturday AM (8-9am) | 1.07 |


| Peak Hour Scenario <br> Seasonal Peak (120 <br> Analysis | FH) |
| :---: | :---: |
| (converts 2012 survey data to theoretical Seasonal <br> Peak values i.e. 120 <br> LOS D target for intersection anallysis) |  |
| Friday AM (8-9am) | 1.12 |
| Friday PM (2-3pm) | $1.41^{*}$ |
| Saturday MD (12-1pm) | 1.25 |
| Friday PM (3-4pm) | 1.41 |
| Saturday MD (8-9am) | 1.17 |

* Note: the Friday PM 1-hour analysis has also been factored to account for the actual local peak which occurred between $2-3 \mathrm{pm}$, rather than the $4-5 \mathrm{pm}$ peak reported in the 2012 survey data, which was approx $15 \%$ lower than the $2-3 \mathrm{pm}$ volume (local roads analysis).
* Note: the above factors are derived from the combined analysis of annual traffic data from the Greenwell Point Road and Forest Road data.


## Survey Data Conversion Rates for Peak Hour Traffic Volumes - PRINCES HIGHWAY DATA Based on Local \& Highway Peaks

This converts 1-hour data from May 2012 survey to theoretical AADT \& Seasonal Peak flows for intersection analysis. These factors apply only to all surveyed north-south through movements on the Highway for analysis based on either local (Coastal Villages) or Highway peak hour flows. The additional factors for Friday ( $3-4 \mathrm{pm}$ ) \& Saturday ( $8-9 \mathrm{am}$ ) are provided for separate analysis as the local and highway peaks do not coincide at these times. Note the Friday AM peak ( $8-9 \mathrm{am}$ ) for local \& Highway traffic coincided, therefore separate analysis is not required. Because of the conflicting peak times our recommendation is for all of the following scenarios to be assessed for worst case in each of the AADT and $120^{\text {th }} \mathrm{HH}$ scenarios.

| Peak Hour Scenario <br> AADT Analysis | Factor <br> (converts 2012 survey data to theoretical AADT <br> values - LOS C target for intersection analysis) |
| :---: | :---: |
| Friday AM (8-9am) | 0.88 |
| Friday PM (2-3pm) ${ }^{\star *}$ | 0.89 |
| Saturday MD (12-1pm) | 1.27 |
| Friday PM (3-4pm) | 0.88 |
| 1.32 |  |
| Peaturday Hour Scenario $(8-9 a m)$  <br> Seasonal Peak (120  <br> H. HH) | Factor <br> (converts 2012 survey data to theoretical Seasonal |


| Analysis | Peak values i.e. $120^{\text {th }}$ Highest Annual Hour - <br> LOS D target for intersection analysis) |
| :---: | :---: |
| Friday AM (8-9am) | 1.25 |
| Friday PM (2-3pm)** | 1.13 |
| Saturday MD (12-1pm) $^{\dagger}$ | 1.37 |
| Friday PM (3-4pm) | 1.15 |
| Saturday MD (8-9am) | 1.43 |

Note: The above factors are based on a permanent count station on the Princes Highway at Falls Creek (ie outside of the Nowra Urban Area) which is subject to AADT volumes of approx 20,000 veh/day. As the Princes Highway through the Nowra Urban Area is subject to volumes approximately twice as high (ie 40,000 veh/day), the influence of these factors can be reduced by half for analysis of the urban Princes Highway intersections (ie at Kalandar Street \& Moss Street). This results in the following factors (reading down the page): 0.94, 0.95, 1.14, 0.94, 1.16, 1.13, 1.07, 1.18, 1.07 \& 1.21 for urban area intersection analysis, and it is our view that it would be appropriate for the latter factors to be applied to all north-south through movements on the Highway to avoid un-justified increases or decreases to through-flows on the Highway for analysis.
** Note: the Friday PM peak hour on the Princes Highway at Falls Creek was 3-4pm, differed from both the surveyed peak (4-5pm) and the local peak (2-3pm). Accordingly, separate factors for undertaking a "Highway" peak hour analysis are also provided (the actual likely Friday PM peak hour).
${ }^{\dagger}$ Note: the Saturday peak hour on the Princes Highway at Falls Creek occurred between 8-9am, which was different to the local peak (12-1pm). Accordingly, separate factors for undertaking a "Highway" peak hour analysis are also provided (the actual likely Saturday peak hour).

## Survey Data Conversion Rates for "Daily" Traffic Volumes - NON-HIGHWAY DATA

These factors convert the applicant's May 2012 survey data to theoretical daily flows from AADT \& Seasonal Peak (120 ${ }^{\text {th }}$ Highest Annual Hour) equivalent peak hour flow levels. This is required to be undertaken to assess road design aspects (cross-section parameters) ie lane widths, clear zones, overtaking lanes etc for all scenarios including BOTH with / without the development.

This analysis is for local roads only, ie cross section assessment is not required to be undertaken on the Princes Highway.

The factors were determined by combining a peak-to-daily factor for each of the 6 scenarios (determined by analysing 2008 AADT \& Seasonal peak-to-daily factors separately) with a conversion factor for either Friday or Saturday, which compared the equivalent survey dates in 2008 (based on proximity to School Holidays) to the AADT \& Seasonal ( $120^{\text {th }}$ HH ) volumes accordingly.

Because of the conflicting peak times our recommendation is for all of the following scenarios to be assessed for worst case in each of the AADT and $120^{\text {th }}$ HH daily flow scenarios.

| AADT Peak-to-Daily Factors <br> (applies to surveyed 1-hour data <br> from May 2012) | Factor <br> (converts 2012 survey data to theoretical Annual <br> Average Daily volume level) |
| :---: | :---: |
| Friday AM (8-9am) | 10.22 |
| Saturday MD (12-1pm) | 14.06 |


| Seasonal Peak-to-Daily <br> Factors (applies to surveyed 1- <br> hour data from May 2012) | Factor <br> (converts 2012 survey data to theoretical Seasonal <br> Peak $\left(120^{\text {th }} \mathrm{HH}\right)$ equivalent daily volume level) |
| :---: | :---: |
| Friday AM (8-9am) | 14.34 |
| Saturday MD (12-1pm) | 16.67 |

* Note: the actual Friday PM local peak occurred between 2-3pm, rather than the 4-5pm peak reported in the 2012 survey data, which was approx $15 \%$ lower than the $2-3 p m$ volume. Accordingly the adjustment has only been applied to the Friday AM data, although only one daily flow calculation is required for the Friday in any case.
* Note: the above factors are to be applied direct to the base 2012 May survey data (local roads only).
* Note: the above factors are to calculate base daily flow levels for the AADT and $120^{\text {th }}$ HH equivalent daily flow scenarios. Your assessment will need to take into account the case with / without the development. To estimate the developments external daily traffic generation, refer to the top table for peak hour generation rates and use the same ratio of (external regional peak hour generation / RMS peak hour generation) to the RMS daily traffic generation rates to estimate external regional daily traffic generation. Similar to the peak hour analysis the balance of daily trips (between the RMS daily rate and the external regional daily development traffic) will then be assigned to/from Culburra Village to assess those more local impacts, in addition to the regional road impact analysis.

I hope all of that makes sense. We have tried to explain it in as simple terms as possible. We don't want to over complicate the assessment, but we do have an obligation to Council and the local community that the assessment has been undertaken correctly.

In regards to the future analysis scenario Council has adopted an ID forecast data set which provides population and dwelling projections in 5 year increments to 2036.

In addition to the above we will review this forecast data to provide our best advice in regards to an appropriate background traffic growth rate to apply for your assessment of future impacts. le as previously advised your analysis will need to consider a more realistic future time upon which the development will fully impact the surrounding road network. A ten year assessment is supported as industry practice however the growth rate to be applied needs to be agreed. We will review our ID data and provide advice in the coming days.

We will also provide our advice regarding external traffic distribution when you can clarify some of the points raised in my previous email and accordingly provide the additional information required.

Hope all of this helps, and I apologise for the delay.
I would expect the RMS will now review this advice and indicate whether they concur to this component of the study assumptions, as they will need to do for the distribution assumptions.

```
Kind Regards,
Scott Wells
Traffic & Transport Unit Manager
Shoalhaven City Council
```



```
wellss@shoalhaven.nsw.gov.au
}ttp://shoalhaven.nsw.gov.au
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```

```
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```


## From 2008 Annual Counts on Forest Rd \& Greenwell Point Rd

Forest Rd AADT: 2131 (FOREST ROAD 550m EAST OF PRINCES HIGHWAY)
Greenwell Point Rd AADT: 6003 (GREENWELL POINT ROAD 737m WEST OF PYREE LANE)
Combined Data AADT: 8134
Combined Equivalent Census Day: 7576 (occurred on Tuesday 12th Aug 2008 - equivalent day to 2011 Census day on 9th Aug 2011)

> Forest Rd 120th HH: 232 2008-04-29 16:00:00 Tuesday
> Greenwell Point Rd 120th HH: 669 2008-03-24 13:00:00 Monday

Combined Data 120th HH :
885 2008-01-25 13:00:00 Friday
(based on combining hourly data from each site to determine total 120th HH )
Equivalent Census Night Occupied Dwellings: 3324 (based on 2011 Census Enumerated population - where people were on Census night)
Note: see separate tab for methodology to calculate number of "equivalent" dwellings
Equivalent 2011 Combined AADT: 8632 ( $2 \%$ compound growth assumed)
Equivalent 2011 Combined Census Day Volume: 8040 ( $2 \%$ compound growth assumed)
External Traffic Generation: 2.4 Average Daily Trips per Occupied Dwelling
Equivalent 2011 AADT Occupied Dwellings: 3569 Theoretical number of occupied dwellings for AADT traffic rates
Fri AADT AM Peak to Daily Factor: Fri AADT PM Peak to Daily Factor: Sat AADT MD Peak to Daily Factor:

Fri AADT AM External Traffic Generation: Fri AADT PM External Traffic Generation: Sat AADT MD External Traffic Generation:

Daily volume Friday 9th May 2008: 9th May 2008 to 2008 AADT Conversion Factor: 9th May 2008 to Seaonal Peak Converstion Factor:

Daily volume Saturday 10th May 2008:
10th May 2008 to 2008 AADT Conversion Factor:
10th May 2008 to Seaonal Peak Converstion Factor:
AM Peak Friday 9th May 2008:
PM Peak Friday 9th May 2008:
MD Peak Saturday 10th May 2008:
3-4pm volume Fri 9th May 2008:
4-5pm volume Fri 9th May 2008: PM Peak Hour Conversion Factor:

HW1 Peak 8-9am Sat 8th May 2004:
Fri AM Peak Conversion Factor for AADT Analysis: Fri PM Peak Conversion Factor for AADT Analysis: Sat MD Peak Conversion Factor for AADT Analysis:

Fri Seasonal Peak AM Peak to Daily Factor: 7.6\% (based on Friday 25th Jan - date of 120th HH and 2nd highest Friday observed in 2008) Fri Seasonal Peak PM Peak to Daily Factor: $7.8 \%$ (based on Friday 25th Jan - date of 120th HH and 2nd highest Friday observed in 2008) Sat Seasonal Peak MD Peak to Daily Factor: $9.6 \%$ (based on Saturday 26th Jan - 3rd highest Saturday observed in 2008)
$9.0 \%$ (based on Fridays with volumes close to AADT volume of 8,134 veh/day)
$8.5 \%$ (based on Fridays with volumes close to AADT volume of 8,134 veh/day)
$9.6 \%$ (based on Saturdays with volumes close to AADT volume of 8,134 veh/day)
0.22 Vehicles per Hour per Occupied Dwelling
0.21 Vehicles per Hour per Occupied Dwelling
0.23 Vehicles per Hour per Occupied Dwelling

8826 Vehicles
0.92 This factor should be applied to the survey data from Fri 4th May 2012 1.35 This factor should be applied to the survey data from Fri 4th May 2012 7438 Vehicles
1.09 This factor should be applied to the survey data from Sat 5th May 2012
1.60 This factor should be applied to the survey data from Sat 5th May 2012

792 8-9am (equivalent day to date of survey Friday 4th May 2012 - based on proximity to school holidays) 741 2-3pm (equivalent day to date of survey Friday 4th May 2012 - based on proximity to school holidays) 706 12-1pm (equivalent day to date of survey Saturday 5th May 2012 - based on proximity to school holidays)

738 3-4pm HW1 Peak Hour
628 4-5pm (equivalent peak hour to survey data 4-5pm - lower than actual 2-3pm peak)
1.18 (converts reported $4-5$ pm data to actual $2-3 p m$ local peak)

487 8-9am HW1 Peak Hour
0.92 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) 1.10 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) 1.11 (Converts consultant's 2012 survey data to AADT values for LOS C analysis)

Daily volume on Friday 25th January 2008: 11885 (Day that 120th Highest Annual Hour Occurred)
120th Highest Annual Hour to Daily Traffic Ratio: $7.4 \%$ Use this figure to convert 2012 120th HH to Daily Volumes for cross-section design analysis
Fri AM Peak Conversion Factor for Seasonal Peak Analysis: Fri PM Peak Conversion Factor for Seasonal Peak Analysis: Sat MD Peak Conversion Factor for Seasonal Peak Analysis:
1.12 (Converts consultant's 2012 survey data to Seasonal Peak values for LOS D analysis) 1.41 (Converts consultant's 2012 survey data to Seasonal Peak values for LOS D analysis) 1.25 (Converts consultant's 2012 survey data to Seasonal Peak values for LOS D analysis)

> HW1 Analysis
> Falls Creek 2004 Data
> 2004 AADT: 18845 (Count Station 07053 at Falls Creek North of Jervis Bay Road)
> 2004 Average Fridays: 22373 (Better comparison due to wide daily variation)
> 2004 Average Saturdays: 17181 (Better comparison due to wide daily variation)

Falls Creek 120th HH: 1965 Thursday 8th Jan, 2004 12pm-1pm
HW1 Fri AADT AM Peak to Daily Factor: $7.3 \%$ (based on Fridays with volumes close to the Average Friday volume of 22,373 veh/day)
HW1 Fri AADT PM Peak to Daily Factor: $8.2 \%$ (based on Fridays with volumes close to the Average Friday volume of 22,373 veh/day) HW1 Sat AADT MD Peak to Daily Factor: $9.7 \%$ (based on Saturdays with volumes close to Average Saturday volume of 17,181 veh/day)

Daily Volume Friday 7th May 2004: 21007 Vehicles (equivalent day to Friday 4th May 2012 survey date)
7th May 2004 to 2004 AADT Conversion Factor: 0.90 This factor should be applied to the survey data from Fri 4th May 2012
Daily volume Saturday 8th May 2004: 16561 Vehicles
8th May 2004 to 2004 AADT Conversion Factor: 1.14 This factor should be applied to the survey data from Sat 5th May 2012
AM Local Culburra Peak Friday 7th May 2004: 1567 8-9am (Same as HW1 peak)
PM Local Culburra Peak Friday 7th May 2004: 1603 2-3pm (HW1 peak occurred 3-4pm - see below)
MD Local Culburra Peak Saturday 8th May 2004: 1436 12-1pm (HW1 peak occurred 8-9am - see below)
HW1 Peak 3-4pm Fri 7th May 2004: 1759 3-4pm HW1 Peak Hour
HW1 4-5pm volume Fri 7th May 2004: 1732 4-5pm (To compare to collected data)
PM Peak Hour Conversion Factor: $\mathbf{0 . 9 3}$ (converts reported 4-5pm data to actual 2-3pm local peak)

Fri AM Peak Conversion Factor for AADT Analysis: Fri PM Peak Conversion Factor for AADT Analysis: Sat MD Peak Conversion Factor for AADT Analysis:
0.88 (Converts consultant's 2012 survey data to AADT values for LOS C analysis)
0.89 (Converts consultant's 2012 survey data to AADT values for LOS C analysis)
1.27 (Converts consultant's 2012 survey data to AADT values for LOS C analysis)

Daily volume on Thursday 8th January 2004: 23295 (Day that 120th Highest Annual Hour Occurred)
120th Highest Annual Hour to Daily Traffic Ratio: $8.4 \%$ Use this figure to convert 2012 120th HH to Daily Volumes for cross-section design analysis

Fri AM Peak Conversion Factor for Seasonal Peak Analysis: Fri PM Peak Conversion Factor for Seasonal Peak Analysis: Sat MD Peak Conversion Factor for Seasonal Peak Analysis:
1.25 (Converts consultant's 2012 survey data to Seasonal Peak values for LOS D analysis) 1.13 (Converts consultant's 2012 survey data to Seasonal Peak values for LOS D analysis) 1.37 (Converts consultant's 2012 survey data to Seasonal Peak values for LOS D analysis)

## HW1 Analysis - factors for Different Highway Peaks

Local Road (i.e. Non-Highway) Flows
Fri PM Highway 3-4pm Conversion Factor for AADT Analysis: Sat AM Highway 8-9am Conversion Factor for AADT Analysis:
ri PM Highway 3-4pm Conversion Factor for 120th HH Analysis: at AM Highway 8-9am Conversion Factor for 120th HH Analysis:
1.10 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) 1.07 (Converts consultant's 2012 survey data to AADT values for LOS C analysis)
1.41 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) 1.17 (Converts consultant's 2012 survey data to AADT values for LOS C analysis)

Highway North-South Flows

Fri PM Highway 3-4pm Conversion Factor for AADT Analysis: Sat AM Highway 8-9am Conversion Factor for AADT Analysis:
ri PM Highway 3-4pm Conversion Factor for 120th HH Analysis: at AM Highway 8-9am Conversion Factor for 120th HH Analysis:
0.88 (Converts consultant's 2012 survey data to AADT values for LOS C analysis 1.32 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) 1.15 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) 1.43 (Converts consultant's 2012 survey data to AADT values for LOS C analysis

| Local Peak Comparison to HW1 Peak |  |  |
| :---: | :---: | :---: |
| Forest Rd + Greenwell Point Rd Friday AM | 792 (8-9am) | Forest Rd + Greenwell Point Rd LOCAL Fri AM Peak (SAME AS HW1 PEAK) |
| Forest Rd + Greenwell Point Rd Friday PM | 741 (2-3pm) | Forest Rd + Greenwell Point Rd LOCAL Fri PM Peak |
|  | 738 (3-4pm) | Forest Rd + Greenwell Point Rd $3-4 \mathrm{pm}$ volume |
| Local to HW1 peak Conversion Factor: | 1.00 (Converts | LOCAL Peak data to HW1 peak for Highway intersection analysis) |
| Forest Rd + Greenwell Point Rd Saturday MD | 706 (12-1pm) | Forest Rd + Greenwell Point Rd LOCAL Sat MD Peak |
|  | 487 (8-9am) | Forest Rd + Greenwell Point Rd 8-9am volume |
| Local to HW1 peak Conversion Factor: | 0.69 (Converts | LOCAL Peak data to HW1 peak for Highway intersection analysis) |

## 50\% Reduction Factor for HW1 intersections in Nowra Urban Area - ie at Kalandar St \& Moss St

[^4]| Date | Forest Rd |  |  | Greenwell Point Road |  |  | Combined |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Combined | E-bound W-bound | $\begin{aligned} & \text { \% E } \\ & \% \text { W } \end{aligned}$ | Combined | E-bound W-bound | $\begin{aligned} & \hline \text { \% E } \\ & \% \text { W } \end{aligned}$ | Combined | E-bound W-bound | $\begin{aligned} & \text { \% E } \\ & \text { \% W } \end{aligned}$ |
| $\begin{array}{\|c\|} \hline \text { Fri } 29-A u g-08 \\ \text { AM Peak } \\ \hline \end{array}$ | 180 | $\begin{gathered} \hline 54 \\ 126 \\ \hline \end{gathered}$ | $\begin{aligned} & \hline 30 \% \\ & 70 \% \\ & \hline \end{aligned}$ | 556 | $\begin{aligned} & \hline 105 \\ & 451 \end{aligned}$ | $\begin{aligned} & \hline 19 \% \\ & 81 \% \end{aligned}$ | 736 | $\begin{aligned} & \hline 159 \\ & 577 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 22 \% \\ & 78 \% \\ & \hline \end{aligned}$ |
| $\begin{array}{\|c} \hline \text { Fri 29-Aug-08 } \\ \text { PM Peak } \end{array}$ | 167 | $\begin{gathered} 112 \\ 55 \end{gathered}$ | $\begin{aligned} & \hline 67 \% \\ & 33 \% \end{aligned}$ | 562 | $\begin{aligned} & 364 \\ & 198 \end{aligned}$ | $\begin{aligned} & \hline 65 \% \\ & 35 \% \end{aligned}$ | 729 | $\begin{aligned} & \hline 476 \\ & 253 \end{aligned}$ | $\begin{aligned} & \hline 65 \% \\ & 35 \% \end{aligned}$ |
| $\begin{gathered} \text { Sat 30-Aug-08 } \\ \text { MD Peak } \\ \hline \end{gathered}$ | 186 | $\begin{aligned} & 94 \\ & 92 \\ & \hline \end{aligned}$ | $\begin{aligned} & 51 \% \\ & 49 \% \\ & \hline \end{aligned}$ | 490 | $\begin{aligned} & 261 \\ & 229 \\ & \hline \end{aligned}$ | $\begin{aligned} & 53 \% \\ & 47 \% \\ & \hline \end{aligned}$ | 676 | $\begin{aligned} & 355 \\ & 321 \end{aligned}$ | $\begin{aligned} & 53 \% \\ & 47 \% \\ & \hline \end{aligned}$ |
| $\begin{gathered} \hline \text { Fri 25-Jan-08 } \\ \text { AM Peak } \\ \hline \end{gathered}$ | 204 | $\begin{gathered} \hline 61 \\ 143 \\ \hline \end{gathered}$ | $\begin{aligned} & \hline 30 \% \\ & 70 \% \\ & \hline \end{aligned}$ | 561 | $\begin{aligned} & \hline 121 \\ & 440 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 22 \% \\ & 78 \% \\ & \hline \end{aligned}$ | 765 | $\begin{aligned} & 182 \\ & 583 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 24 \% \\ & 76 \% \\ & \hline \end{aligned}$ |
| $\begin{gathered} \text { Fri 25-Jan-08 } \\ \text { PM Peak } \end{gathered}$ | 226 | $\begin{gathered} \hline 160 \\ 66 \end{gathered}$ | $\begin{aligned} & \hline 71 \% \\ & 29 \% \end{aligned}$ | 700 | $\begin{aligned} & 530 \\ & 170 \end{aligned}$ | $\begin{aligned} & \hline 76 \% \\ & 24 \% \end{aligned}$ | 926 | $\begin{aligned} & 690 \\ & 236 \end{aligned}$ | $\begin{aligned} & 75 \% \\ & 25 \% \end{aligned}$ |
| Sat 26-Jan-08 MD Peak | 239 | $\begin{aligned} & \hline 125 \\ & 114 \end{aligned}$ | $\begin{aligned} & \hline 52 \% \\ & 48 \% \end{aligned}$ | 764 | $\begin{aligned} & 380 \\ & 384 \end{aligned}$ | $\begin{aligned} & \hline 50 \% \\ & 50 \% \end{aligned}$ | 1003 | $\begin{aligned} & 505 \\ & 498 \end{aligned}$ | $\begin{aligned} & 50 \% \\ & 50 \% \end{aligned}$ |

Note: Split data obtained directly from MetroCount data files


## C. SIDRA INTERSECTION RESULTS



## USER REPORT FOR SITE

Project: 200225sid-N186580 West Culburra Subdivision
$\nabla$ Site: 1 [1. Culburra -Coonamia (Ex Fri AM-120th HH)]
13S1231000 - West Culburra Subdivision
Culburra Road-Coonamia Road
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)
Movement Performance - Vehicles

| Mov Turn ID | Deman <br> Total veh/h |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| South: Coonamia Rd |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 165 | 0.0 | 0.180 | 9.3 | LOS A | 0.7 | 4.9 | 0.38 | 0.69 | 0.38 | 68.1 |
| 3 R2 | 73 | 6.5 | 0.130 | 11.3 | LOS A | 0.5 | 3.8 | 0.48 | 0.74 | 0.48 | 63.4 |
| Approach | 238 | 2.0 | 0.180 | 9.9 | LOS A | 0.7 | 4.9 | 0.41 | 0.70 | 0.41 | 66.6 |
| East: Culburra Rd (E) |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 40 | 2.9 | 0.022 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 64.4 |
| $5 \quad$ T1 | 225 | 3.7 | 0.118 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 80.0 |
| Approach | 265 | 3.6 | 0.118 | 1.1 | NA | 0.0 | 0.0 | 0.00 | 0.09 | 0.00 | 77.1 |
| West: Culburra Rd (W) |  |  |  |  |  |  |  |  |  |  |  |
| 11 T1 | 96 | 12.3 | 0.053 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 80.0 |
| 12 R 2 | 33 | 3.6 | 0.030 | 8.0 | LOS A | 0.1 | 0.8 | 0.35 | 0.62 | 0.35 | 66.4 |
| Approach | 128 | 10.1 | 0.053 | 2.0 | NA | 0.1 | 0.8 | 0.09 | 0.16 | 0.09 | 76.0 |
| All Vehicles | 632 | 4.3 | 0.180 | 4.6 | NA | 0.7 | 4.9 | 0.17 | 0.34 | 0.17 | 72.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.
$\nabla$ Site: 1 [1. Culburra -Coonamia (Ex Fri PM-120th HH)]
13S1231000 - West Culburra Subdivision
Culburra Road-Coonamia Road
Friday PM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Demand Total veh/h | $\begin{gathered} \text { =lows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Coonamia Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 77 | 7.7 | 0.078 | 8.8 | LOS A | 0.3 | 2.2 | 0.27 | 0.63 | 0.27 | 65.8 |
| 3 | R2 | 75 | 2.0 | 0.134 | 11.3 | LOS A | 0.5 | 3.8 | 0.49 | 0.75 | 0.49 | 64.8 |
| Appr | ach | 152 | 4.9 | 0.134 | 10.1 | LOS A | 0.5 | 3.8 | 0.38 | 0.69 | 0.38 | 65.3 |
| East: Culburra Rd (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 75 | 2.0 | 0.041 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 64.7 |
| 5 | T1 | 115 | 2.6 | 0.060 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 80.0 |
| Appr |  | 189 | 2.4 | 0.060 | 2.8 | NA | 0.0 | 0.0 | 0.00 | 0.25 | 0.00 | 73.2 |
| West: Culburra Rd (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 | T1 | 280 | 1.1 | 0.145 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 80.0 |
| 12 | R2 | 152 | 3.9 | 0.129 | 7.7 | LOS A | 0.5 | 3.9 | 0.31 | 0.63 | 0.31 | 66.4 |
| Appr |  | 432 | 2.1 | 0.145 | 2.7 | NA | 0.5 | 3.9 | 0.11 | 0.22 | 0.11 | 74.6 |
| All Ve | icles | 773 | 2.7 | 0.145 | 4.2 | NA | 0.5 | 3.9 | 0.14 | 0.32 | 0.14 | 72.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [1. Culburra -Coonamia (Ex Sat-120th HH)]

13S1231000 - West Culburra Subdivision
Culburra Road-Coonamia Road
Saturday - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Coonamia Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 104 | 1.3 | 0.109 | 9.0 | LOS A | 0.4 | 2.9 | 0.33 | 0.66 | 0.33 | 67.9 |
| 3 | R2 | 66 | 0.0 | 0.119 | 11.3 | LOS A | 0.5 | 3.2 | 0.49 | 0.75 | 0.49 | 65.5 |
| Appr |  | 171 | 0.8 | 0.119 | 9.9 | LOS A | 0.5 | 3.2 | 0.39 | 0.69 | 0.39 | 66.9 |
| East: Culburra Rd (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 80 | 0.0 | 0.043 | 6.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 65.4 |
| 5 | T1 | 169 | 0.8 | 0.087 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 80.0 |
| Appro |  | 249 | 0.5 | 0.087 | 2.2 | NA | 0.0 | 0.0 | 0.00 | 0.20 | 0.00 | 74.6 |
| West: Culburra Rd (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 | T1 | 166 | 0.8 | 0.086 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 80.0 |
| 12 | R2 | 104 | 2.5 | 0.094 | 7.9 | LOS A | 0.4 | 2.7 | 0.35 | 0.64 | 0.35 | 66.7 |
| Approach |  | 271 | 1.5 | 0.094 | 3.1 | NA | 0.4 | 2.7 | 0.14 | 0.25 | 0.14 | 74.3 |
| All Vehicles |  | 691 | 1.0 | 0.119 | 4.4 | NA | 0.5 | 3.2 | 0.15 | 0.34 | 0.15 | 72.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [2. Culburra-Mayfield (Ex Fri AM-120th HH)]

Culburra Road-Mayfield Road
Friday AM (0800-0900) - Equivalent 120th HH
Existing
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \mathrm{v} / \mathrm{c} \\ & \hline \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back Vehicles $\qquad$ | of Queue Distance $\qquad$ m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Culburra Road (S) |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 2 | 0.0 | 0.198 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 74.5 |
| 2 | T1 | 383 | 0.0 | 0.198 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| Appr |  | 385 | 0.0 | 0.198 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.8 |
| North: Culburra Road ( N ) |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 | T1 | 126 | 0.0 | 0.066 | 0.0 | LOS A | 0.0 | 0.1 | 0.01 | 0.01 | 0.01 | 79.8 |
| 9 | R2 | 1 | 0.0 | 0.066 | 8.7 | LOS A | 0.0 | 0.1 | 0.01 | 0.01 | 0.01 | 66.1 |
| Appro |  | 127 | 0.0 | 0.066 | 0.1 | NA | 0.0 | 0.1 | 0.01 | 0.01 | 0.01 | 79.7 |
| West: Mayfield Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 1 | 0.0 | 0.001 | 7.5 | LOS A | 0.0 | 0.0 | 0.42 | 0.56 | 0.42 | 56.9 |
| 12 | R2 | 2 | 0.0 | 0.006 | 12.0 | LOS A | 0.0 | 0.1 | 0.58 | 0.68 | 0.58 | 52.8 |
| Approach |  | 3 | 0.0 | 0.006 | 10.5 | LOS A | 0.0 | 0.1 | 0.53 | 0.64 | 0.53 | 54.1 |
| All Vehicles |  | 516 | 0.0 | 0.198 | 0.1 | NA | 0.0 | 0.1 | 0.01 | 0.01 | 0.01 | 79.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [2. Culburra-Mayfield (Ex Fri PM-120th HH)]

Culburra Road-Mayfield Road
Friday PM (1600-1700) - Equivalent 120th HH
Existing
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | Flows HV $\%$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \mathrm{v} / \mathrm{c} \\ & \hline \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back Vehicles $\qquad$ | of Queue Distance $\qquad$ m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Culburra Road (S) |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 1 | 0.0 | 0.102 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 74.6 |
| 2 | T1 | 193 | 4.6 | 0.102 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| Appr |  | 194 | 4.6 | 0.102 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| North: Culburra Road ( N ) |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 | T1 | 412 | 1.8 | 0.214 | 0.0 | LOS A | 0.0 | 0.1 | 0.00 | 0.00 | 0.00 | 79.9 |
| 9 | R2 | 1 | 0.0 | 0.214 | 7.8 | LOS A | 0.0 | 0.1 | 0.00 | 0.00 | 0.00 | 66.2 |
| Appro |  | 413 | 1.8 | 0.214 | 0.0 | NA | 0.0 | 0.1 | 0.00 | 0.00 | 0.00 | 79.9 |
| West: Mayfield Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 1 | 0.0 | 0.001 | 6.4 | LOS A | 0.0 | 0.0 | 0.29 | 0.53 | 0.29 | 57.6 |
| 12 | R2 | 1 | 100.0 | 0.009 | 33.5 | LOS C | 0.0 | 0.4 | 0.81 | 0.86 | 0.81 | 34.6 |
| Approach |  | 2 | 50.0 | 0.009 | 19.9 | LOS B | 0.0 | 0.4 | 0.55 | 0.69 | 0.55 | 43.3 |
| All Vehicles |  | 608 | 2.9 | 0.214 | 0.1 | NA | 0.0 | 0.4 | 0.00 | 0.00 | 0.00 | 79.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [2. Culburra-Mayfield (Ex Sat-120th HH)]

Culburra Road-Mayfield Road
Saturday - Equivalent 120th HH
Existing
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \mathrm{v} / \mathrm{c} \\ & \hline \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back Vehicles $\qquad$ | of Queue Distance $\qquad$ m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Culburra Road (S) |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 3 | 0.0 | 0.144 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 74.5 |
| 2 | T1 | 277 | 0.5 | 0.144 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 79.8 |
| Appr |  | 280 | 0.5 | 0.144 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 79.7 |
| North: Culburra Road ( N ) |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 | T1 | 248 | 0.5 | 0.129 | 0.0 | LOS A | 0.0 | 0.1 | 0.00 | 0.00 | 0.00 | 79.9 |
| 9 | R2 | 1 | 0.0 | 0.129 | 8.1 | LOS A | 0.0 | 0.1 | 0.00 | 0.00 | 0.00 | 66.1 |
| Appro | ch | 249 | 0.5 | 0.129 | 0.0 | NA | 0.0 | 0.1 | 0.00 | 0.00 | 0.00 | 79.8 |
| West: Mayfield Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 1 | 0.0 | 0.001 | 6.9 | LOS A | 0.0 | 0.0 | 0.36 | 0.54 | 0.36 | 57.4 |
| 12 | R2 | 3 | 0.0 | 0.008 | 12.2 | LOS A | 0.0 | 0.2 | 0.58 | 0.70 | 0.58 | 52.7 |
| Approach |  | 4 | 0.0 | 0.008 | 10.8 | LOS A | 0.0 | 0.2 | 0.52 | 0.66 | 0.52 | 53.8 |
| All Vehicles |  | 534 | 0.5 | 0.144 | 0.2 | NA | 0.0 | 0.2 | 0.01 | 0.01 | 0.01 | 79.5 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [3. Greenwell Pt-Pyree (Ex Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Pyree Lane
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  |  |  | Deg. Satn v/c | Average Delay sec | Level of Service |  |  | Prop. Queued | Effective Stop Rate | Aver. No. Average Cycles Speed km/h |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $\begin{gathered} \text { Deman } \\ \text { Total } \\ \text { veh/h } \end{gathered}$ | $\begin{gathered} \text { Flows } \\ \text { HV } \\ \% \end{gathered}$ |  |  |  | 95\% Back Vehicles veh | of Queue Distance m |  |  |  |  |
| South: Pyreen Ln |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 376 | 1.9 | 0.205 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 65.6 |
| 3 | R2 | 14 | 8.3 | 0.025 | 9.9 | LOS A | 0.1 | 0.7 | 0.44 | 0.67 | 0.44 | 59.2 |
| Appr | ach | 389 | 2.1 | 0.205 | 7.7 | LOS A | 0.1 | 0.7 | 0.02 | 0.60 | 0.02 | 65.4 |
| East: Greenwell Pt Rd (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 20 | 66.7 | 0.094 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.08 | 0.00 | 53.0 |
| 5 | T1 | 147 | 5.6 | 0.094 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.08 | 0.00 | 79.4 |
| Appr | ach | 167 | 12.9 | 0.094 | 1.0 | NA | 0.0 | 0.0 | 0.00 | 0.08 | 0.00 | 74.9 |
| West: Greenwell Pt Rd (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 | T1 | 63 | 5.6 | 0.034 | 6.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 64.9 |
| 12 | R2 | 111 | 7.4 | 0.230 | 12.4 | LOS A | 0.9 | 7.0 | 0.56 | 0.82 | 0.56 | 57.7 |
| Approach |  | 174 | 6.8 | 0.230 | 10.1 | LOS A | 0.9 | 7.0 | 0.36 | 0.74 | 0.36 | 60.1 |
| All V | icles | 731 | 5.7 | 0.230 | 6.7 | NA | 0.9 | 7.0 | 0.09 | 0.51 | 0.09 | 65.9 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [3. Greenwell Pt-Pyree (Ex Fri PM-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Pyree Lane
Friday PM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \mathrm{v} / \mathrm{c} \\ & \hline \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back Vehicles $\qquad$ | of Queue Distance m $\qquad$ | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Pyreen Ln |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 155 | 5.8 | 0.087 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 64.4 |
| 3 | R2 | 32 | 0.0 | 0.078 | 13.1 | LOS A | 0.3 | 2.0 | 0.58 | 0.82 | 0.58 | 58.4 |
| Appr |  | 186 | 4.8 | 0.087 | 8.6 | LOS A | 0.3 | 2.0 | 0.10 | 0.64 | 0.10 | 63.3 |
| East: Greenwell Pt Rd (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 26 | 0.0 | 0.050 | 6.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.18 | 0.00 | 71.7 |
| 5 | T1 | 68 | 4.3 | 0.050 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.18 | 0.00 | 76.6 |
| Appr |  | 95 | 3.1 | 0.050 | 1.9 | NA | 0.0 | 0.0 | 0.00 | 0.18 | 0.00 | 75.2 |
| West: Greenwell Pt Rd (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 | T1 | 173 | 1.7 | 0.090 | 6.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 0.00 | 66.2 |
| 12 | R2 | 391 | 1.5 | 0.567 | 11.8 | LOSA | 5.4 | 37.9 | 0.57 | 0.79 | 0.76 | 59.7 |
| Approach |  | 563 | 1.6 | 0.567 | 10.0 | LOS A | 5.4 | 37.9 | 0.40 | 0.73 | 0.53 | 61.6 |
| All Vehicles |  | 844 | 2.5 | 0.567 | 8.8 | NA | 5.4 | 37.9 | 0.29 | 0.65 | 0.37 | 63.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [3. Greenwell Pt-Pyree (Ex Sat-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Pyree Lane
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \mathrm{v} / \mathrm{c} \\ & \hline \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back Vehicles $\qquad$ | of Queue Distance $\qquad$ m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Pyreen Ln |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 226 | 1.2 | 0.123 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 65.9 |
| 3 | R2 | 45 | 0.0 | 0.078 | 9.7 | LOS A | 0.3 | 2.1 | 0.46 | 0.71 | 0.46 | 61.8 |
| Appr |  | 272 | 1.0 | 0.123 | 8.0 | LOS A | 0.3 | 2.1 | 0.08 | 0.62 | 0.08 | 65.1 |
| East: Greenwell Pt Rd (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 145 | 2.7 | 0.132 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.38 | 0.00 | 67.8 |
| 5 | T1 | 100 | 3.9 | 0.132 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.38 | 0.00 | 73.3 |
| Appro |  | 245 | 3.2 | 0.132 | 4.2 | NA | 0.0 | 0.0 | 0.00 | 0.38 | 0.00 | 69.9 |
| West: Greenwell Pt Rd (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 | T1 | 227 | 1.2 | 0.117 | 6.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 0.00 | 66.3 |
| 12 | R2 | 109 | 1.2 | 0.218 | 11.9 | LOS A | 0.9 | 6.2 | 0.55 | 0.81 | 0.55 | 59.8 |
| Approach |  | 337 | 1.2 | 0.218 | 7.9 | LOS A | 0.9 | 6.2 | 0.18 | 0.66 | 0.18 | 64.1 |
| All Vehicles |  | 854 | 1.7 | 0.218 | 6.9 | NA | 0.9 | 6.2 | 0.10 | 0.57 | 0.10 | 66.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [4. Greenwell Pt-Jindy Andy (Ex Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Jindy Andy Lane
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman <br> Total veh/h | $\begin{gathered} \text { Flows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| NorthEast: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |  |  |
| 25 | T1 | 371 | 4.8 | 0.197 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| 26 | R2 | 159 | 0.7 | 0.223 | 8.6 | LOS A | 1.0 | 6.9 | 0.37 | 0.66 | 0.37 | 63.0 |
| Appr |  | 529 | 3.6 | 0.223 | 2.6 | NA | 1.0 | 6.9 | 0.11 | 0.20 | 0.11 | 74.0 |
| NorthWest: Jindy Andy Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 39 | 3.0 | 0.038 | 7.7 | LOS A | 0.1 | 1.0 | 0.25 | 0.60 | 0.25 | 63.2 |
| 29 | R2 | 12 | 30.0 | 0.056 | 22.7 | LOS B | 0.2 | 1.6 | 0.75 | 0.91 | 0.75 | 45.2 |
| Appr |  | 51 | 9.2 | 0.056 | 11.1 | LOS A | 0.2 | 1.6 | 0.37 | 0.67 | 0.37 | 58.0 |
| SouthWest: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 8 | 42.9 | 0.079 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.04 | 0.00 | 59.1 |
| 31 | T1 | 136 | 8.7 | 0.079 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.04 | 0.00 | 79.5 |
| Approach |  | 144 | 10.7 | 0.079 | 0.5 | NA | 0.0 | 0.0 | 0.00 | 0.04 | 0.00 | 77.9 |
| All Vehicles |  | 724 | 5.4 | 0.223 | 2.8 | NA | 1.0 | 6.9 | 0.11 | 0.20 | 0.11 | 73.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [4. Greenwell Pt-Jindy Andy (Ex Fri PM-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Jindy Andy Lane
Friday AM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov |  | Demand <br> Total veh/h | $\begin{gathered} \text { Flows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| NorthEast: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |  |  |
| 25 | T1 | 175 | 5.9 | 0.094 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 80.0 |
| 26 | R2 | 61 | 2.4 | 0.141 | 12.9 | LOS A | 0.5 | 3.8 | 0.57 | 0.83 | 0.57 | 58.3 |
| Appr |  | 236 | 5.0 | 0.141 | 3.3 | NA | 0.5 | 3.8 | 0.15 | 0.22 | 0.15 | 72.9 |
| NorthWest: Jindy Andy Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 160 | 1.9 | 0.217 | 9.8 | LOS A | 0.8 | 5.9 | 0.50 | 0.77 | 0.50 | 61.6 |
| 29 | R2 | 3 | 0.0 | 0.011 | 16.1 | LOS B | 0.0 | 0.3 | 0.67 | 0.78 | 0.67 | 55.7 |
| Appr |  | 163 | 1.8 | 0.217 | 10.0 | LOS A | 0.8 | 5.9 | 0.50 | 0.77 | 0.50 | 61.5 |
| SouthWest: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 14 | 11.1 | 0.215 | 7.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 69.6 |
| 31 | T1 | 399 | 1.9 | 0.215 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 79.6 |
| Approach |  | 413 | 2.2 | 0.215 | 0.3 | NA | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 79.2 |
| All Vehicles |  | 812 | 2.9 | 0.217 | 3.1 | NA | 0.8 | 5.9 | 0.14 | 0.23 | 0.14 | 73.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.
$\nabla$ Site: 1 [4. Greenwell Pt-Jindy Andy (Ex Sat-120th HH)]
13S1231000 - West Culburra Subdivision
Greenwell Point Road-Jindy Andy Lane
Saturday - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Demand Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| NorthEast: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |  |  |
| 25 | T1 | 254 | 3.1 | 0.134 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 80.0 |
| 26 | R2 | 96 | 1.4 | 0.173 | 10.7 | LOS A | 0.7 | 4.9 | 0.50 | 0.76 | 0.50 | 60.7 |
| Appr |  | 349 | 2.6 | 0.173 | 2.9 | NA | 0.7 | 4.9 | 0.14 | 0.21 | 0.14 | 73.6 |
| NorthWest: Jindy Andy Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 100 | 0.0 | 0.115 | 8.6 | LOS A | 0.4 | 3.0 | 0.40 | 0.68 | 0.40 | 63.5 |
| 29 | R2 | 9 | 0.0 | 0.030 | 15.9 | LOS B | 0.1 | 0.7 | 0.66 | 0.83 | 0.66 | 55.9 |
| Approach |  | 109 | 0.0 | 0.115 | 9.3 | LOS A | 0.4 | 3.0 | 0.42 | 0.69 | 0.42 | 62.7 |
| SouthWest: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 5 | 0.0 | 0.154 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 74.4 |
| 31 | T1 | 288 | 3.2 | 0.154 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 79.7 |
| Appr | ach | 294 | 3.1 | 0.154 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 79.6 |
| All V | icles | 753 | 2.4 | 0.173 | 2.8 | NA | 0.7 | 4.9 | 0.12 | 0.20 | 0.12 | 73.9 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [5. Greenwell Pt-Mayfield (Ex Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Mayfield Road
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Mayfield Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 12 | 0.0 | 0.018 | 7.7 | LOS A | 0.1 | 0.4 | 0.45 | 0.64 | 0.45 | 56.4 |
| 23 | R2 | 1 | 0.0 | 0.018 | 12.9 | LOS A | 0.1 | 0.4 | 0.45 | 0.64 | 0.45 | 56.1 |
| Appr |  | 13 | 0.0 | 0.018 | 8.1 | LOS A | 0.1 | 0.4 | 0.45 | 0.64 | 0.45 | 56.4 |
| NorthEast: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 3 | 0.0 | 0.200 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 74.5 |
| 25 | T1 | 377 | 3.8 | 0.200 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 79.8 |
| Appro |  | 380 | 3.7 | 0.200 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 79.8 |
| SouthWest: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |  |  |
| 31 | T1 | 141 | 9.2 | 0.090 | 0.6 | LOS A | 0.2 | 1.4 | 0.09 | 0.03 | 0.09 | 78.0 |
| 32 | R2 | 6 | 0.0 | 0.090 | 11.9 | LOS A | 0.2 | 1.4 | 0.09 | 0.03 | 0.09 | 64.4 |
| Approach |  | 147 | 8.8 | 0.090 | 1.1 | NA | 0.2 | 1.4 | 0.09 | 0.03 | 0.09 | 77.3 |
| All Vehicles |  | 540 | 5.0 | 0.200 | 0.5 | NA | 0.2 | 1.4 | 0.03 | 0.03 | 0.03 | 78.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [5. Greenwell Pt-Mayfield (Ex Fri PM-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Mayfield Road
Friday PM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{gathered} \text { Flows } \\ \text { HV } \\ \% \end{gathered}$ | $\begin{array}{r} \text { Deg. } \\ \text { Satn } \\ \mathrm{v} / \mathrm{c} \end{array}$ | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Mayfield Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 6 | 0.0 | 0.033 | 6.4 | LOS A | 0.1 | 0.9 | 0.49 | 0.66 | 0.49 | 52.9 |
| 23 | R2 | 6 | 25.0 | 0.033 | 18.7 | LOS B | 0.1 | 0.9 | 0.49 | 0.66 | 0.49 | 47.3 |
| Appr |  | 13 | 12.5 | 0.033 | 12.5 | LOS A | 0.1 | 0.9 | 0.49 | 0.66 | 0.49 | 50.0 |
| NorthEast: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 1 | 0.0 | 0.097 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 74.6 |
| 25 | T1 | 181 | 5.7 | 0.097 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| Appr |  | 182 | 5.7 | 0.097 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| SouthWest: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |  |  |
| 31 | T1 | 413 | 1.4 | 0.236 | 0.2 | LOS A | 0.3 | 2.2 | 0.05 | 0.02 | 0.05 | 79.1 |
| 32 | R2 | 15 | 0.0 | 0.236 | 9.1 | LOS A | 0.3 | 2.2 | 0.05 | 0.02 | 0.05 | 65.1 |
| Approach |  | 427 | 1.4 | 0.236 | 0.5 | NA | 0.3 | 2.2 | 0.05 | 0.02 | 0.05 | 78.5 |
| All Vehicles |  | 622 | 2.9 | 0.236 | 0.6 | NA | 0.3 | 2.2 | 0.04 | 0.03 | 0.04 | 78.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [5. Greenwell Pt-Mayfield (Ex Sat-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Mayfield Road
Saturday - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Deman <br> Total veh/h | $\begin{gathered} =\text { lows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Mayfield Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 8 | 0.0 | 0.012 | 6.8 | LOS A | 0.0 | 0.3 | 0.38 | 0.59 | 0.38 | 56.9 |
| 23 | R2 | 1 | 0.0 | 0.012 | 13.2 | LOS A | 0.0 | 0.3 | 0.38 | 0.59 | 0.38 | 56.5 |
| Appr |  | 9 | 0.0 | 0.012 | 7.5 | LOS A | 0.0 | 0.3 | 0.38 | 0.59 | 0.38 | 56.9 |
| NorthEast: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 4 | 66.7 | 0.139 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 53.1 |
| 25 | T1 | 263 | 1.0 | 0.139 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 79.9 |
| Appr |  | 267 | 2.0 | 0.139 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 79.3 |
| SouthWest: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |  |  |
| 31 | T1 | 289 | 1.8 | 0.170 | 0.3 | LOS A | 0.3 | 2.0 | 0.06 | 0.03 | 0.06 | 78.8 |
| 32 | R2 | 12 | 0.0 | 0.170 | 10.2 | LOS A | 0.3 | 2.0 | 0.06 | 0.03 | 0.06 | 64.9 |
| Approach |  | 301 | 1.7 | 0.170 | 0.7 | NA | 0.3 | 2.0 | 0.06 | 0.03 | 0.06 | 78.1 |
| All Vehicles |  | 578 | 1.9 | 0.170 | 0.6 | NA | 0.3 | 2.0 | 0.04 | 0.03 | 0.04 | 78.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## Site: 1 [6. Greenwell Pt-Millbank-Worrigee (Ex Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Millbank Road-Worrigee Road
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand Total veh/h |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Worrigee Road |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 69 | 5.1 | 0.091 | 10.5 | LOS A | 0.3 | 2.4 | 0.44 | 0.92 | 0.44 | 54.1 |
| 2 T1 | 118 | 3.0 | 0.406 | 20.8 | LOS B | 2.0 | 14.5 | 0.76 | 1.08 | 1.04 | 47.4 |
| 3 R2 | 21 | 0.0 | 0.406 | 22.4 | LOS B | 2.0 | 14.5 | 0.76 | 1.08 | 1.04 | 48.0 |
| Approach | 208 | 3.4 | 0.406 | 17.5 | LOS B | 2.0 | 14.5 | 0.66 | 1.03 | 0.84 | 49.5 |
| East: Greenwell Point Road (E) |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 12 | 0.0 | 0.193 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 74.3 |
| $5 \quad$ T1 | 357 | 3.0 | 0.193 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 79.5 |
| 6 R2 | 32 | 7.4 | 0.028 | 7.8 | LOS A | 0.1 | 0.8 | 0.30 | 0.61 | 0.30 | 61.6 |
| Approach | 400 | 3.2 | 0.193 | 0.8 | NA | 0.1 | 0.8 | 0.02 | 0.07 | 0.02 | 77.6 |
| North: Millbank Road |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 14 | 16.7 | 0.015 | 10.6 | LOS A | 0.1 | 0.4 | 0.29 | 0.88 | 0.29 | 58.0 |
| 8 T1 | 26 | 13.6 | 0.135 | 19.2 | LOS B | 0.5 | 3.7 | 0.73 | 1.01 | 0.73 | 48.9 |
| 9 R2 | 12 | 0.0 | 0.135 | 24.6 | LOS B | 0.5 | 3.7 | 0.73 | 1.01 | 0.73 | 53.7 |
| Approach | 52 | 11.4 | 0.135 | 18.1 | LOS B | 0.5 | 3.7 | 0.61 | 0.97 | 0.61 | 52.1 |
| West: Greenwell Point Road (W) |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 47 | 7.5 | 0.105 | 7.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.16 | 0.00 | 69.0 |
| 11 T1 | 143 | 9.1 | 0.105 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.16 | 0.00 | 77.0 |
| 12 R 2 | 61 | 5.8 | 0.065 | 8.7 | LOS A | 0.3 | 1.8 | 0.44 | 0.68 | 0.44 | 56.9 |
| Approach | 252 | 8.0 | 0.105 | 3.5 | NA | 0.3 | 1.8 | 0.11 | 0.29 | 0.11 | 69.6 |
| All Vehicles | 912 | 5.0 | 0.406 | 6.3 | NA | 2.0 | 14.5 | 0.22 | 0.40 | 0.27 | 65.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## Site: 1 [6. Greenwell Pt-Millbank-Worrigee (Ex Fri PM-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Millbank Road-Worrigee Road
Friday PM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | $\begin{gathered} \text { Demanc } \\ \text { Total } \\ \text { veh/h } \\ \hline \end{gathered}$ | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles $\qquad$ | of Queue Distance m $\qquad$ | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Worrigee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 58 | 0.0 | 0.058 | 8.9 | LOS A | 0.2 | 1.5 | 0.29 | 0.88 | 0.29 | 56.3 |
| 2 | T1 | 47 | 0.0 | 0.249 | 19.7 | LOS B | 1.0 | 6.9 | 0.77 | 1.02 | 0.85 | 47.6 |
| 3 | R2 | 21 | 5.3 | 0.249 | 25.9 | LOS B | 1.0 | 6.9 | 0.77 | 1.02 | 0.85 | 46.6 |
| Appr |  | 126 | 0.9 | 0.249 | 15.8 | LOS B | 1.0 | 6.9 | 0.55 | 0.96 | 0.59 | 51.0 |
| East: Greenwell Point Road (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 21 | 7.1 | 0.099 | 7.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.07 | 0.00 | 70.5 |
| 5 | T1 | 163 | 5.5 | 0.099 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.07 | 0.00 | 78.6 |
| 6 | R2 | 14 | 0.0 | 0.015 | 8.7 | LOS A | 0.1 | 0.4 | 0.45 | 0.65 | 0.45 | 63.1 |
| Appr |  | 198 | 5.3 | 0.099 | 1.4 | NA | 0.1 | 0.4 | 0.03 | 0.11 | 0.03 | 76.4 |
| North: Millbank Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 22 | 0.0 | 0.030 | 11.3 | LOS A | 0.1 | 0.7 | 0.45 | 0.89 | 0.45 | 61.7 |
| 8 | T1 | 54 | 2.8 | 0.259 | 21.3 | LOS B | 1.0 | 7.3 | 0.77 | 1.02 | 0.87 | 48.0 |
| 9 | R2 | 18 | 0.0 | 0.259 | 26.7 | LOS B | 1.0 | 7.3 | 0.77 | 1.02 | 0.87 | 52.1 |
| Appr |  | 94 | 1.6 | 0.259 | 19.9 | LOS B | 1.0 | 7.3 | 0.70 | 0.99 | 0.77 | 51.5 |
| West: Greenwell Point Road (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 17 | 0.0 | 0.221 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.03 | 0.00 | 74.2 |
| 11 | T1 | 409 | 0.7 | 0.221 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.03 | 0.00 | 79.4 |
| 12 | R2 | 115 | 1.3 | 0.097 | 7.6 | LOS A | 0.4 | 2.8 | 0.31 | 0.62 | 0.31 | 57.8 |
| Approach |  | 541 | 0.8 | 0.221 | 1.9 | NA | 0.4 | 2.8 | 0.06 | 0.15 | 0.06 | 73.4 |
| All Vehicles |  | 959 | 1.8 | 0.259 | 5.4 | NA | 1.0 | 7.3 | 0.18 | 0.33 | 0.20 | 67.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## Site: 1 [6. Greenwell Pt-Millbank-Worrigee (Ex Sat-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Millbank Road-Worrigee Road
Saturday - Equivalent 120th HH
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | Turn | Demand <br> Total veh/h |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Worrigee Road 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 77 | 0.0 | 0.084 | 9.4 | LOS A | 0.3 | 2.1 | 0.35 | 0.89 | 0.35 | 56.0 |
| 2 | T1 | 29 | 4.5 | 0.192 | 15.9 | LOS B | 0.7 | 5.2 | 0.67 | 1.00 | 0.67 | 49.7 |
| 3 | R2 | 38 | 3.4 | 0.192 | 17.8 | LOS B | 0.7 | 5.2 | 0.67 | 1.00 | 0.67 | 49.8 |
| Appr |  | 144 | 1.8 | 0.192 | 12.9 | LOS A | 0.7 | 5.2 | 0.50 | 0.94 | 0.50 | 52.9 |
| East: Greenwell Point Road (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 20 | 6.7 | 0.136 | 7.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.05 | 0.00 | 71.0 |
| 5 | T1 | 241 | 1.1 | 0.136 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.05 | 0.00 | 79.1 |
| 6 | R2 | 16 | 0.0 | 0.014 | 7.9 | LOS A | 0.1 | 0.4 | 0.35 | 0.61 | 0.35 | 63.7 |
| Appr |  | 277 | 1.4 | 0.136 | 1.0 | NA | 0.1 | 0.4 | 0.02 | 0.08 | 0.02 | 77.4 |
| North: Millbank Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 8 | 16.7 | 0.010 | 11.2 | LOS A | 0.0 | 0.3 | 0.36 | 0.87 | 0.36 | 57.6 |
| 8 | T1 | 24 | 11.1 | 0.119 | 17.3 | LOS B | 0.4 | 3.2 | 0.67 | 1.01 | 0.67 | 50.6 |
| 9 | R2 | 16 | 0.0 | 0.119 | 19.5 | LOS B | 0.4 | 3.2 | 0.67 | 1.01 | 0.67 | 55.7 |
| Appr |  | 48 | 8.5 | 0.119 | 17.0 | LOS B | 0.4 | 3.2 | 0.62 | 0.98 | 0.62 | 53.3 |
| West: Greenwell Point Road (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 21 | 6.3 | 0.143 | 7.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.05 | 0.00 | 71.2 |
| 11 | T1 | 253 | 1.0 | 0.143 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.05 | 0.00 | 79.1 |
| 12 | R2 | 54 | 0.0 | 0.049 | 7.9 | LOS A | 0.2 | 1.3 | 0.35 | 0.63 | 0.35 | 57.7 |
| Approach |  | 327 | 1.2 | 0.143 | 1.8 | NA | 0.2 | 1.3 | 0.06 | 0.15 | 0.06 | 74.0 |
| All Vehicles |  | 797 | 1.8 | 0.192 | 4.4 | NA | 0.7 | 5.2 | 0.16 | 0.32 | 0.16 | 68.5 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 1 [7. Princes Hwy-Kalandar (Ex Fri AM-120th HH)]
13 1231000 - West Culburra Subdivision
Princes Highway-Kalandar Street
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Signals - Fixed Time Coordinated Cycle Time $=135$ seconds (Site Optimum Cycle Time - Minimum Delay)
Variable Sequence Analysis applied. The results are given for the selected output sequence.
Timings based on settings in the Site Phasing \& Timing dialog
Phase Times determined by the program
Phase Sequence: GTA - RMS/Video survey phasing
Reference Phase: Phase A
Input Phase Sequence: A, D, E, F, F1*, F2*
Output Phase Sequence: A, D, E, F
(* Variable Phase)
Movement Performance - Vehicles


Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 1 [7. Princes Hwy-Kalandar (Ex Fri PM-120th HH)]
13S1231000 - West Culburra Subdivision
Princes Highway-Kalandar Street
Friday PM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Signals - Fixed Time Coordinated Cycle Time $=115$ seconds (Site Optimum Cycle Time - Minimum Delay)
Variable Sequence Analysis applied. The results are given for the selected output sequence.
Timings based on settings in the Site Phasing \& Timing dialog
Phase Times determined by the program
Phase Sequence: GTA - RMS/Video survey phasing
Reference Phase: Phase A
Input Phase Sequence: A, D, E, F, F1*, F2*
Output Phase Sequence: A, D, E, F
(* Variable Phase)
Movement Performance - Vehicles

| Mov ID | Turn | Demand <br> Total veh/h | $\begin{array}{r} \text { lows } \\ \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 12 | 0.0 | 0.665 | 44.8 | LOS D | 18.6 | 133.0 | 0.84 | 0.75 | 1.16 | 35.3 |
| 2 | T1 | 785 | 2.6 | 0.665 | 35.9 | LOS C | 18.6 | 133.0 | 0.83 | 0.73 | 0.99 | 41.4 |
| 3 | R2 | 109 | 1.4 | 0.856 | 72.9 | LOS F | 6.9 | 48.8 | 1.00 | 0.92 | 1.39 | 24.1 |
| Appr |  | 906 | 2.4 | 0.856 | 40.5 | LOS C | 18.6 | 133.0 | 0.85 | 0.75 | 1.04 | 38.7 |
| East: Kalandar St (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 99 | 1.5 | 1.182 | 217.1 | LOS F | 53.5 | 380.8 | 1.00 | 1.60 | 2.52 | 9.8 |
| 5 | T1 | 245 | 2.4 | 1.182 | 211.5 | LOS F | 53.5 | 380.8 | 1.00 | 1.60 | 2.52 | 8.1 |
| 6 | R2 | 567 | 1.8 | 1.182 | 229.4 | LOS F | 58.5 | 415.9 | 1.00 | 1.55 | 2.52 | 9.9 |
| Appr |  | 912 | 2.0 | 1.182 | 223.2 | LOS F | 58.5 | 415.9 | 1.00 | 1.57 | 2.52 | 9.4 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 832 | 1.1 | 0.657 | 8.8 | LOS A | 5.2 | 36.4 | 0.18 | 0.70 | 0.30 | 54.1 |
| 8 | T1 | 1302 | 3.1 | 1.242 | 269.9 | LOS F | 106.6 | 766.3 | 1.00 | 2.05 | 2.72 | 11.1 |
| 9 | R2 | 138 | 5.4 | 1.125 | 188.8 | LOS F | 15.6 | 113.9 | 1.00 | 1.30 | 2.42 | 12.5 |
| Approach |  | 2272 | 2.5 | 1.242 | 169.4 | LOS F | 106.6 | 766.3 | 0.70 | 1.51 | 1.82 | 14.9 |
| West: Kalandar St (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & 10 \\ & 11 \\ & 12 \end{aligned}$ | L2 | 109 | 9.5 | 1.164 | 218.4 | LOS F | 52.3 | 373.9 | 1.00 | 1.80 | 2.45 | 11.4 |
|  | T1 | 305 | 0.0 | 1.164 | 212.6 | LOS F | 52.3 | 373.9 | 1.00 | 1.80 | 2.45 | 8.8 |
|  | R2 | 303 | 1.0 | 0.900 | 69.1 | LOS E | 19.7 | 139.2 | 1.00 | 0.98 | 1.33 | 25.9 |
| Approach |  | 718 | 1.9 | 1.164 | 152.9 | LOS F | 52.3 | 373.9 | 1.00 | 1.46 | 1.98 | 13.6 |
| All Vehicles |  | 4807 | 2.3 | 1.242 | 152.8 | LOS F | 106.6 | 766.3 | 0.83 | 1.37 | 1.83 | 15.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 1 [7. Princes Hwy-Kalandar (Ex Sat-120th HH)]
13S1231000 - West Culburra Subdivision
Princes Highway-Kalandar Street
Saturday - Equivalent 120th HH
Site Category: (None)
Signals - Fixed Time Coordinated Cycle Time $=135$ seconds (Site Optimum Cycle Time - Minimum Delay)
Variable Sequence Analysis applied. The results are given for the selected output sequence.
Timings based on settings in the Site Phasing \& Timing dialog
Phase Times determined by the program
Phase Sequence: GTA - RMS/Video survey phasing
Reference Phase: Phase A
Input Phase Sequence: A, D, E, F, F1*, F2*
Output Phase Sequence: A, D, E, F
(* Variable Phase)
Movement Performance - Vehicles


Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [8. Coonamia-Currarong-Forest (Ex Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Coonamia Road- Currarong Road-Forest Road
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Currarong Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 13 | 0.0 | 0.041 | 0.8 | LOS A | 0.2 | 1.2 | 0.31 | 0.49 | 0.31 | 83.7 |
| 6 | R2 | 40 | 0.0 | 0.041 | 8.5 | LOS A | 0.2 | 1.2 | 0.31 | 0.49 | 0.31 | 76.2 |
| Appr |  | 53 | 0.0 | 0.041 | 6.7 | NA | 0.2 | 1.2 | 0.31 | 0.49 | 0.31 | 77.9 |
| North: Coonamia Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 7 | 0.0 | 0.007 | 8.3 | LOS A | 0.0 | 0.2 | 0.22 | 0.60 | 0.22 | 74.0 |
| 9 | R2 | 67 | 1.8 | 0.097 | 9.2 | LOS A | 0.4 | 2.7 | 0.36 | 0.67 | 0.36 | 71.5 |
| Appro |  | 75 | 1.6 | 0.097 | 9.1 | LOS A | 0.4 | 2.7 | 0.34 | 0.66 | 0.34 | 71.7 |
| West: Forest Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 197 | 1.8 | 0.117 | 7.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 0.00 | 74.8 |
| 11 | T1 | 19 | 6.3 | 0.117 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 0.00 | 83.6 |
| Approach |  | 216 | 2.2 | 0.117 | 7.2 | NA | 0.0 | 0.0 | 0.00 | 0.61 | 0.00 | 75.5 |
| All Vehicles |  | 343 | 1.7 | 0.117 | 7.5 | NA | 0.4 | 2.7 | 0.12 | 0.60 | 0.12 | 75.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [8. Coonamia-Currarong-Forest (Ex Fri PM-120th HH)]

13S1231000 - West Culburra Subdivision
Coonamia Road- Currarong Road-Forest Road
Friday PM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman <br> Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average <br> Speed km/h |
| East: Currarong Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 15 | 0.0 | 0.020 | 0.5 | LOS A | 0.1 | 0.6 | 0.24 | 0.32 | 0.24 | 88.1 |
| 6 | R2 | 15 | 0.0 | 0.020 | 8.3 | LOS A | 0.1 | 0.6 | 0.24 | 0.32 | 0.24 | 79.8 |
| Appr |  | 29 | 0.0 | 0.020 | 4.4 | NA | 0.1 | 0.6 | 0.24 | 0.32 | 0.24 | 83.8 |
| North: Coonamia Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 37 | 0.0 | 0.034 | 8.3 | LOS A | 0.1 | 0.9 | 0.21 | 0.61 | 0.21 | 74.0 |
| 9 | R2 | 185 | 4.0 | 0.256 | 9.2 | LOS A | 1.2 | 8.5 | 0.36 | 0.67 | 0.36 | 70.8 |
| Appr |  | 222 | 3.3 | 0.256 | 9.0 | LOS A | 1.2 | 8.5 | 0.34 | 0.66 | 0.34 | 71.3 |
| West: Forest Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 137 | 5.4 | 0.094 | 8.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.54 | 0.00 | 74.8 |
| 11 | T1 | 34 | 4.3 | 0.094 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.54 | 0.00 | 85.3 |
| Approach |  | 171 | 5.2 | 0.094 | 6.4 | NA | 0.0 | 0.0 | 0.00 | 0.54 | 0.00 | 76.7 |
| All Vehicles |  | 422 | 3.9 | 0.256 | 7.6 | NA | 1.2 | 8.5 | 0.19 | 0.59 | 0.19 | 74.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [8. Coonamia-Currarong-Forest (Ex Sat-120th HH)]

13S1231000 - West Culburra Subdivision
Coonamia Road- Currarong Road-Forest Road
Saturday - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | Demand <br> Total veh/h | $\begin{gathered} \text { lows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Currarong Road |  |  |  |  |  |  |  |  |  |  |  |
| $5 \quad$ T1 | 11 | 0.0 | 0.027 | 0.6 | LOS A | 0.1 | 0.8 | 0.27 | 0.45 | 0.27 | 84.7 |
| 6 R2 | 26 | 0.0 | 0.027 | 8.3 | LOS A | 0.1 | 0.8 | 0.27 | 0.45 | 0.27 | 77.0 |
| Approach | 37 | 0.0 | 0.027 | 6.1 | NA | 0.1 | 0.8 | 0.27 | 0.45 | 0.27 | 79.1 |
| North: Coonamia Road |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 29 | 0.0 | 0.027 | 8.3 | LOS A | 0.1 | 0.7 | 0.19 | 0.61 | 0.19 | 74.1 |
| 9 R2 | 151 | 1.8 | 0.205 | 8.9 | LOS A | 0.9 | 6.4 | 0.34 | 0.66 | 0.34 | 71.8 |
| Approach | 180 | 1.5 | 0.205 | 8.8 | LOS A | 0.9 | 6.4 | 0.32 | 0.66 | 0.32 | 72.2 |
| West: Forest Road |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 152 | 0.9 | 0.092 | 7.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 0.00 | 75.5 |
| 11 T1 | 19 | 0.0 | 0.092 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 0.00 | 84.0 |
| Approach | 171 | 0.8 | 0.092 | 7.0 | NA | 0.0 | 0.0 | 0.00 | 0.59 | 0.00 | 76.4 |
| All Vehicles | 387 | 1.0 | 0.205 | 7.8 | NA | 0.9 | 6.4 | 0.17 | 0.61 | 0.17 | 74.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.
(7 Site: 1 [9. Kalandar St-Kinghorne St (Ex Fri AM-120th HH)]
13 S1231000 - West Culburra Subdivision
Kalandar Street-Kinghorne Street-Albatross Road
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Demand Total veh/h | $\begin{gathered} \text { Hows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Kinghorne Sttreet |  |  |  |  |  |  |  |  |  |  |  |  |
| 1b | L3 | 31 | 7.7 | 0.730 | 17.4 | LOS B | 10.3 | 74.0 | 0.99 | 1.13 | 1.46 | 45.5 |
| 2 | T1 | 483 | 3.4 | 0.730 | 17.1 | LOS B | 10.3 | 74.0 | 0.99 | 1.13 | 1.46 | 46.7 |
| 3 | R2 | 59 | 0.0 | 0.730 | 20.8 | LOS B | 10.3 | 74.0 | 0.99 | 1.13 | 1.46 | 46.6 |
| Appr |  | 573 | 3.3 | 0.730 | 17.5 | LOS B | 10.3 | 74.0 | 0.99 | 1.13 | 1.46 | 46.6 |
| East: Kalandar Street |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 29 | 0.0 | 0.429 | 5.7 | LOS A | 2.7 | 20.2 | 0.46 | 0.62 | 0.46 | 52.4 |
| 4a | L1 | 285 | 8.3 | 0.429 | 5.8 | LOS A | 2.7 | 20.2 | 0.46 | 0.62 | 0.46 | 53.0 |
| 6 | R2 | 156 | 2.3 | 0.429 | 9.9 | LOS A | 2.7 | 20.2 | 0.46 | 0.62 | 0.46 | 53.2 |
| Appr |  | 471 | 5.8 | 0.429 | 7.1 | LOS A | 2.7 | 20.2 | 0.46 | 0.62 | 0.46 | 53.0 |
| North: Kinghorne Street |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 127 | 3.7 | 0.334 | 6.1 | LOS A | 2.3 | 16.5 | 0.59 | 0.68 | 0.59 | 51.8 |
| 8 | T1 | 31 | 0.0 | 0.334 | 6.3 | LOS A | 2.3 | 16.5 | 0.59 | 0.68 | 0.59 | 53.0 |
| 9a | R1 | 176 | 3.4 | 0.334 | 9.3 | LOS A | 2.3 | 16.5 | 0.59 | 0.68 | 0.59 | 52.2 |
| Appr |  | 334 | 3.2 | 0.334 | 7.8 | LOS A | 2.3 | 16.5 | 0.59 | 0.68 | 0.59 | 52.1 |
| SouthWest: Albatross Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 30a | L1 | 261 | 4.5 | 0.753 | 19.1 | LOS B | 10.1 | 74.2 | 1.00 | 1.22 | 1.55 | 44.6 |
| 32a | R1 | 206 | 7.4 | 0.753 | 22.7 | LOS B | 10.1 | 74.2 | 1.00 | 1.22 | 1.55 | 44.3 |
| 32b | R3 | 11 | 22.2 | 0.753 | 25.4 | LOS B | 10.1 | 74.2 | 1.00 | 1.22 | 1.55 | 44.4 |
| Appr |  | 478 | 6.2 | 0.753 | 20.8 | LOS B | 10.1 | 74.2 | 1.00 | 1.22 | 1.55 | 44.5 |
| All Ve | icles | 1855 | 4.6 | 0.753 | 14.0 | LOS A | 10.3 | 74.2 | 0.79 | 0.94 | 1.07 | 48.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.
(7 Site: 1 [9. Kalandar St-Kinghorne St (Ex Fri PM-120th HH)]
13S1231000 - West Culburra Subdivision
Kalandar Street-Kinghorne Street-Albatross Road
Friday AM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Total veh/h |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Kinghorne Sttreet |  |  |  |  |  |  |  |  |  |  |  |  |
| 1b | L3 | 34 | 0.0 | 0.572 | 12.3 | LOS A | 5.6 | 39.3 | 0.90 | 0.97 | 1.10 | 48.4 |
| 2 | T1 | 326 | 0.0 | 0.572 | 12.3 | LOS A | 5.6 | 39.3 | 0.90 | 0.97 | 1.10 | 49.6 |
| 3 | R2 | 77 | 0.0 | 0.572 | 16.1 | LOS B | 5.6 | 39.3 | 0.90 | 0.97 | 1.10 | 49.4 |
| Appro |  | 437 | 0.0 | 0.572 | 13.0 | LOS A | 5.6 | 39.3 | 0.90 | 0.97 | 1.10 | 49.5 |
| East: Kalandar Street |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 40 | 0.0 | 0.435 | 7.0 | LOS A | 2.9 | 20.1 | 0.62 | 0.72 | 0.62 | 51.9 |
| 4a | L1 | 238 | 0.0 | 0.435 | 6.9 | LOS A | 2.9 | 20.1 | 0.62 | 0.72 | 0.62 | 52.6 |
| 6 | R2 | 115 | 0.0 | 0.435 | 11.1 | LOS A | 2.9 | 20.1 | 0.62 | 0.72 | 0.62 | 52.7 |
| Appro |  | 393 | 0.0 | 0.435 | 8.1 | LOS A | 2.9 | 20.1 | 0.62 | 0.72 | 0.62 | 52.6 |
| North: Kinghorne Street |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 315 | 0.0 | 0.764 | 12.3 | LOS A | 10.9 | 76.2 | 0.97 | 1.01 | 1.28 | 48.2 |
| 8 | T1 | 80 | 0.0 | 0.764 | 12.6 | LOS A | 10.9 | 76.2 | 0.97 | 1.01 | 1.28 | 49.1 |
| 9a | R1 | 304 | 0.0 | 0.764 | 15.5 | LOS B | 10.9 | 76.2 | 0.97 | 1.01 | 1.28 | 48.6 |
| Appro |  | 699 | 0.0 | 0.764 | 13.7 | LOS A | 10.9 | 76.2 | 0.97 | 1.01 | 1.28 | 48.5 |
| SouthWest: Albatross Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 30a | L1 | 300 | 0.0 | 0.771 | 14.7 | LOS B | 11.2 | 78.3 | 1.00 | 1.11 | 1.43 | 47.1 |
| 32a | R1 | 326 | 0.0 | 0.771 | 18.0 | LOS B | 11.2 | 78.3 | 1.00 | 1.11 | 1.43 | 46.9 |
| 32b | R3 | 15 | 0.0 | 0.771 | 19.9 | LOS B | 11.2 | 78.3 | 1.00 | 1.11 | 1.43 | 47.5 |
| Appro |  | 641 | 0.0 | 0.771 | 16.5 | LOS B | 11.2 | 78.3 | 1.00 | 1.11 | 1.43 | 47.0 |
| All Ve | icles | 2169 | 0.0 | 0.771 | 13.4 | LOS A | 11.2 | 78.3 | 0.90 | 0.98 | 1.17 | 48.9 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

F Site: 1 [9. Kalandar St-Kinghorne St (Ex Sat-120th HH)]
13S1231000 - West Culburra Subdivision
Kalandar Street-Kinghorne Street-Albatross Road
Saturday - Equivalent 120th HH
Site Category: (None)
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Demand <br> Total <br> veh/h | $\begin{gathered} \text { Flows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. <br> Satn <br> v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Kinghorne Street 1 |  |  |  |  |  |  |  |  |  |  |  |  |
| 1b | L3 | 21 | 12.5 | 0.225 | 7.5 | LOS A | 1.4 | 10.0 | 0.60 | 0.66 | 0.60 | 51.2 |
| 2 | T1 | 162 | 1.6 | 0.225 | 7.2 | LOS A | 1.4 | 10.0 | 0.60 | 0.66 | 0.60 | 53.0 |
| 3 | R2 | 29 | 0.0 | 0.225 | 11.0 | LOS A | 1.4 | 10.0 | 0.60 | 0.66 | 0.60 | 52.8 |
| Appr |  | 213 | 2.5 | 0.225 | 7.8 | LOS A | 1.4 | 10.0 | 0.60 | 0.66 | 0.60 | 52.8 |
| East: Kalandar Street |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 22 | 0.0 | 0.266 | 5.4 | LOS A | 1.4 | 10.5 | 0.38 | 0.59 | 0.38 | 52.7 |
| 4a | L1 | 177 | 6.7 | 0.266 | 5.4 | LOS A | 1.4 | 10.5 | 0.38 | 0.59 | 0.38 | 53.3 |
| 6 | R2 | 96 | 1.4 | 0.266 | 9.6 | LOS A | 1.4 | 10.5 | 0.38 | 0.59 | 0.38 | 53.5 |
| Appr |  | 295 | 4.5 | 0.266 | 6.8 | LOS A | 1.4 | 10.5 | 0.38 | 0.59 | 0.38 | 53.3 |
| North: Kinghorne Street |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 152 | 0.8 | 0.312 | 5.9 | LOS A | 2.0 | 14.3 | 0.53 | 0.65 | 0.53 | 52.2 |
| 8 | T1 | 21 | 0.0 | 0.312 | 6.1 | LOS A | 2.0 | 14.3 | 0.53 | 0.65 | 0.53 | 53.3 |
| 9a | R1 | 157 | 2.6 | 0.312 | 9.1 | LOS A | 2.0 | 14.3 | 0.53 | 0.65 | 0.53 | 52.6 |
| Appr |  | 329 | 1.6 | 0.312 | 7.5 | LOS A | 2.0 | 14.3 | 0.53 | 0.65 | 0.53 | 52.4 |
| SouthWest: Albatross Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 30a | L1 | 161 | 1.4 | 0.375 | 6.1 | LOS A | 2.6 | 18.3 | 0.57 | 0.67 | 0.57 | 52.5 |
| 32a | R1 | 214 | 0.6 | 0.375 | 9.4 | LOS A | 2.6 | 18.3 | 0.57 | 0.67 | 0.57 | 52.2 |
| 32b | R3 | 20 | 13.3 | 0.375 | 11.6 | LOS A | 2.6 | 18.3 | 0.57 | 0.67 | 0.57 | 52.4 |
| Appr |  | 395 | 1.6 | 0.375 | 8.1 | LOS A | 2.6 | 18.3 | 0.57 | 0.67 | 0.57 | 52.3 |
| All Ve | icles | 1232 | 2.4 | 0.375 | 7.6 | LOS A | 2.6 | 18.3 | 0.52 | 0.65 | 0.52 | 52.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

$\nabla$ Site: 1 [10a. Princes Hwy-Forest (Ex Fri AM-120th HH)]
的叟 Network: 1 [10. Princes Hwy-Forest (Ex Fri AM-120th

Site Category: -
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand <br> Total veh/h | Flows HV \% | Arrival Total veh/h | Flows HV $\%$ | Deg. Satn v/c | Average Delay sec | Level of Service |  | Back of ue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles $\qquad$ | Averag Speed km/h |
| South: Princes Highway |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 27 | 11.5 | 27 | 11.5 | 0.037 | 11.2 | LOS A | 0.1 | 0.4 | 0.52 | 0.74 | 0.52 | 66.4 |
| Approach | 27 | 11.5 | 27 | 11.5 | 0.037 | 11.2 | NA | 0.1 | 0.4 | 0.52 | 0.74 | 0.52 | 66.4 |
| East: Forest Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 36 | 2.9 | 36 | 2.9 | 0.029 | 8.8 | LOS A | 0.0 | 0.3 | 0.34 | 0.63 | 0.34 | 71.4 |
| 6 R 1 | 102 | 2.1 | 102 | 2.1 | 0.234 | 14.5 | LOSA | 0.4 | 2.7 | 0.65 | 0.87 | 0.68 | 56.3 |
| Approach | 138 | 2.3 | 138 | 2.3 | 0.234 | 13.0 | LOSA | 0.4 | 2.7 | 0.57 | 0.81 | 0.60 | 61.6 |
| North: Princes Highway |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 58 | 9.1 | 58 | 9.1 | 0.038 | 8.6 | LOSA | 0.1 | 0.5 | 0.09 | 0.61 | 0.09 | 70.7 |
| 8 T1 | 521 | 16.0 | 521 | 16.0 | 0.147 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 99.9 |
| Approach | 579 | 15.3 | 579 | 15.3 | 0.147 | 0.9 | LOS A | 0.1 | 0.5 | 0.01 | 0.06 | 0.01 | 95.9 |
| All Vehicles | 744 | 12.7 | 744 | 12.7 | 0.234 | 3.5 | NA | 0.4 | 2.7 | 0.13 | 0.22 | 0.14 | 88.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^5]Site Category: -
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Demand Flows Arrival Flows Total HV Total HV |  |  |  | Deg. Satn v/c | Average Delay sec | Level of Service | Aver. Back of Queue Prop. Vehicles Distance Queued veh m |  |  | Effective Aver. No.Average Stop Cycles Speed Rate |  |  |
|  | veh/h |  | veh/h | \% |  |  |  |  |  |  |  |  | km/h |
| South: Princes Highway |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 51 | 2.1 | 51 | 2.1 | 0.207 | 23.5 | LOS B | 0.3 | 2.0 | 0.87 | 0.96 | 0.92 | 56.1 |
| Approach | 51 | 2.1 | 51 | 2.1 | 0.207 | 23.5 | NA | 0.3 | 2.0 | 0.87 | 0.96 | 0.92 | 56.1 |
| East: Forest Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 49 | 0.0 | 49 | 0.0 | 0.063 | 11.1 | LOS A | 0.1 | 0.6 | 0.57 | 0.80 | 0.57 | 70.6 |
| 6a R1 | 66 | 1.6 | 66 | 1.6 | 0.788 | 102.5 | LOS F | 1.3 | 9.3 | 0.98 | 1.11 | 1.75 | 15.3 |
| Approach | 116 | 0.9 | 116 | 0.9 | 0.788 | 63.5 | LOS E | 1.3 | 9.3 | 0.81 | 0.98 | 1.25 | 28.7 |
| North: Princes Highway |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $7 \quad$ L2 | 148 | 2.1 | 148 | 2.1 | 0.095 | 8.5 | LOS A | 0.2 | 1.2 | 0.13 | 0.60 | 0.13 | 72.7 |
| 8 T1 | 1475 | 2.0 | 1475 | 2.0 | 0.383 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 99.7 |
| Approach | 1623 | 2.0 | 1623 | 2.0 | 0.383 | 0.8 | LOS A | 0.2 | 1.2 | 0.01 | 0.06 | 0.01 | 96.4 |
| All Vehicles | 1789 | 1.9 | 1789 | 1.9 | 0.788 | 5.5 | NA | 1.3 | 9.3 | 0.09 | 0.14 | 0.12 | 85.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site Category: -
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | Demand Flows Arrival Flows Total HV Total HV |  |  |  | Deg. Satn v/c | Average Delay sec | Level of Service | Aver. Back of Queue Prop. Vehicles Distance Queued |  |  | Effective Aver. No.Average Stop Cycles Speed |  |  |
|  | veh/h |  | veh/h | \% |  |  |  | veh | m |  |  |  | km/h |
| South: Princes Highway |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 40 | 0.0 | 40 | 0.0 | 0.117 | 17.8 | LOS B | 0.2 | 1.1 | 0.78 | 0.93 | 0.78 | 62.5 |
| Approach | 40 | 0.0 | 40 | 0.0 | 0.117 | 17.8 | NA | 0.2 | 1.1 | 0.78 | 0.93 | 0.78 | 62.5 |
| East: Forest Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 45 | 0.0 | 45 | 0.0 | 0.050 | 10.2 | LOS A | 0.1 | 0.5 | 0.51 | 0.74 | 0.51 | 71.8 |
| 6a R1 | 88 | 3.6 | 88 | 3.6 | 0.703 | 62.3 | LOS E | 1.3 | 9.0 | 0.96 | 1.10 | 1.65 | 23.0 |
| Approach | 134 | 2.4 | 134 | 2.4 | 0.703 | 44.7 | LOS D | 1.3 | 9.0 | 0.81 | 0.98 | 1.27 | 34.9 |
| North: Princes Highway |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 125 | 0.0 | 125 | 0.0 | 0.079 | 8.4 | LOS A | 0.1 | 0.9 | 0.11 | 0.60 | 0.11 | 74.6 |
| 8 T1 | 1153 | 1.7 | 1153 | 1.7 | 0.299 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 99.8 |
| Approach | 1278 | 1.6 | 1278 | 1.6 | 0.299 | 0.9 | LOS A | 0.1 | 0.9 | 0.01 | 0.06 | 0.01 | 96.6 |
| All Vehicles | 1452 | 1.6 | 1452 | 1.6 | 0.703 | 5.4 | NA | 1.3 | 9.0 | 0.11 | 0.17 | 0.15 | 85.5 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## USER REPORT FOR SITE

Project: 200225sid-N186580 West Culburra Subdivision

## Site: 1 [11. Princes Hwy-Moss (Ex Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Princes Highway-Moss Street
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Signals - Fixed Time Coordinated Cycle Time $=130$ seconds (Site Optimum Cycle Time - Minimum Delay)
Variable Sequence Analysis applied. The results are given for the selected output sequence.
Timings based on settings in the Site Phasing \& Timing dialog
Phase Times determined by the program
Phase Sequence: GTA - RMS/Video survey phasing/timing

## Reference Phase: Phase A

Input Phase Sequence: A, D, E, F, F1*, F2*
Output Phase Sequence: A, D, E, F, F1*
(* Variable Phase)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman <br> Total veh/h | $\begin{gathered} \text { =lows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 3 | 0.0 | 0.872 | 70.2 | LOS E | 19.4 | 144.3 | 1.00 | 0.96 | 1.19 | 28.8 |
| 2 | T1 | 904 | 7.6 | 0.944 | 72.3 | LOS F | 23.3 | 173.9 | 1.00 | 1.02 | 1.30 | 29.4 |
| 3 | R2 | 158 | 3.7 | 0.946 | 92.2 | LOS F | 12.2 | 88.5 | 1.00 | 1.01 | 1.55 | 23.6 |
| Appr | ch | 1065 | 7.0 | 0.946 | 75.2 | LOS F | 23.3 | 173.9 | 1.00 | 1.02 | 1.33 | 28.3 |
| East: Moss St (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 34 | 3.4 | 0.373 | 49.2 | LOS D | 8.8 | 63.4 | 0.88 | 0.74 | 0.88 | 32.7 |
| 5 | T1 | 227 | 4.1 | 1.013 | 73.6 | LOS F | 33.7 | 245.1 | 0.93 | 0.96 | 1.22 | 24.6 |
| 6 | R2 | 257 | 4.6 | 1.013 | 117.5 | LOS F | 33.7 | 245.1 | 1.00 | 1.26 | 1.67 | 20.0 |
| Appr |  | 518 | 4.3 | 1.013 | 93.8 | LOS F | 33.7 | 245.1 | 0.96 | 1.09 | 1.42 | 22.5 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 187 | 5.0 | 1.010 | 82.9 | LOS F | 41.1 | 303.4 | 1.00 | 1.12 | 1.48 | 21.9 |
| 8 | T1 | 1426 | 6.8 | 1.010 | 92.0 | LOS F | 47.2 | 349.6 | 1.00 | 1.21 | 1.50 | 24.1 |
| 9 | R2 | 380 | 3.7 | 0.975 | 97.5 | LOS F | 32.3 | 233.3 | 1.00 | 1.05 | 1.49 | 22.8 |
| Appr |  | 1994 | 6.1 | 1.010 | 92.2 | LOS F | 47.2 | 349.6 | 1.00 | 1.17 | 1.49 | 23.6 |
| West: Moss St (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 126 | 10.3 | 0.179 | 18.1 | LOS B | 2.8 | 21.4 | 0.67 | 0.72 | 0.67 | 42.2 |
| 11 | T1 | 131 | 3.6 | 0.507 | 51.1 | LOS D | 10.3 | 76.0 | 0.95 | 0.79 | 0.95 | 29.3 |
| 12 | R2 | 49 | 11.9 | 0.507 | 55.8 | LOS D | 10.3 | 76.0 | 0.95 | 0.79 | 0.95 | 30.4 |
| Approach |  | 306 | 7.7 | 0.507 | 38.3 | LOS C | 10.3 | 76.0 | 0.83 | 0.76 | 0.83 | 33.7 |
| All V | icles | 3883 | 6.2 | 1.013 | 83.5 | LOS F | 47.2 | 349.6 | 0.98 | 1.09 | 1.39 | 25.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 1 [11. Princes Hwy-Moss (Ex Fri PM-120th HH)]
$13 S 1231000$ - West Culburra Subdivision
Princes Highway-Moss Street
Friday PM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Signals - Fixed Time Coordinated Cycle Time $=135$ seconds (Site Optimum Cycle Time - Minimum Delay)
Variable Sequence Analysis applied. The results are given for the selected output sequence.
Timings based on settings in the Site Phasing \& Timing dialog
Phase Times determined by the program
Phase Sequence: GTA - RMS/Video survey phasing/timing
Reference Phase: Phase A
Input Phase Sequence: A, D, E, F, F1*, F2*
Output Phase Sequence: A, D, E, F
(* Variable Phase)
Movement Performance - Vehicles

| Mov ID | Turn | Demand Flows |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Queue |  | Prop. Queued | Effective Stop Rate | Aver. No. Average Cycles Speed km/h |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Total veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ |  |  |  | Vehicles veh | Distance m |  |  |  |  |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 11 | 0.0 | 0.812 | 62.2 | LOS E | 22.7 | 162.9 | 0.98 | 0.90 | 1.05 | 30.7 |
| 2 | T1 | 1117 | 3.1 | 0.878 | 60.1 | LOS E | 26.5 | 190.4 | 0.99 | 0.94 | 1.12 | 32.6 |
| 3 | R2 | 115 | 0.0 | 0.321 | 57.2 | LOS E | 6.5 | 45.8 | 0.91 | 0.78 | 0.91 | 30.5 |
| Appr | ach | 1242 | 2.8 | 0.878 | 59.8 | LOS E | 26.5 | 190.4 | 0.99 | 0.93 | 1.10 | 32.3 |
| East: Moss St (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 34 | 4.3 | 0.353 | 55.7 | LOS D | 7.4 | 52.1 | 0.91 | 0.75 | 0.91 | 30.8 |
| 5 | T1 | 149 | 0.0 | 0.959 | 64.7 | LOS E | 23.7 | 167.9 | 0.94 | 0.88 | 1.12 | 26.2 |
| 6 | R2 | 232 | 1.9 | 0.959 | 92.6 | LOS F | 23.7 | 167.9 | 1.00 | 1.12 | 1.48 | 23.3 |
| Appr |  | 415 | 1.4 | 0.959 | 79.6 | LOS F | 23.7 | 167.9 | 0.97 | 1.00 | 1.30 | 24.8 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 199 | 3.7 | 1.149 | 189.1 | LOS F | 56.1 | 404.9 | 1.00 | 1.41 | 2.10 | 13.1 |
| 8 | T1 | 1319 | 3.5 | 1.149 | 197.6 | LOS F | 65.5 | 472.0 | 1.00 | 1.56 | 2.10 | 13.9 |
| 9 | R2 | 408 | 0.7 | 1.148 | 214.9 | LOS F | 54.1 | 380.8 | 1.00 | 1.31 | 2.18 | 12.8 |
| Appr | ach | 1926 | 2.9 | 1.149 | 200.4 | LOS F | 65.5 | 472.0 | 1.00 | 1.49 | 2.11 | 13.6 |
| West: Moss St (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 506 | 1.2 | 0.675 | 22.5 | LOS B | 14.9 | 105.0 | 0.87 | 0.83 | 0.87 | 41.4 |
| 11 | T1 | 245 | 0.0 | 1.164 | 222.9 | LOS F | 52.4 | 367.8 | 1.00 | 1.74 | 2.27 | 12.0 |
| 12 | R2 | 135 | 1.1 | 1.164 | 227.4 | LOS F | 52.4 | 367.8 | 1.00 | 1.74 | 2.27 | 12.4 |
| Appr |  | 886 | 0.8 | 1.164 | 109.1 | LOS F | 52.4 | 367.8 | 0.93 | 1.22 | 1.47 | 20.3 |
| All V | icles | 4469 | 2.3 | 1.164 | 132.0 | LOS F | 65.5 | 472.0 | 0.98 | 1.24 | 1.63 | 18.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 1 [11. Princes Hwy-Moss (Ex Sat-120th HH)]
13S1231000 - West Culburra Subdivision
Princes Highway-Moss Street
Saturday - Equivalent 120th HH
Site Category: (None)
Signals - Fixed Time Coordinated Cycle Time $=135$ seconds (Site Optimum Cycle Time - Minimum Delay)
Variable Sequence Analysis applied. The results are given for the selected output sequence.
Timings based on settings in the Site Phasing \& Timing dialog
Phase Times determined by the program
Phase Sequence: GTA - RMS/Video survey phasing/timing
Reference Phase: Phase A
Input Phase Sequence: A, D, E, F, F1*, F2*
Output Phase Sequence: A, D, E, F
(* Variable Phase)
Movement Performance - Vehicles

| Mov ID |  | Demand Flows |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Queue |  | Prop. Queued | Effective Stop Rate | Aver. No. Average <br> Cycles Speed km/h |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Total veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ |  |  |  | Vehicles veh | Distance m |  |  |  |  |
| South: Princes Hwy (S) 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 12 | 0.0 | 0.729 | 54.4 | LOS D | 21.3 | 152.5 | 0.92 | 0.80 | 0.93 | 32.8 |
| 2 | T1 | 1167 | 2.7 | 0.789 | 50.3 | LOS D | 24.4 | 175.0 | 0.95 | 0.84 | 0.98 | 35.7 |
| 3 | R2 | 104 | 0.0 | 0.361 | 62.1 | LOS E | 6.2 | 43.6 | 0.94 | 0.78 | 0.94 | 29.3 |
| Appr | ach | 1283 | 2.4 | 0.789 | 51.3 | LOS D | 24.4 | 175.0 | 0.95 | 0.84 | 0.97 | 35.0 |
| East: Moss St (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 32 | 0.0 | 0.170 | 49.3 | LOS D | 3.9 | 27.0 | 0.84 | 0.70 | 0.84 | 32.7 |
| 5 | T1 | 80 | 0.0 | 0.463 | 47.2 | LOS D | 10.7 | 75.5 | 0.88 | 0.74 | 0.88 | 29.8 |
| 6 | R2 | 146 | 1.8 | 0.463 | 54.5 | LOS D | 10.7 | 75.5 | 0.92 | 0.79 | 0.92 | 30.8 |
| Appr | ach | 258 | 1.0 | 0.463 | 51.6 | LOS D | 10.7 | 75.5 | 0.90 | 0.77 | 0.90 | 30.7 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 115 | 2.3 | 0.917 | 73.0 | LOS F | 33.8 | 240.3 | 1.00 | 1.05 | 1.46 | 27.8 |
| 8 | T1 | 1303 | 1.5 | 0.917 | 65.3 | LOS E | 34.2 | 242.7 | 1.00 | 1.03 | 1.27 | 31.0 |
| 9 | R2 | 258 | 1.0 | 0.899 | 80.8 | LOS F | 19.3 | 136.4 | 1.00 | 0.96 | 1.31 | 25.5 |
| Appr | ach | 1676 | 1.5 | 0.917 | 68.2 | LOS E | 34.2 | 242.7 | 1.00 | 1.02 | 1.29 | 29.8 |
| West: Moss St (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 311 | 0.0 | 0.491 | 24.0 | LOS B | 8.9 | 62.1 | 0.84 | 0.80 | 0.84 | 40.9 |
| 11 | T1 | 154 | 0.0 | 0.728 | 57.6 | LOS E | 16.4 | 115.3 | 1.00 | 0.87 | 1.04 | 27.7 |
| 12 | R2 | 103 | 1.3 | 0.728 | 62.1 | LOS E | 16.4 | 115.3 | 1.00 | 0.87 | 1.04 | 29.4 |
| Appr | ach | 567 | 0.2 | 0.728 | 40.0 | LOS C | 16.4 | 115.3 | 0.91 | 0.83 | 0.93 | 34.1 |
| All V | icles | 3784 | 1.6 | 0.917 | 57.1 | LOS E | 34.2 | 242.7 | 0.96 | 0.91 | 1.10 | 32.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.
$\nabla$ Site: 1 [1. Culburra -Coonamia (Future Fri AM-120th HH)]
13S1231000 - West Culburra Subdivision
Culburra Road-Coonamia Road
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{gathered} \text { Flows } \\ \text { HV } \\ \% \\ \hline \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Coonamia Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 165 | 0.0 | 0.199 | 9.9 | LOS A | 0.8 | 5.4 | 0.44 | 0.73 | 0.44 | 67.4 |
| 3 | R2 | 91 | 7.0 | 0.189 | 13.0 | LOS A | 0.7 | 5.5 | 0.55 | 0.82 | 0.55 | 61.5 |
| Appr | ach | 256 | 2.5 | 0.199 | 11.0 | LOS A | 0.8 | 5.5 | 0.48 | 0.76 | 0.48 | 65.2 |
| East: Culburra Rd (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 57 | 3.7 | 0.031 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 64.1 |
| 5 | T1 | 297 | 4.3 | 0.156 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| Approach |  | 354 | 4.2 | 0.156 | 1.1 | NA | 0.0 | 0.0 | 0.00 | 0.10 | 0.00 | 76.9 |
| West: Culburra Rd (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & 11 \\ & 12 \end{aligned}$ | T1 | 146 | 12.9 | 0.082 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 80.0 |
|  | R2 | 33 | 3.2 | 0.033 | 8.5 | LOS A | 0.1 | 0.9 | 0.42 | 0.65 | 0.42 | 66.2 |
| Approach |  | 179 | 11.2 | 0.082 | 1.5 | NA | 0.1 | 0.9 | 0.08 | 0.12 | 0.08 | 77.0 |
| All Vehicles |  | 788 | 5.2 | 0.199 | 4.4 | NA | 0.8 | 5.5 | 0.17 | 0.32 | 0.17 | 72.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [1. Culburra -Coonamia (Future Fri PM-120th HH)]

13S1231000 - West Culburra Subdivision
Culburra Road-Coonamia Road
Friday PM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{gathered} \text { lows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Coonamia Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 77 | 8.2 | 0.084 | 9.3 | LOS A | 0.3 | 2.3 | 0.33 | 0.66 | 0.33 | 65.4 |
| 3 | R2 | 94 | 2.2 | 0.186 | 12.5 | LOS A | 0.7 | 5.3 | 0.54 | 0.81 | 0.54 | 63.4 |
| Appr |  | 171 | 4.9 | 0.186 | 11.0 | LOS A | 0.7 | 5.3 | 0.45 | 0.74 | 0.45 | 64.3 |
| East: Culburra Rd (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 91 | 2.3 | 0.050 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 64.6 |
| 5 | T1 | 167 | 2.5 | 0.087 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 80.0 |
| Appro |  | 258 | 2.4 | 0.087 | 2.5 | NA | 0.0 | 0.0 | 0.00 | 0.22 | 0.00 | 73.8 |
| West: Culburra Rd (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 | T1 | 344 | 1.2 | 0.179 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| 12 | R2 | 152 | 4.2 | 0.139 | 8.1 | LOS A | 0.6 | 4.2 | 0.38 | 0.65 | 0.38 | 66.1 |
| Approach |  | 496 | 2.1 | 0.179 | 2.5 | NA | 0.6 | 4.2 | 0.11 | 0.20 | 0.11 | 75.1 |
| All Vehicles |  | 924 | 2.7 | 0.186 | 4.1 | NA | 0.7 | 5.3 | 0.14 | 0.31 | 0.14 | 72.5 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [1. Culburra -Coonamia (Future SAT-120th HH)]

13S1231000 - West Culburra Subdivision
Culburra Road-Coonamia Road
Saturday - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Coonamia Rd |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 104 | 1.0 | 0.117 | 9.4 | LOS A | 0.4 | 3.1 | 0.38 | 0.68 | 0.38 | 67.7 |
| 3 | R2 | 86 | 0.0 | 0.172 | 12.4 | LOS A | 0.7 | 4.7 | 0.54 | 0.81 | 0.54 | 64.1 |
| Appr |  | 191 | 0.6 | 0.172 | 10.8 | LOS A | 0.7 | 4.7 | 0.45 | 0.74 | 0.45 | 66.0 |
| East: Culburra Rd (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 104 | 0.0 | 0.056 | 6.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 65.4 |
| 5 | T1 | 220 | 1.0 | 0.114 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 80.0 |
| Appro |  | 324 | 0.6 | 0.114 | 2.2 | NA | 0.0 | 0.0 | 0.00 | 0.20 | 0.00 | 74.6 |
| West: Culburra Rd (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 | T1 | 219 | 1.0 | 0.114 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 80.0 |
| 12 | R2 | 104 | 3.0 | 0.102 | 8.4 | LOS A | 0.4 | 2.9 | 0.41 | 0.67 | 0.41 | 66.3 |
| Approach |  | 323 | 1.6 | 0.114 | 2.7 | NA | 0.4 | 2.9 | 0.13 | 0.22 | 0.13 | 75.0 |
| All Vehicles |  | 838 | 1.0 | 0.172 | 4.4 | NA | 0.7 | 4.7 | 0.15 | 0.33 | 0.15 | 72.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab)
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [2. Culburra-Mayfield (Future Fri AM-120th HH)]

Culburra Road-Mayfield Road
Friday AM (0800-0900) - Equivalent 120th HH
Future - Full Site Development
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | $\begin{gathered} \text { Demanc } \\ \text { Total } \\ \text { veh/h } \end{gathered}$ | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles $\qquad$ | of Queue Distance m $\qquad$ | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Culburra Road (S) |  |  |  |  |  |  |  |  |  |  |  |
| L2 | 2 | 0.0 | 0.234 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 74.5 |
| 2 T1 | 455 | 0.0 | 0.234 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| Approach | 457 | 0.0 | 0.234 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.8 |
| North: Culburra Road (N) |  |  |  |  |  |  |  |  |  |  |  |
| 8 T1 | 176 | 0.0 | 0.091 | 0.0 | LOS A | 0.0 | 0.1 | 0.01 | 0.00 | 0.01 | 79.9 |
| 9 R2 | 1 | 0.0 | 0.091 | 9.2 | LOS A | 0.0 | 0.1 | 0.01 | 0.00 | 0.01 | 66.1 |
| Approach | 177 | 0.0 | 0.091 | 0.1 | NA | 0.0 | 0.1 | 0.01 | 0.00 | 0.01 | 79.8 |
| West: Mayfield Road |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 1 | 0.0 | 0.002 | 8.1 | LOS A | 0.0 | 0.0 | 0.47 | 0.58 | 0.47 | 56.4 |
| 12 R2 | 2 | 0.0 | 0.007 | 15.0 | LOS B | 0.0 | 0.2 | 0.67 | 0.75 | 0.67 | 50.7 |
| Approach | 3 | 0.0 | 0.007 | 12.7 | LOS A | 0.0 | 0.2 | 0.60 | 0.69 | 0.60 | 52.5 |
| All Vehicles | 637 | 0.0 | 0.234 | 0.1 | NA | 0.0 | 0.2 | 0.01 | 0.01 | 0.01 | 79.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [2. Culburra-Mayfield (Future Fri PM-120th HH)]

Culburra Road-Mayfield Road
Friday PM (1600-1700) - Equivalent 120th HH
Future - Full Site Development
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | Flows HV $\%$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \mathrm{v} / \mathrm{c} \\ & \hline \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back Vehicles $\qquad$ | of Queue Distance $\qquad$ m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Culburra Road (S) |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 1 | 0.0 | 0.131 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 74.6 |
| 2 | T1 | 246 | 4.7 | 0.131 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| Appr |  | 247 | 4.7 | 0.131 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| North: Culburra Road (N) |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 | T1 | 474 | 2.0 | 0.247 | 0.0 | LOS A | 0.0 | 0.1 | 0.00 | 0.00 | 0.00 | 80.0 |
| 9 | R2 | 1 | 0.0 | 0.247 | 8.2 | LOS A | 0.0 | 0.1 | 0.00 | 0.00 | 0.00 | 66.2 |
| Appr |  | 475 | 2.0 | 0.247 | 0.0 | NA | 0.0 | 0.1 | 0.00 | 0.00 | 0.00 | 79.9 |
| West: Mayfield Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 1 | 0.0 | 0.001 | 6.7 | LOS A | 0.0 | 0.0 | 0.34 | 0.53 | 0.34 | 57.5 |
| 12 | R2 | 1 | 100.0 | 0.013 | 48.4 | LOS D | 0.0 | 0.5 | 0.87 | 0.95 | 0.87 | 30.3 |
| Approach |  | 2 | 50.0 | 0.013 | 27.5 | LOS B | 0.0 | 0.5 | 0.60 | 0.74 | 0.60 | 39.7 |
| All Vehicles |  | 724 | 3.1 | 0.247 | 0.1 | NA | 0.0 | 0.5 | 0.00 | 0.00 | 0.00 | 79.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [2. Culburra-Mayfield (Future-120th HH)]

Culburra Road-Mayfield Road
Saturday - Equivalent 120th HH
Future - Full Site Development
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \mathrm{v} / \mathrm{c} \\ & \hline \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back Vehicles $\qquad$ | of Queue Distance $\qquad$ m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Culburra Road (S) |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 3 | 0.0 | 0.170 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 74.5 |
| 2 | T1 | 327 | 0.6 | 0.170 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 79.8 |
| Appr |  | 331 | 0.6 | 0.170 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 79.8 |
| North: Culburra Road ( N ) |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 | T1 | 302 | 0.7 | 0.157 | 0.0 | LOS A | 0.0 | 0.1 | 0.00 | 0.00 | 0.00 | 79.9 |
| 9 | R2 | 1 | 0.0 | 0.157 | 8.5 | LOS A | 0.0 | 0.1 | 0.00 | 0.00 | 0.00 | 66.1 |
| Appro | ch | 303 | 0.7 | 0.157 | 0.0 | NA | 0.0 | 0.1 | 0.00 | 0.00 | 0.00 | 79.9 |
| West: Mayfield Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 1 | 0.0 | 0.001 | 7.2 | LOS A | 0.0 | 0.0 | 0.39 | 0.55 | 0.39 | 57.3 |
| 12 | R2 | 3 | 0.0 | 0.010 | 14.6 | LOS B | 0.0 | 0.2 | 0.66 | 0.76 | 0.66 | 51.0 |
| Approach |  | 4 | 0.0 | 0.010 | 12.7 | LOS A | 0.0 | 0.2 | 0.59 | 0.70 | 0.59 | 52.4 |
| All Vehicles |  | 638 | 0.7 | 0.170 | 0.1 | NA | 0.0 | 0.2 | 0.01 | 0.01 | 0.01 | 79.5 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [3. Greenwell Pt-Pyree (Future Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Pyree Lane
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{array}{r} \text { Flows } \\ \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles $\qquad$ | of Queue Distance $\qquad$ m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Pyreen Ln |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 447 | 2.1 | 0.245 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 65.5 |
| 3 | R2 | 14 | 7.7 | 0.027 | 10.6 | LOSA | 0.1 | 0.7 | 0.48 | 0.70 | 0.48 | 58.7 |
| Appr |  | 461 | 2.3 | 0.245 | 7.7 | LOS A | 0.1 | 0.7 | 0.01 | 0.60 | 0.01 | 65.3 |
| East: Greenwell Pt Rd (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 21 | 65.0 | 0.095 | 8.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.08 | 0.00 | 53.3 |
| 5 | T1 | 147 | 5.7 | 0.095 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.08 | 0.00 | 79.4 |
| Appr |  | 168 | 13.1 | 0.095 | 1.0 | NA | 0.0 | 0.0 | 0.00 | 0.08 | 0.00 | 74.8 |
| West: Greenwell Pt Rd (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 | T1 | 63 | 5.0 | 0.033 | 6.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 0.00 | 65.1 |
| 12 | R2 | 161 | 7.8 | 0.362 | 14.8 | LOS B | 1.9 | 13.8 | 0.63 | 0.91 | 0.79 | 55.4 |
| Approach |  | 224 | 7.0 | 0.362 | 12.4 | LOS A | 1.9 | 13.8 | 0.45 | 0.82 | 0.57 | 57.8 |
| All Vehicles |  | 854 | 5.7 | 0.362 | 7.6 | NA | 1.9 | 13.8 | 0.13 | 0.56 | 0.16 | 64.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [3. Greenwell Pt-Pyree (Future Fri PM-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Pyree Lane
Friday PM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | Demand <br> Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Pyreen Ln |  |  |  |  |  |  |  |  |  |  |  |
| L2 | 207 | 5.6 | 0.116 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 64.5 |
| 3 R2 | 32 | 0.0 | 0.089 | 14.7 | LOS B | 0.3 | 2.2 | 0.63 | 0.87 | 0.63 | 57.0 |
| Approach | 239 | 4.8 | 0.116 | 8.6 | LOS A | 0.3 | 2.2 | 0.08 | 0.64 | 0.08 | 63.4 |
| East: Greenwell Pt Rd (E) |  |  |  |  |  |  |  |  |  |  |  |
| L2 | 27 | 0.0 | 0.051 | 6.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.19 | 0.00 | 71.6 |
| $5 \quad$ T1 | 68 | 4.6 | 0.051 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.19 | 0.00 | 76.5 |
| Approach | 96 | 3.3 | 0.051 | 2.0 | NA | 0.0 | 0.0 | 0.00 | 0.19 | 0.00 | 75.1 |
| West: Greenwell Pt Rd (W) |  |  |  |  |  |  |  |  |  |  |  |
| 11 T1 | 173 | 1.8 | 0.090 | 6.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 0.00 | 66.1 |
| 12 R2 | 453 | 1.6 | 0.687 | 14.8 | LOS B | 8.7 | 62.0 | 0.70 | 0.93 | 1.13 | 56.9 |
| Approach | 625 | 1.7 | 0.687 | 12.4 | LOS A | 8.7 | 62.0 | 0.51 | 0.84 | 0.82 | 59.2 |
| All Vehicles | 960 | 2.6 | 0.687 | 10.4 | NA | 8.7 | 62.0 | 0.35 | 0.72 | 0.56 | 61.5 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [3. Greenwell Pt-Pyree (Future Sat-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Pyree Lane
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{array}{r} \text { lows } \\ \text { HV } \\ \% \\ \hline \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles $\qquad$ | of Queue Distance $\qquad$ m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Pyreen Ln |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 268 | 1.2 | 0.146 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 65.8 |
| 3 | R2 | 54 | 0.0 | 0.099 | 10.4 | LOS A | 0.4 | 2.6 | 0.49 | 0.74 | 0.49 | 61.1 |
| Appr |  | 322 | 1.0 | 0.146 | 8.1 | LOS A | 0.4 | 2.6 | 0.08 | 0.63 | 0.08 | 65.0 |
| East: Greenwell Pt Rd (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 165 | 2.5 | 0.143 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.40 | 0.00 | 67.6 |
| 5 | T1 | 100 | 4.2 | 0.143 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.40 | 0.00 | 73.0 |
| Appr |  | 265 | 3.2 | 0.143 | 4.4 | NA | 0.0 | 0.0 | 0.00 | 0.40 | 0.00 | 69.5 |
| West: Greenwell Pt Rd (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 11 | T1 | 227 | 0.9 | 0.117 | 6.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 0.00 | 66.4 |
| 12 | R2 | 143 | 1.5 | 0.307 | 13.6 | LOS A | 1.4 | 10.1 | 0.61 | 0.88 | 0.70 | 58.1 |
| Approach |  | 371 | 1.1 | 0.307 | 9.0 | LOS A | 1.4 | 10.1 | 0.23 | 0.70 | 0.27 | 62.9 |
| All Vehicles |  | 958 | 1.6 | 0.307 | 7.4 | NA | 1.4 | 10.1 | 0.12 | 0.59 | 0.13 | 65.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.
$\nabla$ Site: 1 [4. Greenwell Pt-Jindy Andy (Future Fri AM-120th HH)]
13S1231000 - West Culburra Subdivision
Greenwell Point Road-Jindy Andy Lane
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \mathrm{Mov} \\ & \mathrm{ID} \end{aligned}$ |  | Demand <br> Total veh/h | $\begin{gathered} \text { Flows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| NorthEast: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |  |  |
| 25 | T1 | 420 | 5.0 | 0.224 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| 26 | R2 | 181 | 0.6 | 0.270 | 9.3 | LOS A | 1.2 | 8.5 | 0.44 | 0.70 | 0.44 | 62.4 |
| Appr |  | 601 | 3.7 | 0.270 | 2.8 | NA | 1.2 | 8.5 | 0.13 | 0.21 | 0.13 | 73.7 |
| NorthWest: Jindy Andy Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 12 | 9.1 | 0.012 | 8.0 | LOS A | 0.0 | 0.3 | 0.29 | 0.60 | 0.29 | 61.2 |
| 29 | R2 | 53 | 6.0 | 0.247 | 24.6 | LOS B | 0.9 | 6.7 | 0.80 | 0.95 | 0.89 | 48.2 |
| Approach |  | 64 | 6.6 | 0.247 | 21.6 | LOS B | 0.9 | 6.7 | 0.71 | 0.89 | 0.79 | 50.1 |
| SouthWest: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & 30 \\ & 31 \\ & \hline \end{aligned}$ | L2 | 8 | 37.5 | 0.100 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.03 | 0.00 | 60.6 |
|  | T1 | 174 | 9.1 | 0.100 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.03 | 0.00 | 79.6 |
| Approach |  | 182 | 10.4 | 0.100 | 0.4 | NA | 0.0 | 0.0 | 0.00 | 0.03 | 0.00 | 78.4 |
| All Vehicles |  | 847 | 5.3 | 0.270 | 3.7 | NA | 1.2 | 8.5 | 0.15 | 0.22 | 0.15 | 72.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.
$\nabla$ Site: 1 [4. Greenwell Pt-Jindy Andy (Future Fri PM-120th HH)]
13S1231000 - West Culburra Subdivision
Greenwell Point Road-Jindy Andy Lane
Friday AM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | Turn | Demand Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| NorthEast: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |  |  |
| 25 | T1 | 213 | 5.9 | 0.114 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 80.0 |
| 26 | R2 | 77 | 2.7 | 0.197 | 14.2 | LOS A | 0.7 | 5.3 | 0.61 | 0.86 | 0.61 | 56.9 |
| Appr |  | 289 | 5.1 | 0.197 | 3.8 | NA | 0.7 | 5.3 | 0.16 | 0.23 | 0.16 | 72.2 |
| NorthWest: Jindy Andy Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 178 | 1.8 | 0.257 | 10.5 | LOS A | 1.0 | 7.3 | 0.54 | 0.82 | 0.56 | 61.0 |
| 29 | R2 | 3 | 0.0 | 0.013 | 19.1 | LOS B | 0.0 | 0.3 | 0.73 | 0.83 | 0.73 | 53.3 |
| Approach |  | 181 | 1.7 | 0.257 | 10.6 | LOS A | 1.0 | 7.3 | 0.54 | 0.82 | 0.56 | 60.9 |
| SouthWest: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 14 | 7.7 | 0.238 | 7.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 71.0 |
| 31 | T1 | 444 | 1.9 | 0.238 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 79.6 |
| Appr |  | 458 | 2.1 | 0.238 | 0.2 | NA | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 79.3 |
| All V | icles | 928 | 2.9 | 0.257 | 3.4 | NA | 1.0 | 7.3 | 0.16 | 0.24 | 0.16 | 72.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.
$\nabla$ Site: 1 [4. Greenwell Pt-Jindy Andy (Future Sat-120th HH)]
$13 S 1231000$ - West Culburra Subdivision
Greenwell Point Road-Jindy Andy Lane
Saturday - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | Turn | Deman <br> Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| NorthEast: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |  |  |
| 25 | T1 | 284 | 3.0 | 0.150 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| 26 | R2 | 108 | 1.0 | 0.204 | 11.1 | LOS A | 0.8 | 5.8 | 0.53 | 0.78 | 0.53 | 60.4 |
| Appro |  | 393 | 2.4 | 0.204 | 3.1 | NA | 0.8 | 5.8 | 0.15 | 0.22 | 0.15 | 73.4 |
| NorthWest: Jindy Andy Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| 27 | L2 | 112 | 0.0 | 0.132 | 8.8 | LOS A | 0.5 | 3.4 | 0.42 | 0.69 | 0.42 | 63.3 |
| 29 | R2 | 9 | 0.0 | 0.034 | 17.6 | LOS B | 0.1 | 0.8 | 0.70 | 0.87 | 0.70 | 54.5 |
| Appr |  | 121 | 0.0 | 0.132 | 9.5 | LOS A | 0.5 | 3.4 | 0.44 | 0.71 | 0.44 | 62.5 |
| SouthWest: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |  |  |
| 30 | L2 | 5 | 0.0 | 0.165 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 74.4 |
| 31 | T1 | 309 | 3.1 | 0.165 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 79.7 |
| Approach |  | 315 | 3.0 | 0.165 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 79.6 |
| All Vehicles |  | 828 | 2.3 | 0.204 | 2.9 | NA | 0.8 | 5.8 | 0.13 | 0.21 | 0.13 | 73.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [5. Greenwell Pt-Mayfield (Future Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Mayfield Road
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman <br> Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Mayfield Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 12 | 0.0 | 0.020 | 8.1 | LOS A | 0.1 | 0.5 | 0.48 | 0.66 | 0.48 | 55.9 |
| 23 | R2 | 1 | 0.0 | 0.020 | 15.2 | LOS B | 0.1 | 0.5 | 0.48 | 0.66 | 0.48 | 55.6 |
| Appr |  | 13 | 0.0 | 0.020 | 8.7 | LOS A | 0.1 | 0.5 | 0.48 | 0.66 | 0.48 | 55.9 |
| NorthEast: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 3 | 0.0 | 0.226 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 74.5 |
| 25 | T1 | 427 | 3.9 | 0.226 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.8 |
| Approach |  | 431 | 3.9 | 0.226 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.8 |
| SouthWest: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & 31 \\ & 32 \\ & \hline \end{aligned}$ | T1 | 178 | 9.5 | 0.112 | 0.7 | LOS A | 0.2 | 1.7 | 0.08 | 0.02 | 0.08 | 78.0 |
|  | R2 | 6 | 0.0 | 0.112 | 13.3 | LOS A | 0.2 | 1.7 | 0.08 | 0.02 | 0.08 | 64.4 |
| Approach |  | 184 | 9.1 | 0.112 | 1.1 | NA | 0.2 | 1.7 | 0.08 | 0.02 | 0.08 | 77.5 |
| All Vehicles |  | 627 | 5.4 | 0.226 | 0.6 | NA | 0.2 | 1.7 | 0.03 | 0.02 | 0.03 | 78.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [5. Greenwell Pt-Mayfield (Future Fri PM-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Mayfield Road
Friday PM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman <br> Total veh/h | $\begin{aligned} & \text { =lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average <br> Speed km/h |
| SouthEast: Mayfield Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 6 | 0.0 | 0.036 | 6.6 | LOS A | 0.1 | 0.9 | 0.54 | 0.69 | 0.54 | 52.0 |
| 23 | R2 | 6 | 16.7 | 0.036 | 20.8 | LOS B | 0.1 | 0.9 | 0.54 | 0.69 | 0.54 | 48.2 |
| Appro |  | 13 | 8.3 | 0.036 | 13.7 | LOS A | 0.1 | 0.9 | 0.54 | 0.69 | 0.54 | 50.0 |
| NorthEast: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 1 | 0.0 | 0.117 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 74.6 |
| 25 | T1 | 219 | 5.8 | 0.117 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| Appro |  | 220 | 5.7 | 0.117 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 79.9 |
| SouthWest: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |  |  |
| 31 | T1 | 458 | 1.6 | 0.261 | 0.2 | LOS A | 0.4 | 2.5 | 0.05 | 0.02 | 0.05 | 79.1 |
| 32 | R2 | 15 | 0.0 | 0.261 | 9.8 | LOS A | 0.4 | 2.5 | 0.05 | 0.02 | 0.05 | 65.1 |
| Approach |  | 473 | 1.6 | 0.261 | 0.5 | NA | 0.4 | 2.5 | 0.05 | 0.02 | 0.05 | 78.5 |
| All Vehicles |  | 705 | 3.0 | 0.261 | 0.6 | NA | 0.4 | 2.5 | 0.04 | 0.03 | 0.04 | 78.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [5. Greenwell Pt-Mayfield (Future Sat-120th HH) ]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Mayfield Road
Saturday - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ | Turn | Deman <br> Total veh/h | $\begin{gathered} \text { Fows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| SouthEast: Mayfield Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 21 | L2 | 8 | 0.0 | 0.013 | 7.0 | LOS A | 0.0 | 0.3 | 0.41 | 0.60 | 0.41 | 56.6 |
| 23 | R2 | 1 | 0.0 | 0.013 | 14.5 | LOS A | 0.0 | 0.3 | 0.41 | 0.60 | 0.41 | 56.3 |
| Appro |  | 9 | 0.0 | 0.013 | 7.8 | LOS A | 0.0 | 0.3 | 0.41 | 0.60 | 0.41 | 56.6 |
| NorthEast: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |  |  |
| 24 | L2 | 4 | 75.0 | 0.155 | 8.3 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 51.3 |
| 25 | T1 | 293 | 1.1 | 0.155 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 79.9 |
| Approach |  | 297 | 2.1 | 0.155 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 79.3 |
| SouthWest: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & 31 \\ & 32 \\ & \hline \end{aligned}$ | T1 | 311 | 1.7 | 0.182 | 0.4 | LOS A | 0.3 | 2.2 | 0.07 | 0.02 | 0.07 | 78.7 |
|  | R2 | 12 | 0.0 | 0.182 | 10.8 | LOS A | 0.3 | 2.2 | 0.07 | 0.02 | 0.07 | 64.8 |
| Approach |  | 322 | 1.6 | 0.182 | 0.8 | NA | 0.3 | 2.2 | 0.07 | 0.02 | 0.07 | 78.1 |
| All Vehicles |  | 628 | 1.8 | 0.182 | 0.6 | NA | 0.3 | 2.2 | 0.04 | 0.03 | 0.04 | 78.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## Site: 1 [6. Greenwell Pt-Millbank-Worrigee (Future Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Millbank Road-Worrigee Road
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | Demand Total veh/h |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Worrigee Road |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 69 | 4.5 | 0.097 | 10.9 | LOS A | 0.3 | 2.5 | 0.47 | 0.94 | 0.47 | 54.0 |
| 2 T1 | 118 | 2.7 | 0.486 | 25.1 | LOS B | 2.5 | 17.7 | 0.83 | 1.12 | 1.23 | 44.9 |
| 3 R 2 | 23 | 0.0 | 0.486 | 27.5 | LOS B | 2.5 | 17.7 | 0.83 | 1.12 | 1.23 | 45.3 |
| Approach | 211 | 3.0 | 0.486 | 20.7 | LOS B | 2.5 | 17.7 | 0.71 | 1.06 | 0.98 | 47.6 |
| East: Greenwell Point Road (E) |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 14 | 0.0 | 0.218 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 74.2 |
| $5 \quad$ T1 | 402 | 3.1 | 0.218 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 79.5 |
| 6 R2 | 34 | 3.1 | 0.030 | 7.8 | LOS A | 0.1 | 0.8 | 0.33 | 0.61 | 0.33 | 62.8 |
| Approach | 449 | 3.0 | 0.218 | 0.8 | NA | 0.1 | 0.8 | 0.02 | 0.07 | 0.02 | 77.8 |
| North: Millbank Road |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 16 | 20.0 | 0.018 | 11.0 | LOS A | 0.1 | 0.5 | 0.32 | 0.88 | 0.32 | 56.9 |
| 8 T1 | 26 | 12.0 | 0.159 | 21.4 | LOS B | 0.6 | 4.2 | 0.77 | 1.00 | 0.77 | 47.3 |
| 9 R2 | 12 | 0.0 | 0.159 | 28.4 | LOS B | 0.6 | 4.2 | 0.77 | 1.00 | 0.77 | 51.7 |
| Approach | 54 | 11.8 | 0.159 | 19.9 | LOS B | 0.6 | 4.2 | 0.64 | 0.97 | 0.64 | 50.7 |
| West: Greenwell Point Road (W) |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 47 | 6.7 | 0.124 | 7.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.14 | 0.00 | 69.7 |
| 11 T1 | 178 | 8.9 | 0.124 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.14 | 0.00 | 77.4 |
| 12 R 2 | 61 | 5.2 | 0.069 | 9.1 | LOS A | 0.3 | 1.9 | 0.47 | 0.71 | 0.47 | 56.7 |
| Approach | 285 | 7.8 | 0.124 | 3.1 | NA | 0.3 | 1.9 | 0.10 | 0.26 | 0.10 | 70.6 |
| All Vehicles | 999 | 4.8 | 0.486 | 6.7 | NA | 2.5 | 17.7 | 0.22 | 0.38 | 0.28 | 65.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 1 [6. Greenwell Pt-Millbank-Worrigee (Future Fri PM-120th HH)]
13S1231000 - West Culburra Subdivision
Greenwell Point Road-Millbank Road-Worrigee Road
Friday PM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Demand <br> Total veh/h |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Worrigee Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 58 | 0.0 | 0.060 | 9.1 | LOS A | 0.2 | 1.5 | 0.32 | 0.88 | 0.32 | 56.1 |
| 2 | T1 | 47 | 0.0 | 0.337 | 24.0 | LOS B | 1.4 | 9.7 | 0.83 | 1.05 | 1.02 | 44.6 |
| 3 | R2 | 29 | 3.6 | 0.337 | 31.3 | LOS C | 1.4 | 9.7 | 0.83 | 1.05 | 1.02 | 44.0 |
| Appr |  | 135 | 0.8 | 0.337 | 19.2 | LOS B | 1.4 | 9.7 | 0.61 | 0.98 | 0.72 | 48.8 |
| East: Greenwell Point Road (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 22 | 9.5 | 0.117 | 7.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.07 | 0.00 | 69.7 |
| 5 | T1 | 197 | 5.3 | 0.117 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.07 | 0.00 | 78.8 |
| 6 | R2 | 16 | 0.0 | 0.018 | 9.0 | LOS A | 0.1 | 0.5 | 0.47 | 0.67 | 0.47 | 62.8 |
| Appr |  | 235 | 5.4 | 0.117 | 1.3 | NA | 0.1 | 0.5 | 0.03 | 0.11 | 0.03 | 76.5 |
| North: Millbank Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 24 | 0.0 | 0.035 | 11.6 | LOS A | 0.1 | 0.8 | 0.47 | 0.90 | 0.47 | 61.4 |
| 8 | T1 | 54 | 2.0 | 0.303 | 24.6 | LOS B | 1.2 | 8.6 | 0.82 | 1.03 | 0.97 | 45.8 |
| 9 | R2 | 18 | 0.0 | 0.303 | 31.4 | LOS C | 1.2 | 8.6 | 0.82 | 1.03 | 0.97 | 49.6 |
| Approach |  | 96 | 1.1 | 0.303 | 22.6 | LOS B | 1.2 | 8.6 | 0.73 | 1.00 | 0.84 | 49.7 |
| West: Greenwell Point Road (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 17 | 0.0 | 0.243 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 74.2 |
| 11 | T1 | 451 | 0.7 | 0.243 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 79.5 |
| 12 | R2 | 115 | 0.9 | 0.100 | 7.8 | LOS A | 0.4 | 2.9 | 0.34 | 0.63 | 0.34 | 57.7 |
| Appr |  | 582 | 0.7 | 0.243 | 1.8 | NA | 0.4 | 2.9 | 0.07 | 0.14 | 0.07 | 73.8 |
| All V | icles | 1047 | 1.8 | 0.337 | 5.8 | NA | 1.4 | 9.7 | 0.19 | 0.32 | 0.21 | 67.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## Site: 1 [6. Greenwell Pt-Millbank-Worrigee (Future Sat-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Millbank Road-Worrigee Road
Saturday - Equivalent 120th HH
Site Category: (None)
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand Total veh/h |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average <br> Speed <br> km/h |
| South: Worrigee Road |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 77 | 0.0 | 0.087 | 9.5 | LOS A | 0.3 | 2.2 | 0.37 | 0.89 | 0.37 | 55.9 |
| 2 T1 | 29 | 3.6 | 0.215 | 17.1 | LOS B | 0.8 | 5.9 | 0.71 | 1.01 | 0.74 | 48.9 |
| 3 R 2 | 40 | 2.6 | 0.215 | 19.3 | LOS B | 0.8 | 5.9 | 0.71 | 1.01 | 0.74 | 49.0 |
| Approach | 146 | 1.4 | 0.215 | 13.7 | LOS A | 0.8 | 5.9 | 0.53 | 0.95 | 0.55 | 52.4 |
| East: Greenwell Point Road (E) |  |  |  |  |  |  |  |  |  |  |  |
| 4 L 2 | 21 | 5.0 | 0.149 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.05 | 0.00 | 71.7 |
| $5 \quad \mathrm{~T} 1$ | 266 | 1.2 | 0.149 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.05 | 0.00 | 79.1 |
| 6 R2 | 19 | 0.0 | 0.018 | 8.0 | LOS A | 0.1 | 0.5 | 0.37 | 0.62 | 0.37 | 63.7 |
| Approach | 306 | 1.4 | 0.149 | 1.0 | NA | 0.1 | 0.5 | 0.02 | 0.08 | 0.02 | 77.4 |
| North: Millbank Road |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 11 | 20.0 | 0.014 | 11.6 | LOS A | 0.0 | 0.4 | 0.38 | 0.87 | 0.38 | 56.6 |
| 8 T1 | 24 | 13.0 | 0.131 | 18.8 | LOS B | 0.5 | 3.6 | 0.70 | 1.01 | 0.70 | 49.7 |
| 9 R2 | 16 | 0.0 | 0.131 | 20.9 | LOS B | 0.5 | 3.6 | 0.70 | 1.01 | 0.70 | 54.6 |
| Approach | 51 | 10.4 | 0.131 | 17.9 | LOS B | 0.5 | 3.6 | 0.64 | 0.98 | 0.64 | 52.5 |
| West: Greenwell Point Road (W) |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 21 | 5.0 | 0.152 | 7.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.05 | 0.00 | 71.7 |
| 11 T1 | 269 | 1.2 | 0.152 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.05 | 0.00 | 79.1 |
| 12 R 2 | 54 | 0.0 | 0.050 | 8.0 | LOS A | 0.2 | 1.3 | 0.37 | 0.64 | 0.37 | 57.6 |
| Approach | 344 | 1.2 | 0.152 | 1.7 | NA | 0.2 | 1.3 | 0.06 | 0.14 | 0.06 | 74.3 |
| All Vehicles | 847 | 1.9 | 0.215 | 4.5 | NA | 0.8 | 5.9 | 0.16 | 0.31 | 0.16 | 68.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 1 [7. Princes Hwy-Kalandar (Future Fri AM-120th HH)]
13 1231000 - West Culburra Subdivision
Princes Highway-Kalandar Street
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Signals - Fixed Time Coordinated Cycle Time $=135$ seconds (Site Optimum Cycle Time - Minimum Delay)
Variable Sequence Analysis applied. The results are given for the selected output sequence.
Timings based on settings in the Site Phasing \& Timing dialog
Phase Times determined by the program
Phase Sequence: GTA - RMS/Video survey phasing
Reference Phase: Phase A
Input Phase Sequence: A, D, E, F, F1*, F2*
Output Phase Sequence: A, D, E, F
(* Variable Phase)
Movement Performance - Vehicles

| Mov ID | Turn | Demand Flows |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Queue |  | Prop. Queued | Effective Stop Rate | Aver. No. Average Cycles Speed km/h |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Total veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ |  |  |  | Vehicles veh | Distance |  |  |  |  |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 3 | 0.0 | 0.983 | 102.5 | LOS F | 42.4 | 310.7 | 1.00 | 1.16 | 1.79 | 21.2 |
| 2 | T1 | 962 | 5.6 | 0.983 | 91.5 | LOS F | 42.4 | 310.7 | 1.00 | 1.16 | 1.59 | 25.4 |
| 3 | R2 | 41 | 0.0 | 0.230 | 68.7 | LOS E | 2.6 | 18.0 | 0.96 | 0.74 | 0.96 | 25.0 |
| Appr |  | 1006 | 5.3 | 0.983 | 90.6 | LOS F | 42.4 | 310.7 | 1.00 | 1.14 | 1.57 | 25.4 |
| East: Kalandar St (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 41 | 10.3 | 1.059 | 126.6 | LOS F | 55.6 | 400.9 | 1.00 | 1.26 | 1.74 | 14.3 |
| 5 | T1 | 318 | 3.3 | 1.059 | 121.0 | LOS F | 55.6 | 400.9 | 1.00 | 1.26 | 1.74 | 12.0 |
| 6 | R2 | 809 | 2.9 | 1.059 | 138.8 | LOS F | 63.7 | 456.8 | 1.00 | 1.23 | 1.75 | 14.7 |
| Appr |  | 1168 | 3.2 | 1.059 | 133.5 | LOS F | 63.7 | 456.8 | 1.00 | 1.24 | 1.75 | 14.0 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 279 | 7.2 | 0.202 | 6.9 | LOS A | 0.4 | 2.7 | 0.03 | 0.59 | 0.03 | 56.0 |
| 8 | T1 | 898 | 7.3 | 1.043 | 123.2 | LOS F | 47.8 | 355.8 | 1.00 | 1.31 | 1.65 | 20.5 |
| 9 | R2 | 157 | 11.4 | 0.963 | 100.8 | LOS F | 13.1 | 100.5 | 1.00 | 1.02 | 1.59 | 20.3 |
| Appr |  | 1334 | 7.7 | 1.043 | 96.3 | LOS F | 47.8 | 355.8 | 0.80 | 1.13 | 1.30 | 23.1 |
| West: Kalandar St (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 52 | 14.3 | 0.701 | 65.3 | LOS E | 13.4 | 97.7 | 1.00 | 0.85 | 1.04 | 27.9 |
| 11 | T1 | 158 | 1.3 | 0.701 | 59.3 | LOS E | 13.4 | 97.7 | 1.00 | 0.85 | 1.04 | 23.1 |
| 12 | R2 | 156 | 5.4 | 0.560 | 63.5 | LOS E | 9.7 | 70.9 | 0.98 | 0.81 | 0.98 | 26.9 |
| Appr |  | 365 | 4.9 | 0.701 | 62.0 | LOS E | 13.4 | 97.7 | 0.99 | 0.83 | 1.01 | 25.5 |
| All V | icles | 3874 | 5.5 | 1.059 | 102.8 | LOS F | 63.7 | 456.8 | 0.93 | 1.14 | 1.48 | 20.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 1 [7. Princes Hwy-Kalandar (Future Fri PM-120th HH)]
13S1231000 - West Culburra Subdivision
Princes Highway-Kalandar Street
Friday PM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Signals - Fixed Time Coordinated Cycle Time $=135$ seconds (Site Optimum Cycle Time - Minimum Delay)
Variable Sequence Analysis applied. The results are given for the selected output sequence.
Timings based on settings in the Site Phasing \& Timing dialog
Phase Times determined by the program
Phase Sequence: GTA - RMS/Video survey phasing
Reference Phase: Phase A
Input Phase Sequence: A, D, E, F, F1*, F2*
Output Phase Sequence: A, D, E, F
(* Variable Phase)
Movement Performance - Vehicles


Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## Site: 1 [7. Princes Hwy-Kalandar (Future Sat-120th HH) ]

13S1231000 - West Culburra Subdivision
Princes Highway-Kalandar Street
Saturday - Equivalent 120th HH
Site Category: (None)
Signals - Fixed Time Coordinated Cycle Time $=135$ seconds (Site Optimum Cycle Time - Minimum Delay)
Variable Sequence Analysis applied. The results are given for the selected output sequence.
Timings based on settings in the Site Phasing \& Timing dialog
Phase Times determined by the program
Phase Sequence: GTA - RMS/Video survey phasing
Reference Phase: Phase A
Input Phase Sequence: A, D, E, F, F1*, F2*
Output Phase Sequence: A, D, E, F
(* Variable Phase)
Movement Performance - Vehicles

| Mov ID | Turn | Demand <br> Total veh/h | lows HV $\%$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average <br> Speed km/h |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 5 | 20.0 | 0.693 | 44.2 | LOS D | 25.3 | 181.2 | 0.80 | 0.72 | 1.11 | 33.9 |
| 2 | T1 | 960 | 2.6 | 0.693 | 34.5 | LOS C | 25.3 | 181.2 | 0.77 | 0.69 | 0.94 | 42.1 |
| 3 | R2 | 117 | 4.5 | 1.096 | 176.2 | LOS F | 13.4 | 97.2 | 1.00 | 1.19 | 2.14 | 12.5 |
| Appr |  | 1082 | 2.9 | 1.096 | 49.9 | LOS D | 25.3 | 181.2 | 0.80 | 0.74 | 1.07 | 34.9 |
| East: Kalandar St (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 93 | 0.0 | 1.089 | 151.3 | LOS F | 44.5 | 312.9 | 1.00 | 1.32 | 1.92 | 12.8 |
| 5 | T1 | 217 | 0.5 | 1.089 | 145.7 | LOS F | 44.5 | 312.9 | 1.00 | 1.32 | 1.92 | 10.6 |
| 6 | R2 | 560 | 0.9 | 1.089 | 164.3 | LOS F | 49.7 | 350.5 | 1.00 | 1.29 | 1.92 | 13.0 |
| Appr |  | 869 | 0.7 | 1.089 | 158.3 | LOS F | 49.7 | 350.5 | 1.00 | 1.30 | 1.92 | 12.4 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 505 | 0.2 | 0.365 | 7.0 | LOS A | 0.9 | 6.4 | 0.03 | 0.60 | 0.03 | 56.1 |
| 8 | T1 | 1386 | 1.0 | 1.112 | 164.0 | LOS F | 94.7 | 668.7 | 0.99 | 1.58 | 1.89 | 16.6 |
| 9 | R2 | 91 | 11.6 | 0.904 | 89.5 | LOS F | 6.9 | 53.2 | 1.00 | 0.96 | 1.51 | 21.9 |
| Approach |  | 1982 | 1.3 | 1.112 | 120.6 | LOS F | 94.7 | 668.7 | 0.75 | 1.30 | 1.40 | 19.7 |
| West: Kalandar St (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & 10 \\ & 11 \\ & 12 \end{aligned}$ | L2 | 66 | 1.6 | 0.775 | 67.6 | LOS E | 15.9 | 111.7 | 1.00 | 0.90 | 1.10 | 27.3 |
|  | T1 | 173 | 0.0 | 0.775 | 62.0 | LOS E | 15.9 | 111.7 | 1.00 | 0.90 | 1.10 | 22.4 |
|  | R2 | 173 | 0.6 | 0.600 | 63.8 | LOS E | 10.8 | 75.9 | 0.98 | 0.81 | 0.98 | 27.1 |
| Approach |  | 412 | 0.5 | 0.775 | 63.6 | LOS E | 15.9 | 111.7 | 0.99 | 0.86 | 1.05 | 25.3 |
| All Vehicles |  | 4345 | 1.5 | 1.112 | 105.1 | LOS F | 94.7 | 668.7 | 0.83 | 1.12 | 1.39 | 20.5 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [8. Coonamia-Currarong-Forest (Future Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Coonamia Road- Currarong Road-Forest Road
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman <br> Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average <br> Speed km/h |
| East: Currarong Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 13 | 0.0 | 0.042 | 0.9 | LOS A | 0.2 | 1.2 | 0.33 | 0.49 | 0.33 | 83.6 |
| 6 | R2 | 40 | 0.0 | 0.042 | 8.6 | LOS A | 0.2 | 1.2 | 0.33 | 0.49 | 0.33 | 76.1 |
| Appr |  | 53 | 0.0 | 0.042 | 6.7 | NA | 0.2 | 1.2 | 0.33 | 0.49 | 0.33 | 77.8 |
| North: Coonamia Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 7 | 0.0 | 0.007 | 8.4 | LOS A | 0.0 | 0.2 | 0.23 | 0.60 | 0.23 | 73.9 |
| 9 | R2 | 84 | 1.3 | 0.123 | 9.4 | LOS A | 0.5 | 3.5 | 0.37 | 0.68 | 0.37 | 71.4 |
| Appr |  | 92 | 1.1 | 0.123 | 9.3 | LOS A | 0.5 | 3.5 | 0.36 | 0.67 | 0.36 | 71.6 |
| West: Forest Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 214 | 2.0 | 0.127 | 7.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 0.00 | 74.7 |
| 11 | T1 | 19 | 5.6 | 0.127 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 0.00 | 83.5 |
| Approach |  | 233 | 2.3 | 0.127 | 7.3 | NA | 0.0 | 0.0 | 0.00 | 0.61 | 0.00 | 75.3 |
| All Vehicles |  | 377 | 1.7 | 0.127 | 7.7 | NA | 0.5 | 3.5 | 0.13 | 0.61 | 0.13 | 74.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [8. Coonamia-Currarong-Forest (Future Fri PM-120th HH)]

13S1231000 - West Culburra Subdivision
Coonamia Road- Currarong Road-Forest Road
Friday PM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{gathered} \text { lows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Currarong Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 15 | 0.0 | 0.020 | 0.6 | LOS A | 0.1 | 0.6 | 0.26 | 0.32 | 0.26 | 88.0 |
| 6 | R2 | 15 | 0.0 | 0.020 | 8.4 | LOS A | 0.1 | 0.6 | 0.26 | 0.32 | 0.26 | 79.7 |
| Appr |  | 29 | 0.0 | 0.020 | 4.5 | NA | 0.1 | 0.6 | 0.26 | 0.32 | 0.26 | 83.6 |
| North: Coonamia Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 37 | 0.0 | 0.034 | 8.3 | LOS A | 0.1 | 0.9 | 0.22 | 0.61 | 0.22 | 74.0 |
| 9 | R2 | 201 | 4.2 | 0.283 | 9.4 | LOS A | 1.3 | 9.6 | 0.39 | 0.68 | 0.39 | 70.5 |
| Appr |  | 238 | 3.5 | 0.283 | 9.2 | LOS A | 1.3 | 9.6 | 0.36 | 0.67 | 0.36 | 71.0 |
| West: Forest Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 156 | 5.4 | 0.105 | 8.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.55 | 0.00 | 74.6 |
| 11 | T1 | 34 | 3.1 | 0.105 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.55 | 0.00 | 85.0 |
| Approach |  | 189 | 5.0 | 0.105 | 6.6 | NA | 0.0 | 0.0 | 0.00 | 0.55 | 0.00 | 76.3 |
| All Vehicles |  | 457 | 3.9 | 0.283 | 7.8 | NA | 1.3 | 9.6 | 0.20 | 0.60 | 0.20 | 73.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## $\nabla$ Site: 1 [8. Coonamia-Currarong-Forest (Future Sat-120th HH)]

13S1231000 - West Culburra Subdivision
Coonamia Road- Currarong Road-Forest Road
Saturday - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| East: Currarong Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | T1 | 11 | 0.0 | 0.028 | 0.7 | LOS A | 0.1 | 0.8 | 0.29 | 0.45 | 0.29 | 84.6 |
| 6 | R2 | 26 | 0.0 | 0.028 | 8.4 | LOS A | 0.1 | 0.8 | 0.29 | 0.45 | 0.29 | 76.9 |
| Appr |  | 37 | 0.0 | 0.028 | 6.2 | NA | 0.1 | 0.8 | 0.29 | 0.45 | 0.29 | 78.9 |
| North: Coonamia Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 29 | 0.0 | 0.027 | 8.3 | LOS A | 0.1 | 0.7 | 0.21 | 0.61 | 0.21 | 74.0 |
| 9 | R2 | 174 | 1.8 | 0.240 | 9.1 | LOS A | 1.1 | 7.7 | 0.37 | 0.67 | 0.37 | 71.5 |
| Appr |  | 203 | 1.6 | 0.240 | 9.0 | LOS A | 1.1 | 7.7 | 0.34 | 0.67 | 0.34 | 71.9 |
| West: Forest Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 173 | 0.6 | 0.103 | 7.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 75.5 |
| 11 | T1 | 19 | 0.0 | 0.103 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 83.8 |
| Approach |  | 192 | 0.5 | 0.103 | 7.1 | NA | 0.0 | 0.0 | 0.00 | 0.60 | 0.00 | 76.2 |
| All Vehicles |  | 432 | 1.0 | 0.240 | 7.9 | NA | 1.1 | 7.7 | 0.19 | 0.62 | 0.19 | 74.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

F Site: 1 [9. Kalandar St-Kinghorne St (Future Fri AM-120th HH)]
13S1231000 - West Culburra Subdivision
Kalandar Street-Kinghorne Street-Albatross Road
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman <br> Total veh/h | Fows HV $\%$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Kinghorne Sttreet |  |  |  |  |  |  |  |  |  |  |  |  |
| 1b | L3 | 31 | 6.9 | 0.739 | 18.0 | LOS B | 10.7 | 76.9 | 1.00 | 1.15 | 1.50 | 45.1 |
| 2 | T1 | 483 | 3.5 | 0.739 | 17.8 | LOS B | 10.7 | 76.9 | 1.00 | 1.15 | 1.50 | 46.3 |
| 3 | R2 | 60 | 0.0 | 0.739 | 21.5 | LOS B | 10.7 | 76.9 | 1.00 | 1.15 | 1.50 | 46.2 |
| Appro |  | 574 | 3.3 | 0.739 | 18.2 | LOS B | 10.7 | 76.9 | 1.00 | 1.15 | 1.50 | 46.2 |
| East: Kalandar Street |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 31 | 0.0 | 0.439 | 5.7 | LOS A | 2.8 | 20.9 | 0.47 | 0.62 | 0.47 | 52.4 |
| 4a | L1 | 293 | 8.3 | 0.439 | 5.8 | LOS A | 2.8 | 20.9 | 0.47 | 0.62 | 0.47 | 52.9 |
| 6 | R2 | 159 | 2.0 | 0.439 | 9.9 | LOS A | 2.8 | 20.9 | 0.47 | 0.62 | 0.47 | 53.2 |
| Appro |  | 482 | 5.7 | 0.439 | 7.2 | LOS A | 2.8 | 20.9 | 0.47 | 0.62 | 0.47 | 53.0 |
| North: Kinghorne Street |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 132 | 4.0 | 0.341 | 6.2 | LOS A | 2.3 | 16.9 | 0.59 | 0.68 | 0.59 | 51.8 |
| 8 | T1 | 31 | 0.0 | 0.341 | 6.3 | LOS A | 2.3 | 16.9 | 0.59 | 0.68 | 0.59 | 53.0 |
| 9 a | R1 | 176 | 3.6 | 0.341 | 9.4 | LOS A | 2.3 | 16.9 | 0.59 | 0.68 | 0.59 | 52.2 |
| Appro |  | 338 | 3.4 | 0.341 | 7.8 | LOS A | 2.3 | 16.9 | 0.59 | 0.68 | 0.59 | 52.1 |
| SouthWest: Albatross Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 30a | L1 | 261 | 4.4 | 0.764 | 19.9 | LOS B | 10.5 | 77.3 | 1.00 | 1.23 | 1.59 | 44.2 |
| 32a | R1 | 211 | 7.5 | 0.764 | 23.4 | LOS B | 10.5 | 77.3 | 1.00 | 1.23 | 1.59 | 43.9 |
| 32b | R3 | 11 | 20.0 | 0.764 | 26.0 | LOS B | 10.5 | 77.3 | 1.00 | 1.23 | 1.59 | 44.1 |
| Approach |  | 482 | 6.1 | 0.764 | 21.6 | LOS B | 10.5 | 77.3 | 1.00 | 1.23 | 1.59 | 44.1 |
| All Vehicles |  | 1876 | 4.7 | 0.764 | 14.4 | LOS A | 10.7 | 77.3 | 0.79 | 0.95 | 1.09 | 48.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

F Site: 1 [9. Kalandar St-Kinghorne St (Future Fri PM-120th HH)]
13S1231000 - West Culburra Subdivision
Kalandar Street-Kinghorne Street-Albatross Road
Friday AM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Demand Total veh/h | $\begin{gathered} \text { ows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average <br> Speed km/h |
| South: Kinghorne Sttreet |  |  |  |  |  |  |  |  |  |  |  |  |
| 1b | L3 | 34 | 0.0 | 0.579 | 12.6 | LOS A | 5.8 | 40.4 | 0.91 | 0.99 | 1.12 | 48.2 |
| 2 | T1 | 326 | 0.0 | 0.579 | 12.6 | LOS A | 5.8 | 40.4 | 0.91 | 0.99 | 1.12 | 49.4 |
| 3 | R2 | 78 | 0.0 | 0.579 | 16.5 | LOS B | 5.8 | 40.4 | 0.91 | 0.99 | 1.12 | 49.2 |
| Appro |  | 438 | 0.0 | 0.579 | 13.3 | LOS A | 5.8 | 40.4 | 0.91 | 0.99 | 1.12 | 49.3 |
| East: Kalandar Street |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 41 | 0.0 | 0.448 | 7.0 | LOS A | 3.0 | 21.0 | 0.63 | 0.72 | 0.63 | 51.9 |
| 4a | L1 | 245 | 0.0 | 0.448 | 6.9 | LOSA | 3.0 | 21.0 | 0.63 | 0.72 | 0.63 | 52.6 |
| 6 | R2 | 118 | 0.0 | 0.448 | 11.2 | LOSA | 3.0 | 21.0 | 0.63 | 0.72 | 0.63 | 52.7 |
| Appro |  | 404 | 0.0 | 0.448 | 8.1 | LOS A | 3.0 | 21.0 | 0.63 | 0.72 | 0.63 | 52.6 |
| North: Kinghorne Street |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 319 | 0.0 | 0.773 | 12.7 | LOS A | 11.3 | 78.9 | 0.98 | 1.03 | 1.31 | 47.9 |
| 8 | T1 | 80 | 0.0 | 0.773 | 13.0 | LOS A | 11.3 | 78.9 | 0.98 | 1.03 | 1.31 | 48.8 |
| 9a | R1 | 304 | 0.0 | 0.773 | 15.9 | LOS B | 11.3 | 78.9 | 0.98 | 1.03 | 1.31 | 48.3 |
| Approach |  | 703 | 0.0 | 0.773 | 14.1 | LOS A | 11.3 | 78.9 | 0.98 | 1.03 | 1.31 | 48.2 |
| SouthWest: Albatross Road |  |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & 30 a \\ & 32 a \\ & 32 b \end{aligned}$ | L1 | 300 | 0.0 | 0.780 | 15.1 | LOS B | 11.6 | 81.0 | 1.00 | 1.13 | 1.46 | 46.8 |
|  | R1 | 331 | 0.0 | 0.780 | 18.5 | LOS B | 11.6 | 81.0 | 1.00 | 1.13 | 1.46 | 46.6 |
|  | R3 | 15 | 0.0 | 0.780 | 20.4 | LOS B | 11.6 | 81.0 | 1.00 | 1.13 | 1.46 | 47.2 |
| Approach |  | 645 | 0.0 | 0.780 | 17.0 | LOS B | 11.6 | 81.0 | 1.00 | 1.13 | 1.46 | 46.7 |
| All Vehicles |  | 2191 | 0.0 | 0.780 | 13.7 | LOS A | 11.6 | 81.0 | 0.91 | 0.99 | 1.19 | 48.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

F Site: 1 [9. Kalandar St-Kinghorne St (Future Sat-120th HH)]
13S1231000 - West Culburra Subdivision
Kalandar Street-Kinghorne Street-Albatross Road
Saturday - Equivalent 120th HH
Site Category: (None)
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Total veh/h |  | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Kinghorne Street 0 |  |  |  |  |  |  |  |  |  |  |  |  |
| 1b | L3 | 21 | 10.0 | 0.227 | 7.5 | LOS A | 1.4 | 10.0 | 0.60 | 0.66 | 0.60 | 51.3 |
| 2 | T1 | 162 | 1.3 | 0.227 | 7.2 | LOS A | 1.4 | 10.0 | 0.60 | 0.66 | 0.60 | 53.0 |
| 3 | R2 | 29 | 0.0 | 0.227 | 11.1 | LOS A | 1.4 | 10.0 | 0.60 | 0.66 | 0.60 | 52.8 |
| Appr | ach | 213 | 2.0 | 0.227 | 7.8 | LOS A | 1.4 | 10.0 | 0.60 | 0.66 | 0.60 | 52.8 |
| East: Kalandar Street |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 22 | 0.0 | 0.272 | 5.4 | LOS A | 1.5 | 10.8 | 0.38 | 0.59 | 0.38 | 52.7 |
| 4a | L1 | 181 | 7.0 | 0.272 | 5.4 | LOS A | 1.5 | 10.8 | 0.38 | 0.59 | 0.38 | 53.3 |
| 6 | R2 | 98 | 1.1 | 0.272 | 9.6 | LOS A | 1.5 | 10.8 | 0.38 | 0.59 | 0.38 | 53.5 |
| Appr |  | 301 | 4.5 | 0.272 | 6.8 | LOS A | 1.5 | 10.8 | 0.38 | 0.59 | 0.38 | 53.3 |
| North: Kinghorne Street |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 153 | 0.7 | 0.314 | 5.9 | LOS A | 2.0 | 14.4 | 0.53 | 0.66 | 0.53 | 52.2 |
| 8 | T1 | 21 | 0.0 | 0.314 | 6.2 | LOS A | 2.0 | 14.4 | 0.53 | 0.66 | 0.53 | 53.3 |
| 9 a | R1 | 157 | 2.7 | 0.314 | 9.2 | LOS A | 2.0 | 14.4 | 0.53 | 0.66 | 0.53 | 52.6 |
| Appr |  | 331 | 1.6 | 0.314 | 7.5 | LOS A | 2.0 | 14.4 | 0.53 | 0.66 | 0.53 | 52.4 |
| SouthWest: Albatross Road |  |  |  |  |  |  |  |  |  |  |  |  |
| 30a | L1 | 161 | 1.3 | 0.378 | 6.1 | LOS A | 2.6 | 18.5 | 0.57 | 0.67 | 0.57 | 52.5 |
| 32a | R1 | 216 | 0.5 | 0.378 | 9.4 | LOS A | 2.6 | 18.5 | 0.57 | 0.67 | 0.57 | 52.2 |
| 32b | R3 | 20 | 10.5 | 0.378 | 11.6 | LOS A | 2.6 | 18.5 | 0.57 | 0.67 | 0.57 | 52.5 |
| Appr |  | 397 | 1.3 | 0.378 | 8.2 | LOS A | 2.6 | 18.5 | 0.57 | 0.67 | 0.57 | 52.3 |
| All V | icles | 1241 | 2.3 | 0.378 | 7.6 | LOS A | 2.6 | 18.5 | 0.52 | 0.65 | 0.52 | 52.7 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: GTA CONSULTANTS | Created: Thursday, 19 March 2020 6:06:37 PM
Project: \Igta.com.aulprojectfiles\ProjectFilesSydN18600-18699IN186580 West Culburra Concept Plan\Modellingl200225sid-N186580 West Culburra Subdivision.sip8

Site Category: -
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{array}{ll} \hline \text { Mov } & \text { Turn } \\ \text { ID } \end{array}$ | Demand Total | $\begin{aligned} & \text { Flows } \\ & \mathrm{HV} \end{aligned}$ | Arrival Total | $\begin{gathered} \text { Flows } \\ \text { HV } \end{gathered}$ | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Prop. Vehicles Distance Queued$\qquad$veh m |  |  | Effective Aver. No.Average Stop Cycles Speed Rate |  |  |
|  | veh/h |  | $\mathrm{veh} / \mathrm{h}$ | \% | v/c | sec |  |  |  |  |  |  | km/h |
| South: Princes Highway |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 38 | 8.3 | 38 | 8.3 | 0.051 | 11.0 | LOS A | 0.1 | 0.6 | 0.52 | 0.75 | 0.52 | 67.5 |
| Approach | 38 | 8.3 | 38 | 8.3 | 0.051 | 11.0 | NA | 0.1 | 0.6 | 0.52 | 0.75 | 0.52 | 67.5 |
| East: Forest Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 47 | 2.2 | 47 | 2.2 | 0.038 | 8.8 | LOS A | 0.1 | 0.4 | 0.35 | 0.64 | 0.35 | 71.6 |
| 6a R1 | 107 | 2.0 | 107 | 2.0 | 0.250 | 14.9 | LOS B | 0.4 | 2.9 | 0.66 | 0.88 | 0.72 | 55.6 |
| Approach | 155 | 2.0 | 155 | 2.0 | 0.250 | 13.0 | LOS A | 0.4 | 2.9 | 0.56 | 0.81 | 0.60 | 62.0 |
| North: Princes Highway |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 63 | 8.3 | 63 | 8.3 | 0.042 | 8.7 | LOS A | 0.1 | 0.5 | 0.11 | 0.60 | 0.11 | 70.9 |
| 8 T1 | 521 | 16.0 | 521 | 16.0 | 0.147 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 99.9 |
| Approach | 584 | 15.1 | 584 | 15.1 | 0.147 | 0.9 | LOS A | 0.1 | 0.5 | 0.01 | 0.07 | 0.01 | 95.6 |
| All Vehicles | 777 | 12.2 | 777 | 12.2 | 0.250 | 3.8 | NA | 0.4 | 2.9 | 0.15 | 0.25 | 0.16 | 87.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: P:IN18600-18699\N186580 West Culburra Concept Plan\Modelling\200225sid-N186580 West Culburra Subdivision.sip8
$\nabla$ site: 1 [10a. Princes Hwy-Forest (Future Fri 呻 Network: 6 [10. Princes Hwy-Forest (Future Fri PM-120th HH)] PM-120th HH)]

Site Category: -
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand Total veh/h | Demand Flows Arrival Flows Total HV Total HV | Arriva Total veh/h | Fows <br> HV <br> \% | Deg. Satn v/c | Average Delay sec | Level of Service | Aver. Back of Queue Prop. Vehicles Distance Queued |  |  | Effective Aver. No.Average Stop Cycles Speed |  |  |
| South: Princes Highway |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 61 | 1.7 | 61 | 1.7 | 0.249 | 24.1 | LOS B | 0.3 | 2.4 | 0.88 | 0.97 | 0.97 | 55.6 |
| Approach | 61 | 1.7 | 61 | 1.7 | 0.249 | 24.1 | NA | 0.3 | 2.4 | 0.88 | 0.97 | 0.97 | 55.6 |
| East: Forest Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 60 | 0.0 | 60 | 0.0 | 0.076 | 11.1 | LOS A | 0.1 | 0.8 | 0.57 | 0.81 | 0.57 | 70.5 |
| 6a R1 | 72 | 1.5 | 72 | 1.5 | 0.872 | 126.0 | LOS F | 1.7 | 11.8 | 0.99 | 1.17 | 2.11 | 12.8 |
| Approach | 132 | 0.8 | 132 | 0.8 | 0.872 | 73.6 | LOS F | 1.7 | 11.8 | 0.80 | 1.01 | 1.41 | 26.2 |
| North: Princes Highway |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 157 | 2.0 | 157 | 2.0 | 0.102 | 8.6 | LOS A | 0.2 | 1.2 | 0.15 | 0.60 | 0.15 | 72.7 |
| 8 T1 | 1475 | 2.0 | 1475 | 2.0 | 0.383 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 99.7 |
| Approach | 1632 | 2.0 | 1632 | 2.0 | 0.383 | 0.9 | LOS A | 0.2 | 1.2 | 0.01 | 0.06 | 0.01 | 96.2 |
| All Vehicles | 1824 | 1.9 | 1824 | 1.9 | 0.872 | 6.9 | NA | 1.7 | 11.8 | 0.10 | 0.16 | 0.15 | 82.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: GTA CONSULTANTS | Created: Thursday, 19 March 2020 6:10:45 PM
Project: P:IN18600-18699|N186580 West Culburra Concept Plan\Modellingl200225sid-N186580 West Culburra Subdivision.sip8
$\nabla$ site: 1 [10a. Princes Hwy-Forest (Future Fri 呻 Network: 6 [10. Princes Hwy-Forest (Future Fri PM-120th HH)] PM-120th HH)]

Site Category: -
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | Demand Total veh/h | Demand Flows Arrival Flows Total HV Total HV | Arriva Total veh/h | Fows <br> HV <br> \% | Deg. Satn v/c | Average Delay sec | Level of Service | Aver. Back of Queue Prop. Vehicles Distance Queued |  |  | Effective Aver. No.Average Stop Cycles Speed |  |  |
| South: Princes Highway |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 R2 | 61 | 1.7 | 61 | 1.7 | 0.249 | 24.1 | LOS B | 0.3 | 2.4 | 0.88 | 0.97 | 0.97 | 55.6 |
| Approach | 61 | 1.7 | 61 | 1.7 | 0.249 | 24.1 | NA | 0.3 | 2.4 | 0.88 | 0.97 | 0.97 | 55.6 |
| East: Forest Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 60 | 0.0 | 60 | 0.0 | 0.076 | 11.1 | LOS A | 0.1 | 0.8 | 0.57 | 0.81 | 0.57 | 70.5 |
| 6a R1 | 72 | 1.5 | 72 | 1.5 | 0.872 | 126.0 | LOS F | 1.7 | 11.8 | 0.99 | 1.17 | 2.11 | 12.8 |
| Approach | 132 | 0.8 | 132 | 0.8 | 0.872 | 73.6 | LOS F | 1.7 | 11.8 | 0.80 | 1.01 | 1.41 | 26.2 |
| North: Princes Highway |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 157 | 2.0 | 157 | 2.0 | 0.102 | 8.6 | LOS A | 0.2 | 1.2 | 0.15 | 0.60 | 0.15 | 72.7 |
| 8 T1 | 1475 | 2.0 | 1475 | 2.0 | 0.383 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 99.7 |
| Approach | 1632 | 2.0 | 1632 | 2.0 | 0.383 | 0.9 | LOS A | 0.2 | 1.2 | 0.01 | 0.06 | 0.01 | 96.2 |
| All Vehicles | 1824 | 1.9 | 1824 | 1.9 | 0.872 | 6.9 | NA | 1.7 | 11.8 | 0.10 | 0.16 | 0.15 | 82.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: GTA CONSULTANTS | Created: Thursday, 19 March 2020 6:10:45 PM
Project: P:IN18600-18699|N186580 West Culburra Concept Plan\Modellingl200225sid-N186580 West Culburra Subdivision.sip8

## USER REPORT FOR SITE

Project: 200225sid-N186580 West Culburra Subdivision

## Site: 1 [11. Princes Hwy-Moss (Future Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Princes Highway-Moss Street
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Signals - Fixed Time Coordinated Cycle Time = 135 seconds (Site Optimum Cycle Time - Minimum Delay)
Variable Sequence Analysis applied. The results are given for the selected output sequence.
Timings based on settings in the Site Phasing \& Timing dialog
Phase Times determined by the program
Phase Sequence: GTA - RMS/Video survey phasing/timing

## Reference Phase: Phase A

Input Phase Sequence: A, B, D, E, F, F1*, F2*
Output Phase Sequence: A, B, D, E, F
(* Variable Phase)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  | Deman <br> Total veh/h | $\begin{gathered} \text { =lows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 5 | 0.0 | 0.967 | 95.2 | LOS F | 24.0 | 178.7 | 1.00 | 1.09 | 1.59 | 24.0 |
| 2 | T1 | 921 | 7.7 | 1.046 | 115.5 | LOS F | 31.7 | 236.7 | 1.00 | 1.21 | 1.67 | 21.6 |
| 3 | R2 | 158 | 4.0 | 0.738 | 72.3 | LOS F | 10.6 | 76.9 | 1.00 | 0.85 | 1.10 | 27.1 |
| Appr |  | 1084 | 7.1 | 1.046 | 109.1 | LOS F | 31.7 | 236.7 | 1.00 | 1.15 | 1.59 | 22.2 |
| East: Moss St (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 37 | 2.9 | 0.396 | 51.3 | LOS D | 9.6 | 69.8 | 0.89 | 0.75 | 0.89 | 32.2 |
| 5 | T1 | 238 | 4.4 | 1.074 | 92.4 | LOS F | 42.1 | 306.6 | 0.94 | 1.02 | 1.32 | 21.7 |
| 6 | R2 | 268 | 4.7 | 1.074 | 158.6 | LOS F | 42.1 | 306.6 | 1.00 | 1.39 | 1.89 | 16.2 |
| Appr |  | 543 | 4.5 | 1.074 | 122.3 | LOS F | 42.1 | 306.6 | 0.97 | 1.19 | 1.57 | 18.9 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 196 | 4.8 | 1.065 | 121.9 | LOS F | 51.7 | 380.7 | 1.00 | 1.25 | 1.70 | 17.6 |
| 8 | T1 | 1445 | 6.8 | 1.065 | 130.3 | LOS F | 57.1 | 423.1 | 1.00 | 1.36 | 1.71 | 19.0 |
| 9 | R2 | 380 | 3.6 | 1.049 | 116.3 | LOS F | 32.0 | 230.8 | 1.00 | 1.14 | 1.77 | 17.5 |
| Appr |  | 2021 | 6.0 | 1.065 | 126.9 | LOS F | 57.1 | 423.1 | 1.00 | 1.31 | 1.72 | 18.6 |
| West: Moss St (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L2 | 126 | 10.0 | 0.177 | 18.1 | LOS B | 3.0 | 22.8 | 0.66 | 0.71 | 0.66 | 42.3 |
| 11 | T1 | 138 | 3.8 | 0.543 | 53.7 | LOS D | 11.2 | 82.6 | 0.95 | 0.79 | 0.95 | 28.7 |
| 12 | R2 | 49 | 12.8 | 0.543 | 58.9 | LOS E | 11.2 | 82.6 | 0.96 | 0.80 | 0.96 | 29.6 |
| Approach |  | 314 | 7.7 | 0.543 | 40.2 | LOS C | 11.2 | 82.6 | 0.84 | 0.76 | 0.84 | 33.1 |
| All V | icles | 3962 | 6.2 | 1.074 | 114.5 | LOS F | 57.1 | 423.1 | 0.98 | 1.20 | 1.59 | 20.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 1 [11. Princes Hwy-Moss (Future Fri PM-120th HH)]
$13 S 1231000$ - West Culburra Subdivision
Princes Highway-Moss Street
Friday PM (1600-1700) - Equivalent 120th HH
Site Category: (None)
Signals - Fixed Time Coordinated Cycle Time $=125$ seconds (Site Optimum Cycle Time - Minimum Delay)
Variable Sequence Analysis applied. The results are given for the selected output sequence.
Timings based on settings in the Site Phasing \& Timing dialog
Phase Times determined by the program
Phase Sequence: GTA - RMS/Video survey phasing/timing
Reference Phase: Phase A
Input Phase Sequence: A, B, D, E, F, F1*, F2*
Output Phase Sequence: A, B, D, E, F
(* Variable Phase)
Movement Performance - Vehicles

| Mov Turn ID | Demand <br> Total veh/h | $\begin{array}{r} \text { lows } \\ \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 12 | 0.0 | 1.070 | 135.8 | LOS F | 33.3 | 239.3 | 1.00 | 1.28 | 1.85 | 17.4 |
| 2 T1 | 1128 | 3.1 | 1.158 | 183.2 | LOS F | 49.4 | 354.7 | 1.00 | 1.49 | 2.12 | 14.9 |
| 3 R 2 | 115 | 0.0 | 0.454 | 61.3 | LOS E | 6.6 | 46.2 | 0.97 | 0.79 | 0.97 | 29.5 |
| Approach | 1255 | 2.8 | 1.158 | 171.6 | LOS F | 49.4 | 354.7 | 1.00 | 1.42 | 2.02 | 15.6 |
| East: Moss St (E) |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 36 | 2.9 | 0.339 | 50.3 | LOS D | 7.0 | 49.1 | 0.90 | 0.74 | 0.90 | 32.4 |
| $5 \quad$ T1 | 158 | 0.0 | 0.919 | 55.2 | LOS D | 21.4 | 151.7 | 0.94 | 0.86 | 1.08 | 28.1 |
| 6 R2 | 240 | 1.8 | 0.919 | 75.0 | LOS F | 21.4 | 151.7 | 1.00 | 1.06 | 1.37 | 26.3 |
| Approach | 434 | 1.2 | 0.919 | 65.7 | LOS E | 21.4 | 151.7 | 0.97 | 0.96 | 1.22 | 27.4 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 209 | 4.0 | 0.981 | 87.3 | LOS F | 41.7 | 301.2 | 1.00 | 1.17 | 1.67 | 24.9 |
| 8 T1 | 1344 | 3.5 | 0.981 | 80.9 | LOS F | 42.0 | 302.8 | 1.00 | 1.16 | 1.47 | 27.4 |
| 9 R 2 | 408 | 0.8 | 1.152 | 193.4 | LOS F | 44.2 | 311.7 | 1.00 | 1.33 | 2.29 | 12.8 |
| Approach | 1962 | 3.0 | 1.152 | 105.0 | LOS F | 44.2 | 311.7 | 1.00 | 1.20 | 1.66 | 22.0 |
| West: Moss St (W) |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 506 | 1.2 | 0.637 | 19.9 | LOS B | 13.1 | 92.8 | 0.84 | 0.82 | 0.84 | 42.6 |
| 11 T1 | 255 | 0.0 | 1.212 | 257.3 | LOS F | 56.2 | 394.0 | 1.00 | 1.95 | 2.56 | 10.8 |
| 12 R 2 | 135 | 0.8 | 1.212 | 261.9 | LOS F | 56.2 | 394.0 | 1.00 | 1.95 | 2.56 | 11.0 |
| Approach | 896 | 0.8 | 1.212 | 123.8 | LOS F | 56.2 | 394.0 | 0.91 | 1.31 | 1.59 | 18.7 |
| All Vehicles | 4546 | 2.3 | 1.212 | 123.3 | LOS F | 56.2 | 394.0 | 0.98 | 1.26 | 1.70 | 19.5 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

目 Site: 1 [11. Princes Hwy-Moss (Future Sat-120th HH)]
13S1231000 - West Culburra Subdivision
Princes Highway-Moss Street
Saturday - Equivalent 120th HH
Site Category: (None)
Signals - Fixed Time Coordinated Cycle Time $=130$ seconds (Site Optimum Cycle Time - Minimum Delay)
Variable Sequence Analysis applied. The results are given for the selected output sequence.
Timings based on settings in the Site Phasing \& Timing dialog
Phase Times determined by the program
Phase Sequence: GTA - RMS/Video survey phasing/timing
Reference Phase: Phase A
Input Phase Sequence: A, B, D, E, F, F1*, F2*
Output Phase Sequence: A, B, D, E, F
(* Variable Phase)

Movement Performance - Vehicles

| Mov ID |  | Demand <br> Total veh/h | $\begin{array}{r} \text { lows } \\ \text { HV } \\ \% \\ \hline \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 14 | 0.0 | 0.889 | 69.4 | LOS E | 25.7 | 184.1 | 1.00 | 0.99 | 1.32 | 28.9 |
| 2 | T1 | 1181 | 2.7 | 0.962 | 73.6 | LOS F | 32.1 | 230.0 | 1.00 | 1.06 | 1.35 | 29.1 |
| 3 | R2 | 104 | 0.0 | 0.561 | 68.7 | LOS E | 6.5 | 45.7 | 1.00 | 0.79 | 1.00 | 27.8 |
| Appr | ch | 1299 | 2.4 | 0.962 | 73.2 | LOS F | 32.1 | 230.0 | 1.00 | 1.04 | 1.32 | 28.9 |
| East: Moss St (E) |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L2 | 35 | 0.0 | 0.186 | 48.6 | LOS D | 4.0 | 28.0 | 0.85 | 0.70 | 0.85 | 32.9 |
| 5 | T1 | 84 | 0.0 | 0.506 | 46.7 | LOS D | 11.0 | 78.3 | 0.89 | 0.75 | 0.89 | 29.9 |
| 6 | R2 | 154 | 2.1 | 0.506 | 54.0 | LOS D | 11.0 | 78.3 | 0.94 | 0.80 | 0.94 | 30.9 |
| Appr |  | 273 | 1.2 | 0.506 | 51.1 | LOS D | 11.0 | 78.3 | 0.91 | 0.77 | 0.91 | 30.8 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 120 | 2.6 | 0.783 | 51.2 | LOS D | 26.0 | 184.6 | 0.92 | 0.87 | 1.22 | 33.3 |
| 8 | T1 | 1312 | 1.5 | 0.783 | 43.8 | LOS D | 26.6 | 188.7 | 0.93 | 0.84 | 1.02 | 37.9 |
| 9 | R2 | 258 | 0.8 | 0.956 | 68.3 | LOS E | 15.3 | 107.8 | 1.00 | 1.02 | 1.52 | 27.9 |
| Approach |  | 1689 | 1.5 | 0.956 | 48.1 | LOS D | 26.6 | 188.7 | 0.94 | 0.87 | 1.11 | 35.6 |
| West: Moss St (W) |  |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & 10 \\ & 11 \\ & 12 \end{aligned}$ | L2 | 311 | 0.0 | 0.435 | 20.9 | LOS B | 7.5 | 52.2 | 0.79 | 0.78 | 0.79 | 42.4 |
|  | T1 | 163 | 0.0 | 0.726 | 54.8 | LOS D | 16.3 | 114.6 | 1.00 | 0.87 | 1.04 | 28.3 |
|  | R2 | 103 | 1.0 | 0.726 | 59.3 | LOS E | 16.3 | 114.6 | 1.00 | 0.87 | 1.04 | 30.1 |
| Approach |  | 577 | 0.2 | 0.726 | 37.3 | LOS C | 16.3 | 114.6 | 0.88 | 0.82 | 0.90 | 34.9 |
| All Vehicles |  | 3838 | 1.6 | 0.962 | 55.2 | LOS D | 32.1 | 230.0 | 0.95 | 0.91 | 1.14 | 32.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## D. SIDRA INTERSECTION LAYOUTS



## SITE LAYOUT

## Site: 1 [11. Princes Hwy-Moss (Ex Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Princes Highway-Moss Street
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Signals - Fixed Time Coordinated


## SITE LAYOUT

## Site: 1 [1. Culburra -Coonamia (Ex Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Culburra Road-Coonamia Road
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

4 Culburra Rd (W)


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## SITE LAYOUT

## $\checkmark$ Site: 1 [2. Culburra-Mayfield (Ex Fri AM-120th HH)]

Culburra Road-Mayfield Road
Friday AM (0800-0900) - Equivalent 120th HH
Existing
Site Category: (None)
Giveway / Yield (Two-Way)


Culburra Subdivision.sip8

## SITE LAYOUT

## Site: 1 [3. Greenwell Pt-Pyree (Ex Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Pyree Lane
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)

N Greenwell Pt Rd (W)


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## SITE LAYOUT

$\nabla$ Site: 1 [4. Greenwell Pt-Jindy Andy (Ex Fri AM-120th HH)]
13S1231000 - West Culburra Subdivision
Greenwell Point Road-Jindy Andy Lane
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)


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## SITE LAYOUT

$\checkmark$ Site: 1 [5. Greenwell Pt-Mayfield (Ex Fri AM-120th HH)]
13S1231000 - West Culburra Subdivision
Greenwell Point Road-Mayfield Road
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)


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## SITE LAYOUT

## ST0F Site: 1 [6. Greenwell Pt-Millbank-Worrigee (Ex Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Millbank Road-Worrigee Road
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Stop (Two-Way)


## SITE LAYOUT

## Site: 1 [7. Princes Hwy-Kalandar (Ex Fri AM-120th HH)]

13S1231000 - West Culburra Subdivision
Princes Highway-Kalandar Street
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Signals - Fixed Time Coordinated


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## SITE LAYOUT

$\nabla$ Site: 1 [8. Coonamia-Currarong-Forest (Ex Fri AM-120th HH)]
13S1231000 - West Culburra Subdivision
Coonamia Road- Currarong Road-Forest Road
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Giveway / Yield (Two-Way)


Currarong Road

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## SITE LAYOUT

9 Site: 1 [9. Kalandar St-Kinghorne St (Ex Fri AM-120th HH)]
13S1231000 - West Culburra Subdivision
Kalandar Street-Kinghorne Street-Albatross Road
Friday AM (0800-0900) - Equivalent 120th HH
Site Category: (None)
Roundabout


## NETWORK LAYOUT

虫审 Network: 1 [10. Princes Hwy-Forest (Ex Fri AM-120th HH)]

Network Category: -


SITES IN NETWORK

| Site ID | CCG ID | Site Name |
| :--- | :--- | :--- |
| $\nabla_{1}$ | NA | 10a. Princes Hwy-Forest (Ex Fri AM-120th HH) |
| $\nabla_{1}$ | NA | 10b. Princes Hwy-Forest (Ex Fri AM-120th HH) |

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## E.PRELIMINARY INTERSECTION CONCEPT DESIGN



## PRELIMINARY ENGINEERING DRAWINGS FOR PROPOSED MIXED-USE SUBDIVISION AT WEST CULBURRA

## DRAWING SCHEDULE


$25405-100$
$25405-101$
$25405-102$
$25405-103$
$25405-104$
$2540-105$
$25405-106$
$25405-107$
$25405-108$
$25405-109$
$25405-110$
$25405-111$
$25405-112$
$2540-113$
$25405-114$
$25405-115$
$25405-116$
$25405-117$
$25405-118$
$25405-119$
$25405-120$
$25405-121$
$2540-122$
$25405-123$
$25405-124$
$25405-125$
$25405-126$
$25405-127$
$25405-128$

COVER SHEET AND INDEX PLAN PRELIMINARY RESIDENTIAL PRECINCT LAYOUT PLAN PRELIMINARY INDUSTRIAL PRECINCT LAYOUT PLAN PRELIMINARY TOWN CENTRE PRECINCT LAYOUT PLAN PRELIMINARY OVERALL CONCEPT ROUNDABOUT 01 DESIGN ROLIDABOUT 01 VEHICL MOVEBENT LAYOUT PL ROUNDABOUT 01 VEHICLE MOVEMENT LAYOUT PLAN SHEET 01 PRELIMINARY ROUNDABOUT 01 SIGHT LNE LAYOUT PLAN PRELIMINARY CONCEPT INDUSTRIAL ENTRY AND EXIT LAYOUT PLAN PRELIMINARY CONCEPT INDUSTRIAL ENTRY AND EXIT VEHICLE MOVEMENTS AND SIGHT DISTANCE LAYOUT PLAN PRELIMINARY CONCEPT ROUNDABOUT 02 LAYOUT PLAN PRELIMINARY CONCEPT ROUNDABOUT 02 DESIGN ROUNDABOUT 02 VEHICLE MOVEMENT LAYOUT PLAN PRELIMINARY ROUNDABOUT 02 SIGHT LINE LAYOUT PLAN PRELIMINARY CONCEPT ROUNDABOUT 03 LAYOUT PLAN PRELIMINARY CONCEPT ROUNDABOUT 03 DESIGN ROUNDABOUT 03 VEHICLE MOVEMENT LAYOUT PLAN SHEET 01 ROUNDABOUT 03 VEHICLE MOVEMENT LAYOUT PLAN SHEET 02 ReLiminary roundabout ỏ sight Line Layout plan PRELIMNARY TYPICAL ROAD CROSS SECTIONS PLAN SHEET PRELIMNARY TYPICAL ROAD CROSS SECTIONS PLAN SHEET 03 PRELIMINARY TYPICAL ROAD CROSS SECTIONS PLAN SHEET 04 PRELIMINARY TYPICAL ROAD CROSS SECTIONS PLAN SHEET 0 WESTERN POND CONCEPT LAYOUT PLAN CENTRAL POND CONCEPT LAYOUT PLAN EASTERN POND CONCEPT LAYOUT PLAN TYPICAL WESTERN AND EASTERN POND CROSS SECTIONS PLAN


























SCALE 1:50


TYPICAL 7m ROAD IN 10 m ROAD RESERVE CROSS SECTION
FOR ACCESS ROAD 01
SCALE 1:50


## NOTE:



 FOR EMERGENCY EGRESS ROUTE 01 AND 02
TYPICAL 4m FIRE TRAIL PRIVATE PROPERTY/PUBLIC RESERVE CROSS SECTION SCALE 1:50

6) | DIALL BEFORG |
| :--- |
| YOU 0, |


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| RATIO: $1: 50$ <br> (AT A1 ORIGINAL) | DATUM <br> AUSTRALIAN HEIGHT DATUM <br> origin: SSM <br> RL | surver | APs | ReV Description | Br | date | allen price \& scarratts pty Itd land and development consultants Nowra Office: 75 Plunkett Street, Nowra NSW 2541 <br>  | TYPICAL WESTERN AND EASTERN POND CROSS SECTIONS PLAN <br> OVER CULBURRA ROAD <br> AT CULBURRA BEACH <br> FOR SEALARK PTY LTD | DRAWNG STATUS PRELIMINARYnot to BE USED FOR CONSTRUCTION PURPOSES |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | DESIGN | ${ }^{\text {cus }}$ |  |  |  |  |  |  |  |
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|  |  | CHECKD | MJP |  |  |  |  |  | 25405-128 | ог 29 |

## F.BUS OPERATOR CORRESPONDENCE



## Ken Hollyoak <br> Associate Director <br> GTA Consultants <br> PO Box 5254 <br> West Chatswood NSW 1515

## Re: Proposed Residential Development - Culburra

As a follow up from our initial discussion relating to your proposed development of 900 home sites in Culburra, the following information is provided.

Kennedys Bus \& Coach is the contracted provider for Transport for NSW in supplying school \& route services to Culburra / Orient Point. Additional patronage from your development would be welcomed and approval to include this development within our normal operations can be easily arranged. If the Development Proposal is approved we would be happy to extend our current service arrangements. There would be no additional cost to the government undercurrent contract arrangements.

It is important that you be aware in the design of this residential housing estate, that development enables bus stops to be located within 400 metre from access points / dwellings. The geometry of the road must be designed in order to allow for a minimum 12.5 metre to 13.5 metre bus to circulate through the proposed development.

Consideration also needs to be had relating to provision of Disability Access for low floor wheelchair buses which will be compulsory on all route services by 2020.

We look forward to providing services to this proposed new development. If I can be of any further assistance please contact me on the number below.

Yours sincerely

12th September 2072

GTAconsultants
www.gta.com.au


[^0]:    Source: Centre for Road Safety, Transport for New South Wales (accessed March 2020)

[^1]:    ${ }^{2}$ https://www.rms.nsw.gov.au/projects/01documents/nowra-bridges-shoalhaven-river/nowra-bridge-project-app-g-landscape-character-and-visual-impact-assessment.pdf

[^2]:    ${ }^{3}$ This refers to the proposed roundabout connecting Road 01 and Road 02

[^3]:    Source: Shoalhaven City Council (Appendix B of this report)

[^4]:    Fri AM Peak Conversion Factor for AADT Analysis: 0.94 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) Fri PM Peak Conversion Factor for AADT Analysis: Sat MD Peak Conversion Factor for AADT Analysis: 0.95 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) 1.14 (Converts consultant's 2012 survey data to AADT values for LOS C analysis)

[^5]:    SIDRA INTERSECTION 8.0 | Copyright © 2000-2019 Akcelik and Associates Pty Ltd | sidrasolutions.com
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